

PROJECT

901 N. LAUREL AVE.
 LOS ANGELES, CA 90046
 OWNER: LI INVESTMENTS LLC

City of Los Angeles, Department of Building & Safety
APPROVED PLANS

This perforated set of plans are not approval for construction until the required permit fees are paid and the permit is issued.
 The permit(s) is valid for two years from the date the permit fees are paid.
 Visit <http://www.permitting.org/perm/index.cfm> to check the status of this set of plans by entering the 15 digit Permit number. "Issued" status means the permit fees have been paid.
 No inspection can be scheduled until the permit fees are paid.

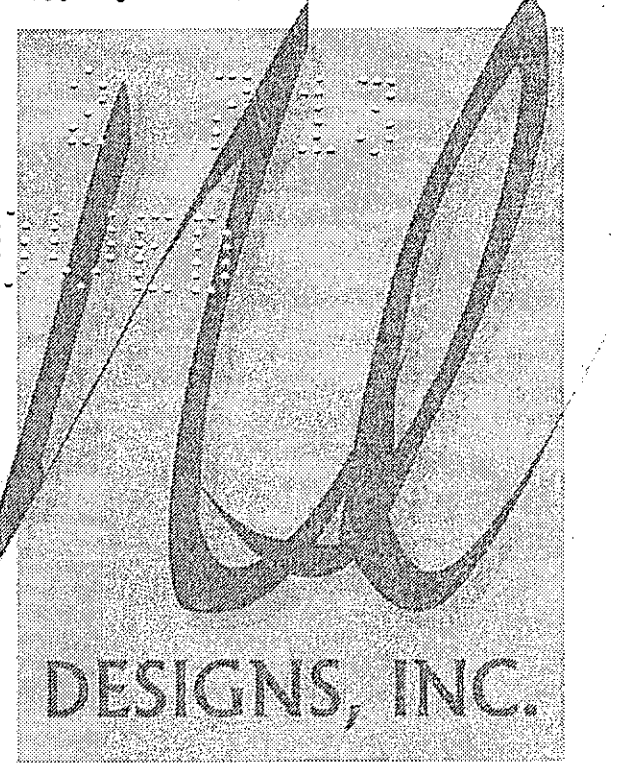
BY: *[Signature]* DATE: *FEB. 7 2013*
 Permit No. *7290 - 00006* - *02763*
 Application No. *13030* *2000* *20601*

This set of plans **MUST** be at the job site during construction.
 It is unlawful to alter, change, or deviate from these plans.
 The stamping of this plan **SHALL NOT** be held to permit or to be an approval of the violation of any provisions of any Ordinance or Law.
 SEPARATE permits are required for BUILDING, ELECTRICAL, PLUMBING, FIRE SPRINKLER, ELEVATOR, HEATING or REFRIGERATION work, unless this permit was issued as a combination permit for a One or Two Family Dwelling pursuant to LAMC Section 91.4107.2.2, and the work by that trade does not require Plan Check.

SHEET INDEX

ARCHITECTURAL

- A0.0 COVER SHEET
- A1.0 GREEN BUILDING FORMS
- A1.1 GREEN NOTES
- A1.2 GENERAL NOTES
- A2.0 SITE PLAN
- A2.1 MANSIONIZATION CALCULATION
- A2.2 VOLUME CALCULATION
- A3.0 FIRST FLOOR PLAN
- A3.1 SECOND FLOOR PLAN
- A3.2 FIRST FLOOR LIGHTING PLAN
- A3.3 SECOND FLOOR LIGHTING PLAN
- A3.4 FIRST FLOOR CEILING PLAN
- A3.5 SECOND FLOOR CEILING PLAN
- A4.0 ROOF PLAN
- A5.0 SECTION
- A6.0 ELEVATIONS
- A6.1 ELEVATIONS
- A7.0 DOORS & WINDOW SCHEDULE
- AD-1 ARCHITECTURAL DETAILS
- AD-2 ARCHITECTURAL DETAILS
- AD-3 ARCHITECTURAL DETAILS



RESIDENTIAL, COMMERCIAL & INTERIOR DESIGN SERVICES

NATHALIE GISPAN 818.915.4119
 ERAN GISPAN 818.915.4118

OFFICE 818.789.6439
 FAX 818.789.6941

www.NEDESIGNSINC.com

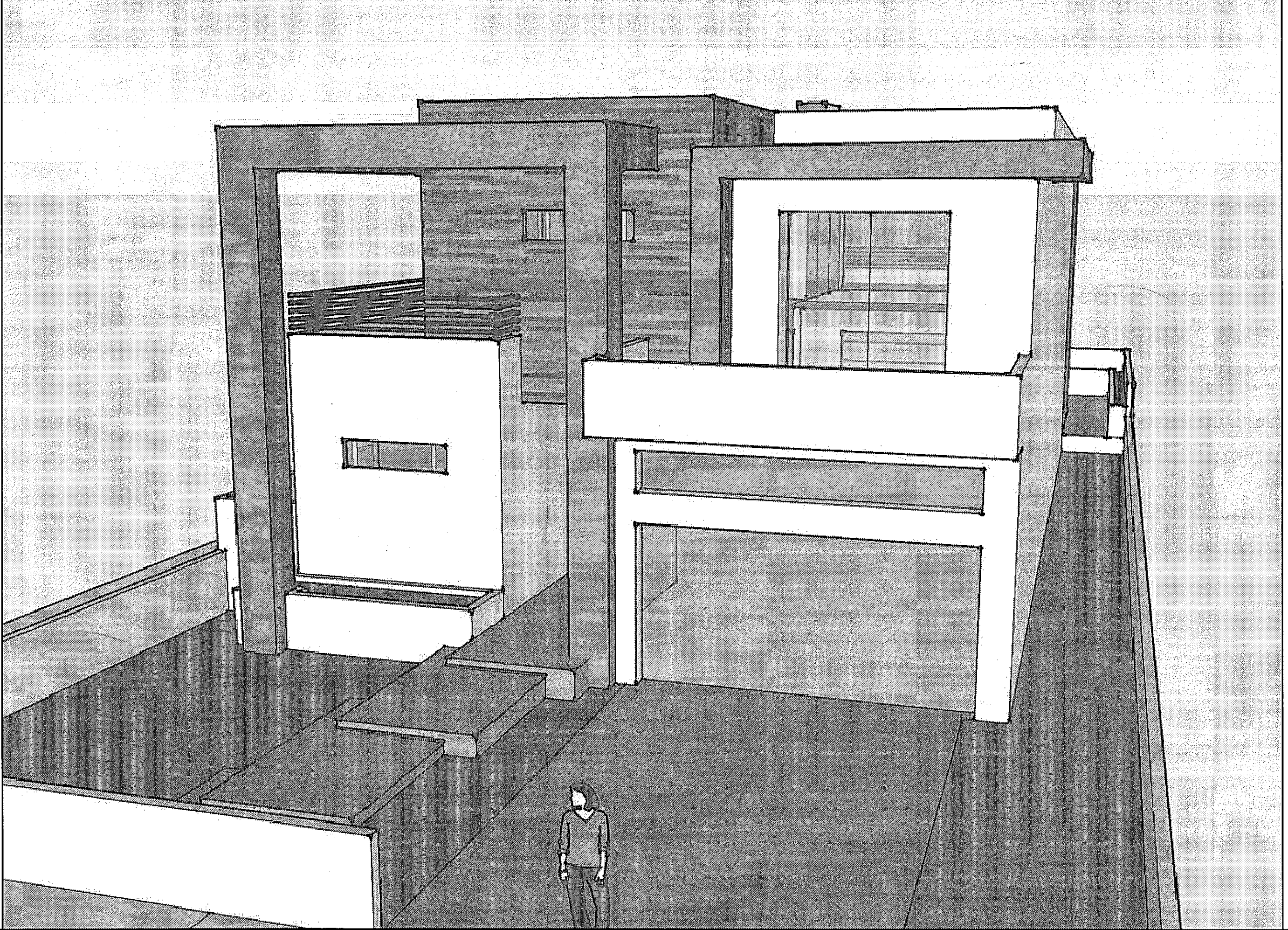
BUILDING ANALYSIS

CONSTRUCTION : TYPE VB
 OCCUPANCY GROUP: R-3 / U
 GOVERNING BUILDING CODE: LARC 2011
 LEGAL DESCRIPTION : LOT 26 BLOCK NONE
 TRACT: TR 3425
 LOT AREA: 6,601.8 SQ.FT.
 APN# : 5529023012

ZONING : R1-1
 STORY: 2 STORY
 BUILDING HEIGHT: 28'-0"
 BUILDING SETBACK: REAR: 15'-6"
 SIDE: 6'-0", 6'-0"
 FRONT: 25'-0"

FLOOR AREA	
1ST FLOOR	2167 SQ. FT.
SECOND FLOOR	2214 SQ. FT.
TOTAL :	4381 SQ. FT.
2 CAR GARAGE	407 SQ. FT.
PATIO	294 SQ. FT.

ZONING AREA	
1ST FLOOR	2012 SQ. FT.
SECOND FLOOR	1794 SQ. FT.
TOTAL :	3806 SQ. FT.
2 CAR GARAGE	381 SQ. FT.
PATIO	294 SQ. FT.



STRUCTURAL

- SD-1.1 STRUCTURAL OBSERVATION
- S-1.1 STRUCTURAL SPECIFICATIONS
- S-1.2 STRUCTURAL SPECIFICATIONS
- S-1.3A STRUCTURAL QUALITY ASSURANCE PLAN
- S-1.3B STRUCTURAL QUALITY ASSURANCE PLAN
- S-1.3C STRUCTURAL QUALITY ASSURANCE PLAN
- S-2.1 FOUNDATION DETAILS
- S-2.2 FOUNDATION DETAILS
- S-2.3 FOUNDATION DETAILS
- S-3.1 SHEAR WALL SCHEDULE
- S-3.2 SHEAR WALL DETAILS
- S-3.3 FRAMING DETAILS
- S-3.4 FRAMING DETAILS
- S-3.5 FRAMING DETAILS
- S-3.6 FRAMING DETAILS
- S-3.7 FRAMING DETAILS
- S-3.8 FRAMING DETAILS
- S-3.9 FRAMING DETAILS
- S-4.1 FOUNDATION PLAN
- S-5.1 SECOND FLOOR FRAMING PLAN
- S-5.2 ROOF FRAMING PLAN
- SSW1 SSW (STEEL STRONG WALL)

General Notes

RESTRICTIONS:
 THESE PLANS AND INCORPORATED DESIGNS EMBODIED THEREON ARE THE PROPERTY OF N.E. DESIGNS INC. THE USE OF THESE PLANS ARE RESTRICTED TO THE ORIGINAL SITE AND OWNER FOR WHICH THEY WERE PREPARED. PUBLICATION AND REPRODUCTION IS RESTRICTED TO SUCH USE. PUBLICATION AND REPRODUCTION BY ANY METHOD, IN WHOLE OR IN PART, IS PROHIBITED. TITLE TO THE PLANS AND DESIGNS REMAIN WITH N.E. DESIGNS INC. VISUAL CONTACT WITH THEM CONSTITUTE APPROVAL WITH THESE RESTRICTIONS.

No.	Revision/Issue	Date

Project
 901 N. LAUREL AVE.
 LOS ANGELES, CA 90046

NEW 2 STORY HOUSE

Drawing Title
 COVER SHEET

Project	12-553	Sheet	A0.0
Date	02-04-13		
Scale	N.T.S.		

SCOPE OF WORK

CONSULTANTS

VICINITY MAP

- * NEW 2 STORY HOUSE WITH ATTACHED GARAGE
- * BUILDING SHALL BE EQUIPED WITH AN AUTOMATIC RESIDENTIAL FIRE SPRINKLER IN ACCORDANCE WITH SECTION R313.3 OR NFPA13D. (R313, 12.21A17(D))

STRUCTURAL ENGINEER

HRD ENGINEERING
 7463 VARNA AVE.
 NORTH HOLLYWOOD, CA 91605
 (805) 431-5415

OWNER

LI INVESTMENTS LLC
 1180 BEVERLY DR.
 LOS ANGELES, CA 90035

City of Los Angeles
 Department of Building and Safety
 Green Building Division

This set of plans and specifications has been reviewed and is approved for compliance with state and local ordinances related to the Green Building Code.

This set of plans **MUST** be at the job site during construction.
 It is unlawful to alter, change, or deviate from these plans.

The stamping of this plan **SHALL NOT** be construed to be approval of a violation of any provisions of any Ordinance or Law.

By: *[Signature]* Total of 11 Sheets
 Date: *2/5/2013*

City of Los Angeles
 Department of Building and Safety
 Green Building Division

This set of plans and specifications has been reviewed and is approved for compliance with state and local ordinances related to the Green Building Code.

This set of plans **MUST** be at the job site during construction.
 It is unlawful to alter, change, or deviate from these plans.

The stamping of this plan **SHALL NOT** be construed to be approval of a violation of any provisions of any Ordinance or Law.

By: _____ Total of _____ Sheets
 Date: _____



STORM WATER POLLUTION CONTROL (2011 Los Angeles Green Building Code)

FORM GRN 1

Storm Water Pollution Control Requirements for Construction Activities Minimum Water Quality Protection Requirements for All Construction Projects

The following notes shall be incorporated in the approved set of construction/grading plans and represents the minimum standards of good housekeeping which must be implemented on all construction projects.

Construction means constructing, clearing, grading or excavation that result in soil disturbance. Construction includes structure teardown (demolition). It does not include routine maintenance to maintain original line and grade, hydraulic capacity, or original purpose of facility; emergency construction activities required to immediately protect public health and safety; interior remodeling with no outside exposure of construction material or construction waste to storm water; mechanical permit work; or sign permit work. (Order No. 01-182, NPDES Permit No. CAS004001 - Part 5: Definitions)

- 1. Eroded sediments and pollutants shall be retained on site and shall not be transported from the site via sheet flow, swales, area drains, natural drainage or wind.
2. Stockpiles of earth and other construction-related materials shall be covered and/or protected from being transported from the site by wind or water.
3. Fuels, oils, solvents and other toxic materials must be stored in accordance with their listing and shall not contaminate the soil nor the surface waters. All approved toxic storage containers are to be protected from the weather. Spills must be cleaned up immediately and disposed of properly and shall not be washed into the drainage system.
4. Non-storm water runoff from equipment and vehicle washing and any other activity shall be contained on the project site.
5. Excess or waste concrete may not be washed into the public way or any drainage system. Provisions shall be made to retain concrete waste on-site until it can be appropriately disposed of or recycled.
6. Trash and construction-related solid wastes must be deposited into a covered receptacle to prevent contamination of storm water and dispersal by wind.
7. Sediments and other materials shall not be tracked from the site by vehicle traffic. The construction entrance roadways must be stabilized so as to inhibit sediments from being deposited into the street/public ways. Accidental depositions must be swept up immediately and may not be washed down by rain or by any other means.
8. Retention basins of sufficient size shall be provided to retain storm water runoff on-site and shall be properly located to collect all tributary site runoff.
9. Where retention of storm water runoff on-site is not feasible due to site constraints, runoff may be conveyed to the street and the storm drain system provided that an approved filtering system is installed and maintained on-site during the construction duration.



VOC CONTENT VERIFICATION CHECKLIST (2011 Los Angeles Green Building Code)

FORM GRN 2

VOC content verification of paints, coatings, carpets, cushions, resilient flooring, adhesives, sealants, and caulks.

Attach product specification sheets and other supporting documents. Use additional sheets if necessary.

Table with 6 columns: Item #, Product Category, Product Manufacturer, Product Specification, VOC Content, Allowable VOC Content.



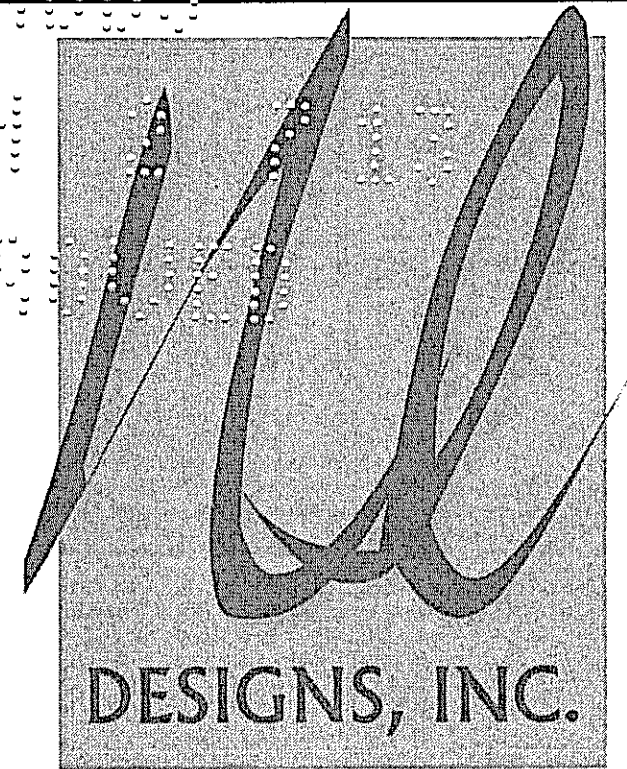
FORMALDEHYDE EMISSIONS VERIFICATION CHECKLIST (2011 Los Angeles Green Building Code)

FORM GRN 3

Formaldehyde emissions verification of non-structural engineered wood, hardwood plywood, particleboard, and medium density fiberboard composite wood.

Attach product specification sheets and other supporting documents. Use additional sheets if necessary.

Table with 6 columns: Item #, Product Category, Product Manufacturer, Product Specification, Formaldehyde Content, Formaldehyde Limits.



RESIDENTIAL, COMMERCIAL & INTERIOR DESIGN SERVICES

NATHALIE GISPAN 818.915.4119 ERAN GISPAN 818.915.4118

OFFICE 818.789.6439 FAX 818.789.6941

www.NEDESIGNSINC.COM

General Notes

RESTRICTIONS: THESE PLANS AND INCORPORATED DESIGNS EMBODIED THEREON ARE THE PROPERTY OF N.E. DESIGNS INC. THE USE OF THESE PLANS ARE RESTRICTED TO THE ORIGINAL SITE AND OWNER FOR WHICH THEY WERE PREPARED. PUBLICATION AND REPRODUCTION IS RESTRICTED TO SUCH USE. PUBLICATION AND REPRODUCTION BY ANY METHOD, IN WHOLE OR IN PART, IS PROHIBITED. TITLE TO THE PLANS AND DESIGNS REMAIN WITH N.E. DESIGNS INC. VISUAL CONTACT WITH THEM CONSTITUTE APPROVAL WITH THESE RESTRICTIONS.



2011 Los Angeles Green Building Code and 2012 CALGreen Supplement

FORM GRN 4

MANDATORY REQUIREMENTS CHECKLIST

NEWLY-CONSTRUCTED RESIDENTIAL BUILDINGS OF SIX STORIES OR LESS

(INCORPORATE THIS FORM INTO THE PLANS)

Project Address: 901 N. Laurel Ave Date: 1/21/12

Table with 5 columns: ITEM #, CODE SECTION, REQUIREMENT, REFERENCE SHEET, COMMENTS.



2011 Los Angeles Green Building Code and 2012 CALGreen Supplement

FORM GRN 4

Table with 5 columns: ITEM #, CODE SECTION, REQUIREMENT, REFERENCE SHEET, COMMENTS.



VOC AND FORMALDEHYDE LIMITS (2011 Los Angeles Green Building Code)

FORM GRN 11

The tables below are taken from the 2011 Los Angeles Green Building Code Tables 4.504.1, 4.504.2, 4.504.3, 4.504.4, 4.504.5, 5.504.4.1, 5.504.4.2, 5.504.4.3, 5.504.4.5, 9.504.1, 9.504.2, 9.504.3, 9.504.5, 10.504.4.1, 10.504.4.2, 10.504.4.3, 10.504.4.5

TABLE 4.504.3 VOC CONTENT LIMITS FOR ARCHITECTURAL COATINGS

Table with 4 columns: COATING CATEGORY, EFFECTIVE 1/1/2010, EFFECTIVE 1/1/2012, COMMENTS.

Values in this table are derived from those specified by the California Air Resources Board, Architectural Coatings Suggested Control Measure, February 1, 2008. More information is available from the Air Resources Board.

TABLE 4.504.1 Less Water and Less Exempt Compounds in Grams per Liter

Table with 2 columns: ARCHITECTURAL APPLICATIONS, CURRENT VOC LIMIT.

TABLE 4.504.2 SEALANT VOC LIMIT

Table with 2 columns: SEALANTS, CURRENT VOC LIMIT.

TABLE 4.504.5 FORMALDEHYDE LIMITS

Table with 3 columns: PRODUCT, CURRENT LIMIT, JULY 1, 2012.

Values in this table are derived from those specified by the California Air Resources Board, Air Toxics Control Measure for Composite Wood as tested in accordance with ASTM E 1333 (9/2003). For additional information, see California Code of Regulations, Title 17, Sections 93120 through 93121.2.

City of Los Angeles Department of Building and Safety Green Building Division

This set of plans and specifications has been reviewed and is approved for compliance with state and local ordinances related to the Green Building Code.

This set of plans MUST be at the job site during construction. It is unlawful to alter, change, or deviate from these plans.

The stamping of this plan SHALL NOT be construed to be approval or violation of any provisions of any Ordinance or Law.

By: [Signature] Total of [] Sheets Date: 02-04-13

Project 901 N. LAUREL AVE. LOS ANGELES, CA 90046

NEW 2 STORY HOUSE

Drawing Title GREEN FORMS

Project 12-553

Date 02-04-13

Scale 1/4"=1'-0"

Sheet A1.0

1. Removed
2. Removed
3. Each new appliance provided and installed meets ENERGY STAR if an ENERGY STAR designation is applicable for that appliance. (4.210, 9.210)
4. The flow rates for all plumbing fixtures shall comply with the minimum flow rates in Table 4.303.2/ Table 9.303.2. (4.303.1, 9.303.1)
5. When single shower fixtures are served by more than one showerhead, the combined flow rate of all the showerheads shall not exceed the maximum flow rates specified in the 20 percent reduction column contained in Table 4.303.2/ Table 9.303.2 or the shower shall be designed to only allow one showerhead to be in operation at a time. (4.303.2, 9.303.2)
6. Installed automatic irrigation system controllers shall be weather or soil-based controllers. (4.304.1, 9.304.1)
7. Annular spaces around pipes, electric cables, conduits, or other openings in the building's envelope at exterior walls shall be protected against the passage of rodents by closing such openings with cement mortar, concrete masonry, or metal plates. (4.406.1, 9.406.1)
8. Materials delivered to the construction site shall be protected from rain or other sources of moisture. (4.407.4, 9.407.4)
9. Only a City of Los Angeles certified hauler will be used for hauling of construction waste. (4.408, 9.408)
10. For all new equipment, an Operation and Maintenance Manual including, at a minimum, the items listed in Section 4.410.1, shall be completed and placed in the building at the time of final inspection. (4.410, 9.410)
11. All duct and other related air distribution component openings shall be covered with tape, plastic, or sheet metal until the final startup of the heating and cooling equipment. (4.504.1, 9.504.1)
12. Architectural paints and coatings, adhesives, caulks and sealants shall comply with the Volatile Organic Compound (VOC) limits. (4.504.1-4.504.4, 9.504.1-9.504.4)
13. The VOC Content Verification Checklist, Form GRN 2, shall be completed and verified prior to final inspection approval. The manufacturer's specifications showing VOC content for all applicable products shall be readily available at the job site and be provided to the field inspector for verification. (4.504.5, 9.504.5)
14. All new carpet installed in the building interior shall meet the testing and product requirements of one of the following:
 - a. Carpet and Rug Institute's Green Label Plus Program
 - b. California Department of Public Health Standard Practice for the testing of VOCs (Specification 01350)
 - c. NSF/ANSI 140 at the Gold level
 - d. Scientific Certifications Systems Indoor Advantage Gold (4.504.3, 9.504.3)

15. All new carpet cushion installed in the building interior shall meet the requirements of the Carpet and Rug Institute Green Label program. (4.504.3.1, 9.504.3.1)
16. 50% of the total area receiving new resilient flooring shall comply with the VOC limits or be certified under the Resilient Floor Covering Institute (RFCI) Floor Score program. (4.504.4, 9.504.4)
17. New hardwood, plywood, particle board, and medium density fiberboard composite wood products used in the interior or exterior of the building shall meet the formaldehyde limits listed in Table 4.504.5/ Table 9.504.5. (4.504.5, 9.504.5)
18. The Formaldehyde Emissions Verification Checklist, Form GRN 3, shall be completed prior to final inspection approval. The manufacturer's specifications showing formaldehyde content for all applicable wood products shall be readily available at the job site and be provided to the field inspector for verification. (4.504.5.1, 9.504.5.1)
19. Building materials with visible signs of water damage shall not be installed. Wall and floor framing shall not be enclosed until it is inspected and found to be satisfactory by the building inspector. (4.505.3, 9.505.3)
20. Newly installed bathroom exhaust fans shall be ENERGY STAR compliant and be ducted to terminate to the outside of the building. Provide the manufacturer's cut sheet for verification. (4.506.1, 9.506.1)
21. Bathroom exhaust fans, not functioning as a component of a whole house ventilation system, must be controlled by a humidistat which shall be readily accessible. (4.506.1, 9.506.1)
22. Whole house exhaust fans shall have covers or louvers which close when the fan is off and that are insulated with a minimum insulation value of R-4.2. (4.507.1)
23. The size and layout of the heating and air-conditioning systems shall be in accordance with ACCA Manual J, ACCA 29-D and ACCA 36-S, ASHRAE handbooks. (4.507.2, 9.507.2)
24. An operation and maintenance manual including, at a minimum, the items listed in section 9.410.1, shall be completed and placed in the building at the time of final inspection. (9.410.1)
25. A 4-inch thick base of 1/2 inch or larger clean aggregate shall be provided for the proposed slab on grade construction. AD-2 Detail #1 (4.505.2.1)
26. A vapor barrier shall be provided in direct contact with concrete for the proposed slab on grade construction. (4.505.2.1)
27. Construction waste shall be reduced by 50%
28. An Operation and Maintenance Manual including, at a minimum, the items listed in Section 4.410.1, shall be completed and placed in the building at the time of final inspection. (4.410.1)



PLUMBING FIXTURE FLOW RATES
Residential Occupancies
2011 Los Angeles Green Building Code
(incorporate this form into the plans)

FORM GRN 16

**TABLE 4.303.2
FIXTURE FLOW RATES**

FIXTURE TYPE	MAXIMUM FLOW RATE AT > 20 percent REDUCTION
Showerheads	2 gpm @ 80 psi
Lavatory faucets, residential	1.5 gpm @ 60 psi ¹
Lavatory Faucets, nonresidential	0.4 gpm @ 60 psi ²
Kitchen faucets	1.8 gpm @ 60 psi ³
Gravity tank type water closets	1.28 gallons/flush ⁴
Flushometer tank water closets	1.28 gallons/flush ⁴
Flushometer valve water closets	1.28 gallons/flush ⁴
Urinals	0.125 gallons/flush

**TABLE 9.303.2
FIXTURE FLOW RATES**

FIXTURE TYPE	MAXIMUM ALLOWABLE FLOW RATE
Showerheads	2 gpm @ 80 psi
Lavatory faucets residential	1.5 gpm @ 60 psi
Gravity tank type water closets	1.28 gallons/flush ¹
Flushometer tank water closets	1.28 gallons/flush ¹
Flushometer valve water closets	1.28 gallons/flush ¹
Urinals	0.125 gallons/flush

¹ Lavatory Faucets shall not have a flow rate less than 0.8 gpm at 20 psi.
² Kitchen faucets may temporarily increase flow above the maximum rate, but not above 2.2 gpm @ 60psi and must default to a maximum flow rate of 1.8 gpm @ 60psi.
³ Where complying faucets are unavailable, aerators rated at .35 gpm or other means may be used to achieve reduction.
⁴ Includes single and dual flush water closets with an effective flush of 1.28 gallons or less.
 Single Flush Toilets - The effective flush volume shall not exceed 1.28 gallons (4.8 liters). The effective flush volume is the average flush volume when tested in accordance with ASME A112.19.2.33.2.
 Dual Flush Toilets - The effective flush volume shall not exceed 1.28 gallons (4.8 liters). The effective flush volume is defined as the composite, average flush volume of two reduced flushes and one full flush. Flush volumes will be tested in accordance with ASME A112.19.2 and ASME A112.19.14.



RESIDENTIAL, COMMERCIAL & INTERIOR DESIGN SERVICES

NATHALIE GISPAN 818.915.4119
ERAN GISPAN 818.915.4118

OFFICE 818.789.6439
FAX 818.789.6941

www.NEDESIGNSINC.com

General Notes

RESTRICTIONS:
THESE PLANS AND INCORPORATED DESIGNS EMBODIED THEREON ARE THE PROPERTY OF N.E. DESIGNS INC. THE USE OF THESE PLANS ARE RESTRICTED TO THE ORIGINAL SITE AND OWNER FOR WHICH THEY WERE PREPARED. PUBLICATION AND REPRODUCTION IS RESTRICTED TO SUCH USE. PUBLICATION AND REPRODUCTION BY ANY METHOD, IN WHOLE OR IN PART, IS PROHIBITED. TITLE TO THE PLANS AND DESIGNS REMAIN WITH N.E. DESIGNS INC. VISUAL CONTACT WITH THEM CONSTITUTE APPROVAL WITH THESE RESTRICTIONS.

No.	Revision/Issue	Date

Project
901 N. LAUREL AVE.
LOS ANGELES, CA 90046

NEW 2 STORY HOUSE

Drawing Title
GREEN NOTES

Project	12-553	Sheet	A1.1
Date	02-04-13		
Scale	1/4"=1'-0"		

City of Los Angeles
Department of Building and Safety
Green Building Division

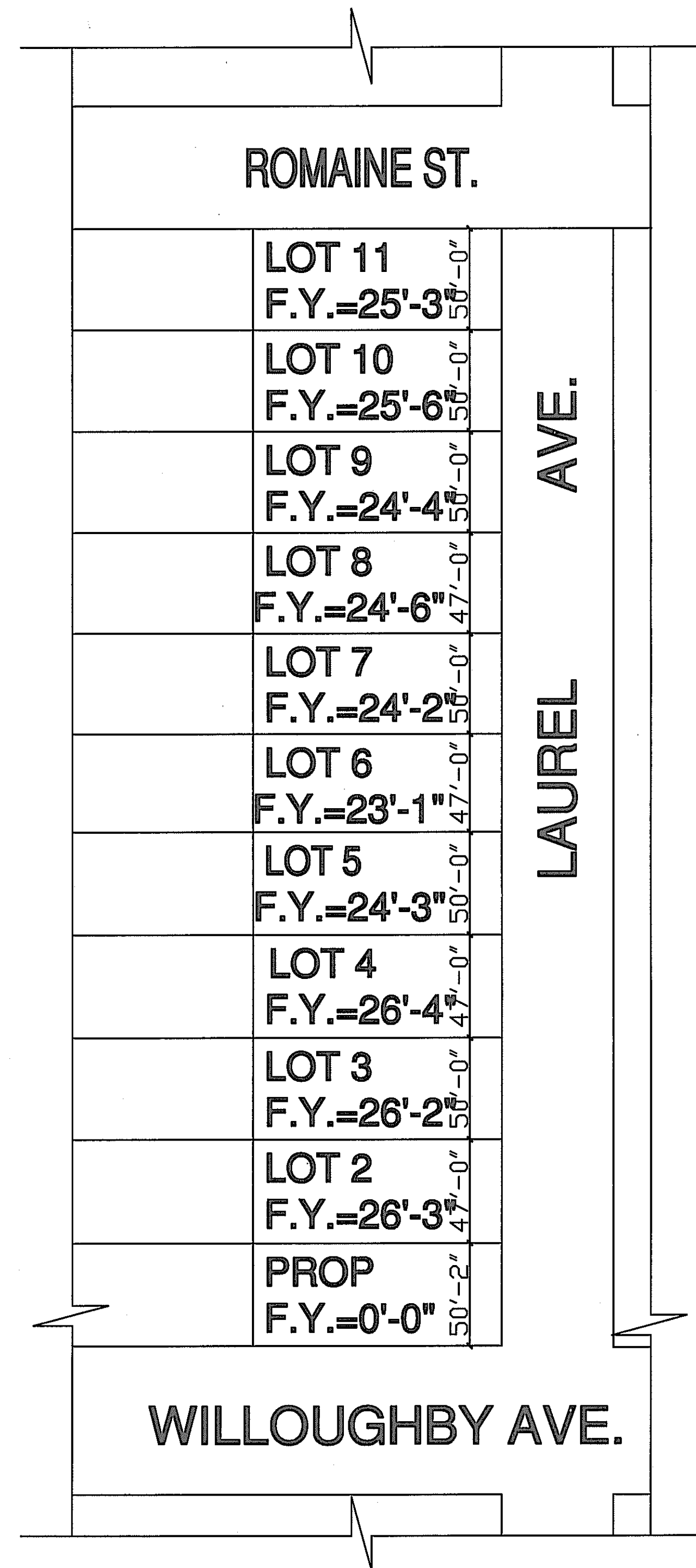
This set of plans and specifications has been reviewed and is approved for compliance with state and local ordinances related to the Green Building Code.

This set of plans MUST be at the job site during construction.

It is unlawful to alter, change, or deviate from these plans.

The stamping of this plan SHALL NOT be construed to be approval of or violation of any provisions of any Ordinance or Law.

By: Total of _____ Sheets
Date: _____



Untitled Document Page 1 of 1

Prevailing Setback Calculator
City of Los Angeles, Department of Building and Safety

LA DBS
DEPARTMENT OF BUILDING AND SAFETY

Refresh
ladbs home
Inches to Feet Conversion Table
Important Note
Zoning Code
Prevailing Setback Information Bulletin
Help

Calculate

No	Lot	Frontage	Setback	No	Lot	Frontage	Setback	No	Lot	Frontage	Setback
1	1	50.16	0	2	2	47.00	26.25	3	3	50.00	26.16
4	4	47.00	26.33	5	5	50.00	24.25	6	6	47.00	23.08
7	7	50.00	24.16	8	8	47.00	24.5	9	9	50.00	24.33
10	10	50.00	25.50	11	11	50.00	25.25	12			
13				14				15			
16				17				18			
19				20				21			
22				23				24			
25				26				27			
28				29				30			
31				32				33			
34				35				36			
37				38				39			
40				41				42			
43				44				45			

Prevailing Setback=24.98 ft

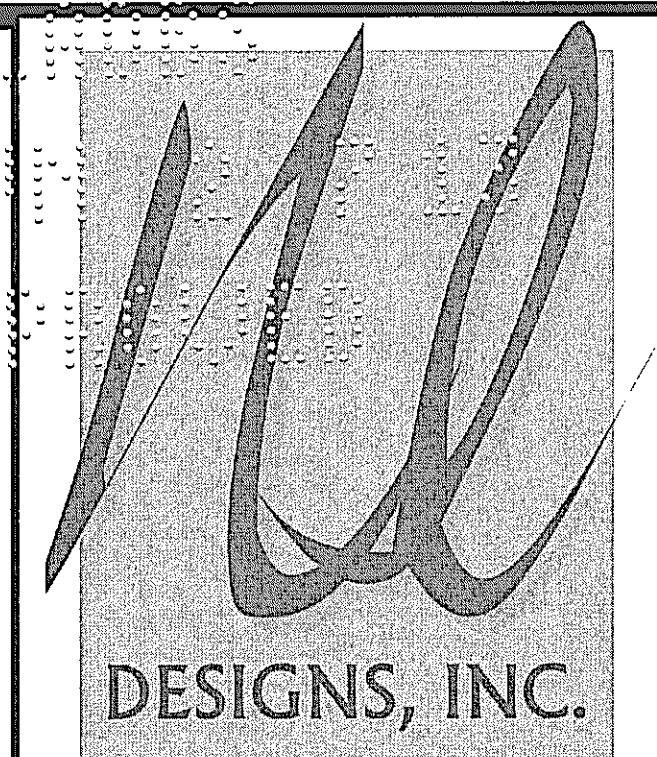
Total no of lots entered=11
Total frontage entered=538.00 ft
40% of Total frontage entered=215.20 ft

No of lots used in the calculations=10
Setback range used=23.08 - 33.08 ft
Total frontage used in the calculations=488 ft
No of trials=2

Lots Used

Lot	Frontage	Setback
2	47.00	26.25
3	50.00	26.16
4	47.00	26.33
5	50.00	24.25
6	47.00	23.08
7	50.00	24.16
8	47.00	24.5
9	50.00	24.33
10	50.00	25.50
11	50.00	25.25

View Calculations Detail



RESIDENTIAL, COMMERCIAL & INTERIOR DESIGN SERVICES

NATHALIE GISPAN 818.915.4119
ERAN GISPAN 818.915.4118

OFFICE 818.789.6439
FAX 818.789.6941

www.NEDESIGNSINC.com

General Notes

RESTRICTIONS:
THESE PLANS AND INCORPORATED DESIGNS EMBODIED THEREON ARE THE PROPERTY OF N.E. DESIGNS INC. THE USE OF THESE PLANS ARE RESTRICTED TO THE ORIGINAL SITE AND OWNER FOR WHICH THEY WERE PREPARED. PUBLICATION AND REPRODUCTION IS RESTRICTED TO SUCH USE. PUBLICATION AND REPRODUCTION BY ANY METHOD, IN WHOLE OR IN PART, IS PROHIBITED. TITLE TO THE PLANS AND DESIGNS REMAIN WITH N.E. DESIGNS INC. VISUAL CONTACT WITH THEM CONSTITUTE APPROVAL WITH THESE RESTRICTIONS.

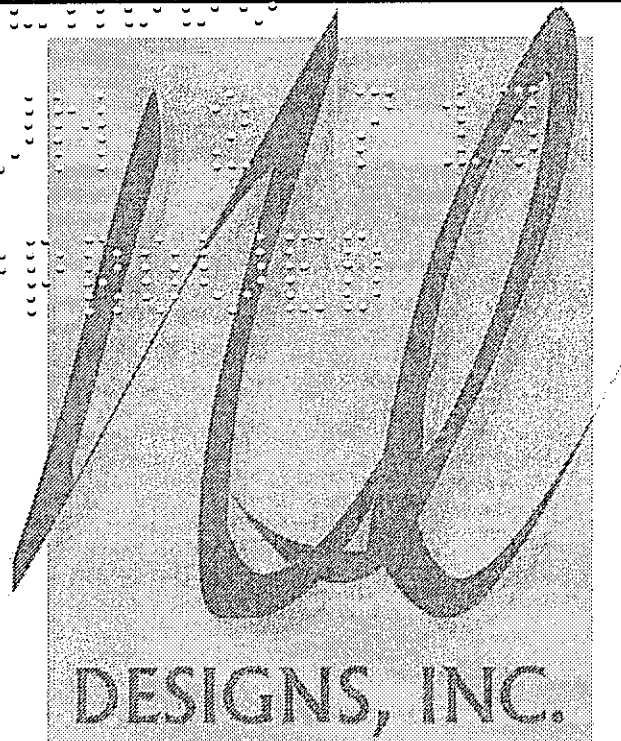
No.	Revision/Issue	Date

Project
901 N. LAUREL AVE.
LOS ANGELES, CA 90046

NEW 2 STORY HOUSE

Drawing Title
PREVAILING SETBACK CALC.

Project 12-553	Sheet
Date 02-04-13	A2.3
Scale N.T.S.	

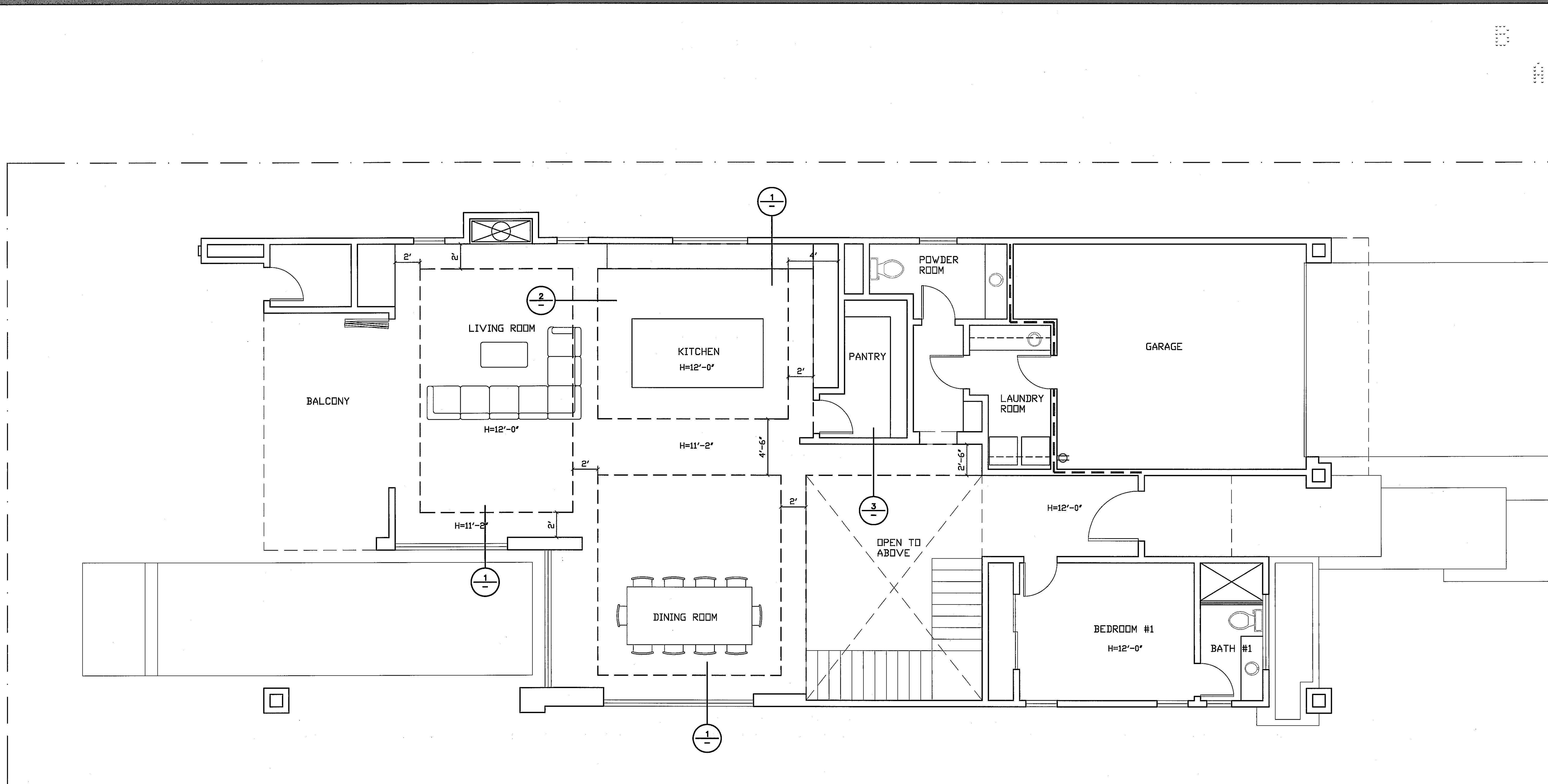


RESIDENTIAL, COMMERCIAL
& INTERIOR DESIGN SERVICES

NATHALIE GISPAN 818.915.4119
ERAN GISPAN 818.915.4118

OFFICE 818.789.6439
FAX 818.789.6941

www.NEDESIGNSINC.COM



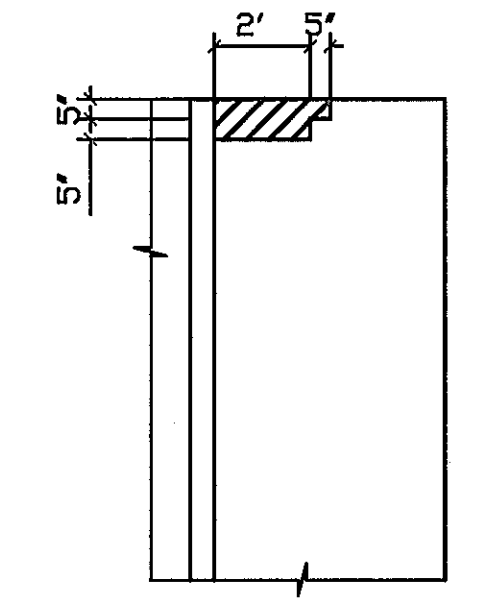
General Notes

RESTRICTIONS:
THESE PLANS AND INCORPORATED
DESIGNS EMBODIED THEREON ARE
THE PROPERTY OF N.E. DESIGNS INC.
THE USE OF THESE PLANS ARE
RESTRICTED TO THE ORIGINAL SITE
AND OWNER FOR WHICH THEY WERE
PREPARED. PUBLICATION AND
REPRODUCTION IS RESTRICTED TO
SUCH USE. PUBLICATION AND
REPRODUCTION BY ANY METHOD, IN
WHOLE OR IN PART, IS PROHIBITED.
TITLE TO THE PLANS AND DESIGNS
REMAIN WITH N.E. DESIGNS INC.
VISUAL CONTACT WITH THEM
CONSTITUTE APPROVAL WITH THESE
RESTRICTIONS.

No.	Revision/Issue	Date

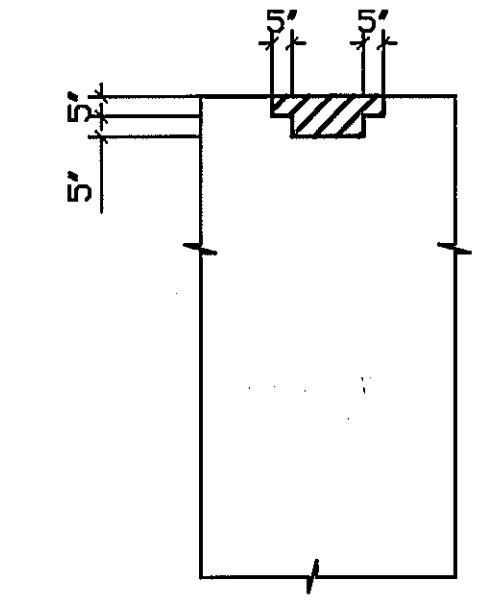
FIRST FLOOR CEILING PLAN

1/4" = 1'-0"



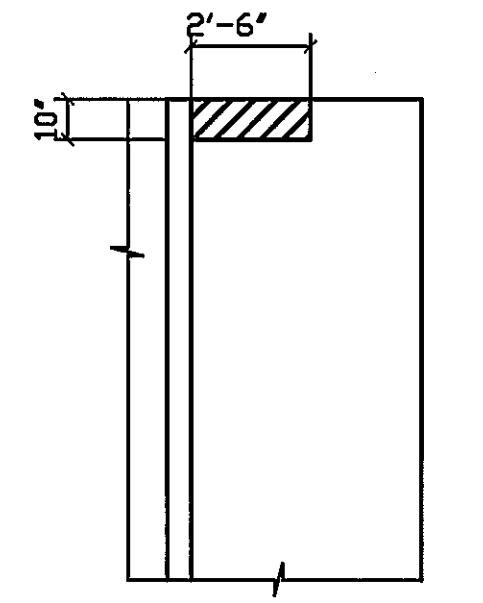
CEILING DETAIL 1

1/4" = 1'-0"



CEILING DETAIL 2

1/4" = 1'-0"



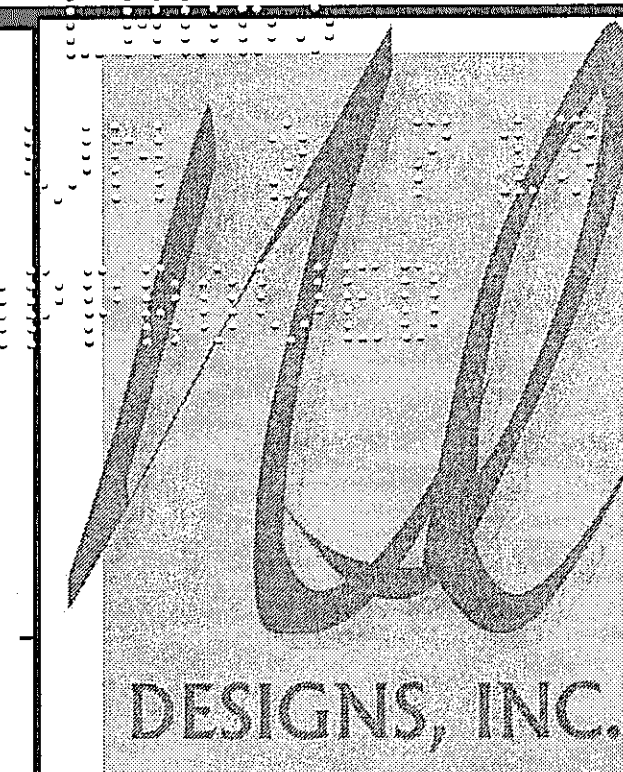
CEILING DETAIL 3

1/4" = 1'-0"

Project
901 N. LAUREL AVE.
LOS ANGELES, CA 90046

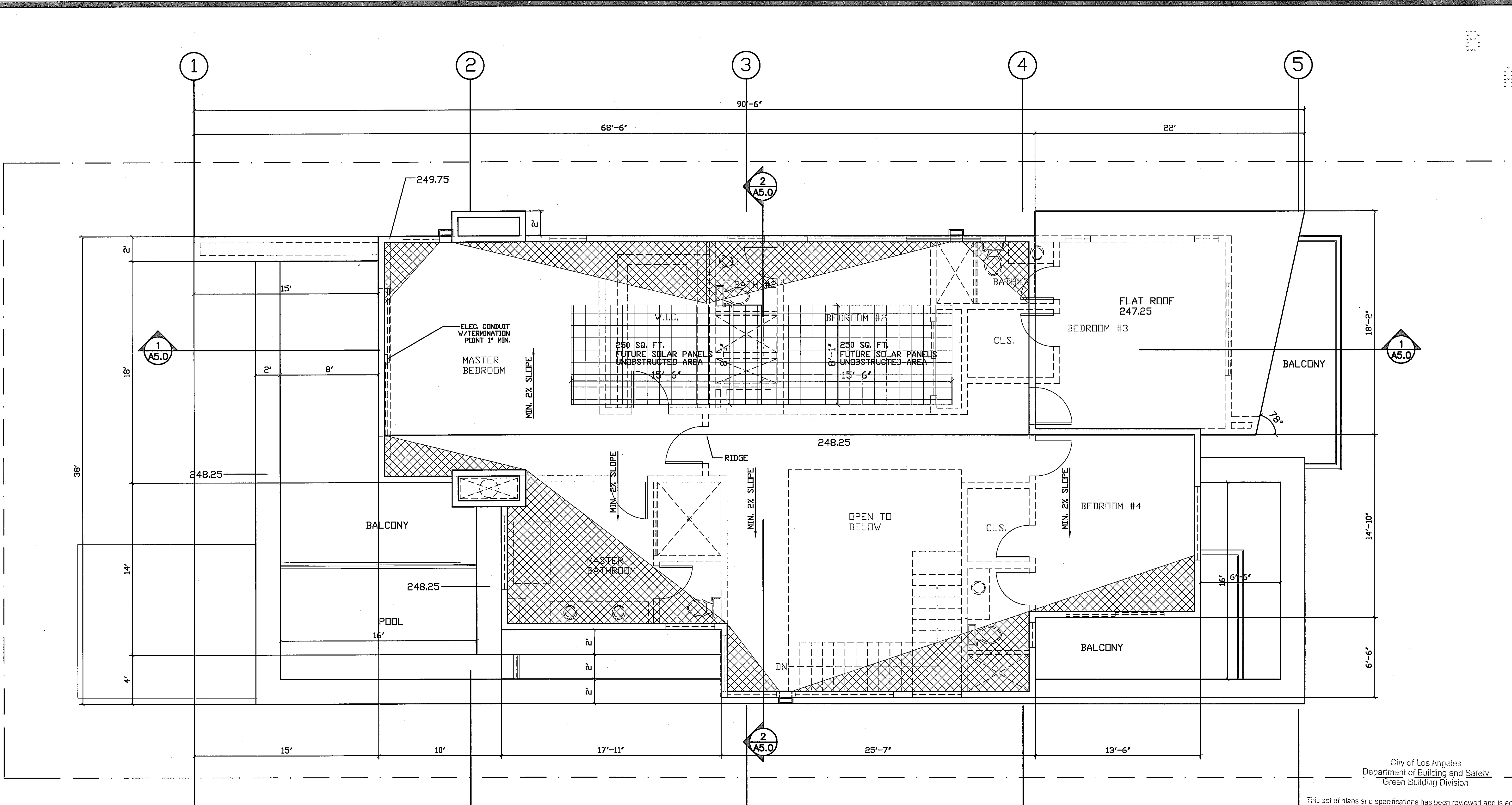
Drawing Title
FIRST FLOOR REFLECTED
CEILING PLAN

Project	12-553	Sheet	A3.4
Date	02-04-13	Scale	
Scale	1/4"=1'-0"		



RESIDENTIAL, COMMERCIAL & INTERIOR DESIGN SERVICES
 NATHALIE GISPAN 818.915.4119
 ERAN GISPAN 818.915.4118
 OFFICE 818.789.6439
 FAX 818.789.6941
 www.NEDESIGNSINC.COM

General Notes
 RESTRICTIONS:
 THESE PLANS AND INCORPORATED DESIGNS EMBODIED THEREON ARE THE PROPERTY OF N.E. DESIGNS INC. THE USE OF THESE PLANS ARE RESTRICTED TO THE ORIGINAL SITE AND OWNER FOR WHICH THEY WERE PREPARED. PUBLICATION AND REPRODUCTION IS RESTRICTED TO SUCH USE. PUBLICATION AND REPRODUCTION BY ANY METHOD, IN WHOLE OR IN PART, IS PROHIBITED. TITLE TO THE PLANS AND DESIGNS REMAIN WITH N.E. DESIGNS INC. VISUAL CONTACT WITH THEM CONSTITUTE APPROVAL WITH THESE RESTRICTIONS.



ROOF PLAN

1/4" = 1'-0"

City of Los Angeles
 Department of Building and Safety
 Green Building Division

This set of plans and specifications has been reviewed and is approved for compliance with state and local ordinances related to the Green Building Code.

This set of plans MUST be at the job site during construction. It is unlawful to alter, change, or deviate from these plans.

The stamping of this plan SHALL NOT be construed to be approval of a violation of any provisions of any Ordinance or Law.

By: *[Signature]* Total of _____ Sheets
 Date: _____

Revision/Issue	Date

Project
**901 N. LAUREL AVE.
 LOS ANGELES, CA 90046**

Drawing Title
ROOF PLAN

Project	12-553	Sheet	A4.0
Date	02-04-13		
Scale	1/4"=1'-0"		

- Note :**
- MINIMUM ROOF SLOPE OF 3/8" PER FOOT SHALL BE PROVIDED AT ANY POINT OF THE ROOF. PROVIDE UNIFORM SLOPE TOWARD DOWNSPOUTS AND/OR SCUPPERS AS INDICATED ON ROOF PLAN. (REFER TO DETAILS FOR DOWNSPOUTS AND SCUPPERS INCLUDED IN THIS DRAWINGS.)
 - INSULATIONS SHALL BE INSTALLED AT BOTTOM OF JOISTS OR FRAMING UNDER ENTIRE ROOF. USE R-30 INSULATION WITH VAPOR BARRIER.
 - CHECK & VERIFY LOCATIONS & EXACT SIZES OF ALL A/C EQUIPMENT, REQUIRED OPENINGS & SUPPORTS WITH A/C CONTRACTORS BEFORE FRAMING BEGINS. ANY REMWORKING REQUIRED TO ACCOMMODATE EQUIPMENT IN QUESTION SHALL BE DONE AT NO ADDITIONAL COST TO THE OWNER.
 - PAINT ALL ROOF EQUIPMENTS. ALL ROOF METAL SHALL RECEIVE TWO (2) COATS OF EXTERIOR SEMI GLOSS PAINT. VERIFY COLOR W/ARCHITECT.
 - TELEVISION ANTENNA CROSSARMS OR OTHER ROOF OBSTRUCTIONS SHALL BE LOCATED 20'-0" MIN. FROM EDGE OF ROOF & 7'-0" MIN. ABOVE ROOF.
 - PROVIDE ATTIC DRAFT STOPS AS REQUIRED (SEE ROOF PLAN & DETAILS). DRAFT STOPS SHALL DIVIDE ATTIC SPACES INTO AREAS NOT TO EXCEED 3,000 SQ. FT. PER UBC SEC.3205 (8). SEE DETAILS FOR WOOD & GYP. BOARD DRAFT STOPS. PROVIDE 2" X _____ WOOD FRAMING AS REQUIRED TO COMPLETE ITS CONSTRUCTION.
 - PROVIDE AN APPROVED SPARK ARRESTER FOR THE CHIMNEY OF A FIREPLACE, STOVE OR BARBECUE.
 - PROVIDE GRAVITY TYPE ATTIC VENTILATORS AT ROOF IF TRUSSES ARE NOT OF SOLID WEB CONSTRUCTION. WHEN ATTIC DRAFT STOP ARE REQUIRED PROVIDE TWO (2) ROOF GRAVITY VENTILATORS FOR EACH DIVISION.
 - CONTRACTOR TO CONFIRM THAT ALL AREAS ARE PROVIDED WITH POSITIVE DRAINAGE PRIOR TO SHEATHING THE ROOF.
 - CRICKET WITH 1/2" PLYWOOD OVER 2" X _____ SLEEPERS TYPICAL.
 - THE ROOFS FOR THE MAIN BUILDING AND THE EXISTING BUILDING ARE TO BE INSPECTED BY A QUALIFIED ROOFING SPECIALIST TO VERIFY THAT:
 - ALL EXISTING ROOFING MATERIALS AND INSTALLATION METHODS HAVE BEEN PROPERLY INSTALLED AND MAINTAINED.
 - ASSOCIATED FLASHINGS AN SHEET METAL HAVE BEEN PROPERLY INSTALLED AND MAINTAINED.
 - THE ROOFS ARE TO BE FLOOD TESTED IN A MANNER APPROPRIATE TO THE TYPE EXISTING ROOFING SEE NOTE 12 BELOW.
 - ANY DEFICIENCIES OR LEAK CONDITIONS ARE TO BE CORRECTED TO INSURE A WATERTIGHT INSTALLATION FOR A PERIOD OF ONE YEAR FROM THE TIME OF PROJECT ACCEPTANCE BY THE TENNANT.
 - ROOF FLOOD TEST TO BE CONDUCTED IN THE PRESENCE OF THE BUILDING OWNER AND THE TENANT. CONTRACTOR TO PROVIDE MIN. 96 HRS. NOTICE TO BOTH PARTIES.
 - ATTIC NET VENTILATION AREA RATIO IS 1/150 OF ATTIC AREA ,VENTS WITH THE BALANCE OF THE REQUIRED VENTILATION PROVIDED BY THE EAVE VENTS. OPENINGS SHALL HAVE 1/4 INCH CORROSION RESISTANT METAL MESH COVERING.
 - * PROVIDE RAIN GUTTERS AND CONVEY RAIN WATER TO THE STREET.

SPECIAL HAZARDS

GLAZING IN HAZARDOUS LOCATIONS SHALL BE TEMPERED (2406.4)

- a. INGRESS AND EGRESS DOORS.
- b. PANELS IN SLIDING OR SWINGING DOORS
- c. DOORS AND ENCLOSURE FOR HOT TUB, BATHTUB, SHOWERS (ALSO GLAZING IN WALL ENCLOSING THESE COMPARTMENTS WITHIN 5' OF STANDING SURFACE)
- d. IF WITHIN 2' VERTICAL EDGE OF CLOSED DOOR AND WITHIN 5' OF STANDING SURFACE
- e. IN WALL ENCLOSING STAIRWAY LANDING.
- f. GLAZING 5'-0" FROM TOP OR BOTTOM OF STEPS WITH BOTTOM EDGE LESS THAN 60" ABOVE WALKING SURFACE.

U. SECURITY REQUIREMENTS

General:

1. All entry doors to dwelling units or guest rooms shall be arranged so that the occupant has a view of the area immediately outside the door without opening the door. Such view may be provided by a door viewer, through windows located in the vicinity of the door or through view ports in the door or adjoining wall. 91.6706

2. Screens, barricades, or fences made of material which preclude human climbing shall be provided at every portion of every roof, balcony, or similar surface which is within 8ft. of the utility pole or similar structures. 91.6707

DOORS:

3. Wood flush-type doors shall be 1 3/8" thick minimum with solid core construction. 91.6709.1 - Door stops of in-swinging doors shall be of one-piece construction with the jamb or joined by rabbit to the jamb. 91.6709.4

4. Every door in a security opening for an apartment house shall be provided with a light bulb (60 watt min.) At a maximum height of 8 feet on the exterior. 91.6708

5. All pin-type door hinges accessible from outside shall have min. 1/4" dia. Steel jamb stud with 1/4" min. protection. The strike plate for latches and holding device for projecting dead bolts in wood construction shall be secured to the jamb and the wall framing with screws no less than 2-1/2" long. 91.6709.5, 91.6709.7

6. Provided dead bolts with hardened inserts; deadlocking latch with key-operated locks on exterior. Locks must be open able from inside without key, special knowledge or special effort (latch not required in B,F, and S occupancies. 91.6709.2)

7. Straight dead bolts shall have a min. throw of 1" and an embedment of not less than 5/8", and a hook-shaped or an expanding-lug deadbolt shall have a minimum throw of 2". 91.6709.2

8. The use of a locking system which consists of a deadlocking latch operated by a doorknob and a deadbolt operated by a non-removable thumb turn which is independent of the deadlocking latch and which must be separately operated, shall not be considered a system which requires special knowledge or effort when used in dwelling units. The door knob and the thumb turn which operates the deadbolt shall not be separated by more than 8 inches.

9. Wood panel type doors must have panels at least 9/16 in. thick with shaped portions not less 1/4 in. thick and individual panels must be no more than 300 sq. in. in area. Mullions shall be considered a part of adjacent panels except mullions not over 18 inches long may have an overall width of not less than 2 inches. Stiles and rails shall be of solid lumber in thickness with overall dimensions of not less than 1 3/8 inches and 3 inches in width. 91.6709.1 item 2

10. Sliding doors shall be provided with device in the upper channel of the moving panel in the closed or partially open position. 91.6710.

11. Sliding doors shall be equipped with locking devices and shall be so constructed and installed that tests specified in 91.6717.1

12. Metal or wooden overhead or sliding doors shall be secured with a cylinder lock, padlock with a min. 9/32" diameter hardened steel shackle and bolted, hardened steel hasps, metal slide board, bolt or equivalent device unless secured electrically operated. 91.6711

13. Provide metal guides at top and bottom of metal accordion grate or grille-type doors and cylinder locks or padlocks. Cylinder projects beyond the face of the door or is otherwise accessible to gripping tools. 91.6712

GLAZING:

14. In B, F, M, and S occupancies, panes of glazing with at least one dimension greater than 5 in. but less than 48 in, shall be construction of tempered or approved burglary-resistant material or protected with metal bars or grilles. 91.6714?A

15. Glazed openings within 40" of the door lock when the door is in the closed position, shall be fully tempered glass or approved burglary resistant material, or shall be protected by metal bars, screens or grilles having a maximum opening of 2". The provisions of the section shall not apply to view ports or windows which do not exceed 2" in their greatest dimensions. 91.6713

16. Louvered windows shall be protected by metal bars or grilles with openings that have at least one dimension of 6" or less, which are constructed to preclude human entry. 91.6715.3

17. Other open able windows shall be provided with substantial locking devices. In B, F, M and S occupancies, such devices shall be glide bars, bolts cross-bars, and/or padlocks with minimum 9/32" hardened steel shackles and bolted, hardened steel hasps. 91.6715.2

18. Sliding windows shall be provided with a device in the upper channel of the moving panel to prohibit raising and removing of moving panel in the closed or partially open position. 91.6715.1

19. Sliding windows shall be equipped with locking device and shall be so constructed and installed that they remain intact and engaged when subjected to the tests specified in 91.6717.2

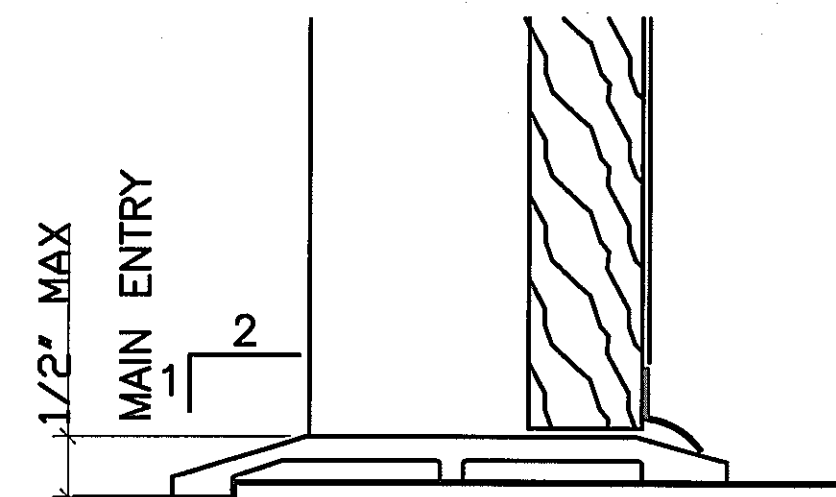
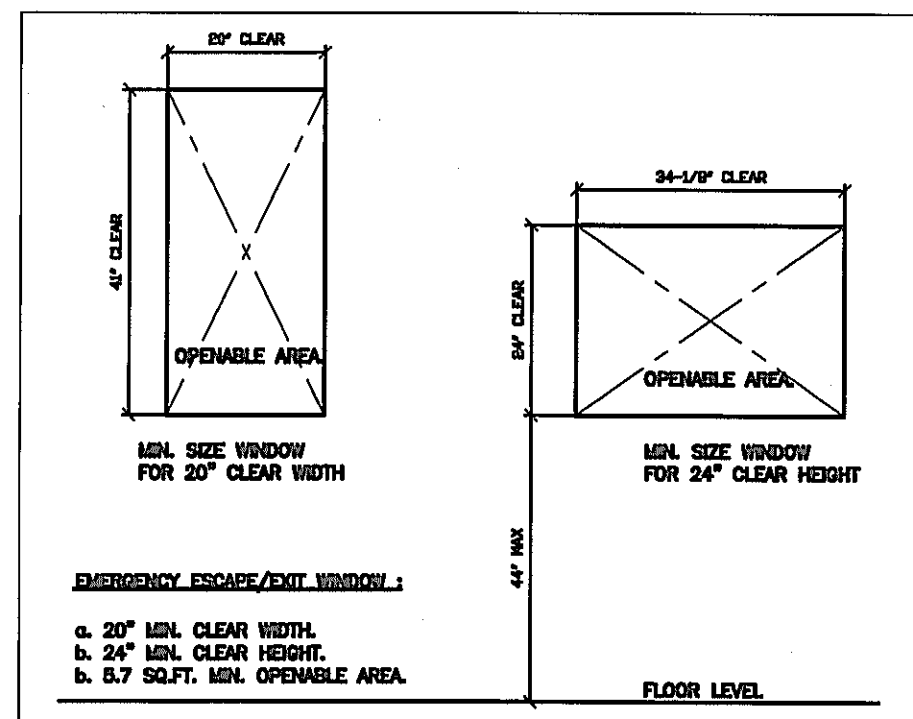
20. Any release for metal bars, grilles, grates or similar devices constructed to preclude human entry that are installed shall be located on the inside of the adjacent room and at least 24 inches from the closest opening through such metal bars, grilles, grates or similar devices that exceeds two inches in any dimension. 91.6715.4

OPENINGS OTHER THAN DOORS OR GLAZED OPENINGS:

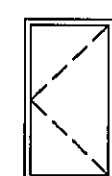
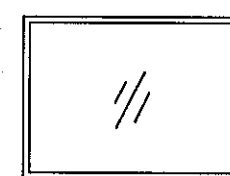
21. All other openings must be protected by metal bars or grilles with openings of not less than 6 inches in one dimension. 91.6716

23. THE CONSTRUCTION SHALL NOT RESTRICT A FIVE FOOT CLEAR AND UNOBSTRUCTED TO ANY WATER OR POWER DISTRIBUTION FACILITIES (POWER POLES, PULL BOXES, TRANSFORMERS, VAULTS, PUMPS, VALVES, METERS, APPURTENANCES, ETC) OR TO THE LOCATION OF THE HOOD-UP THE CONSTRUCTION SHALL NOT BE WITHIN TEN FEET OF ANY POWER LINES-WEHTER OR NOT THE LINES ARE LOCATED ON THE PROPERTY. FAILURE RO COMPLY MAY CAUSE COSNTRUCTION DELAYS AND/OR ADDITIONAL EXPENSES.

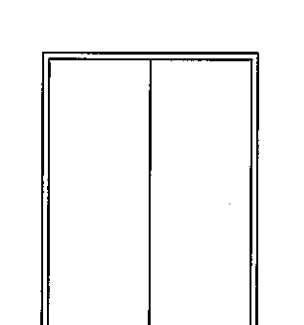
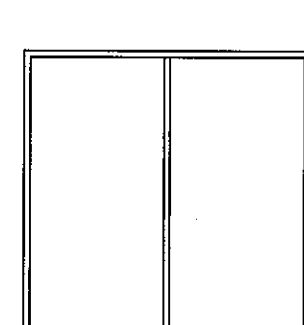
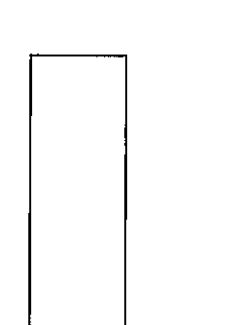
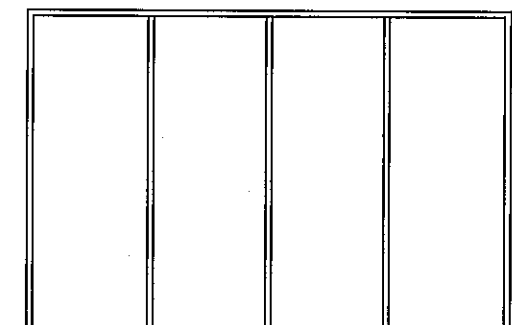
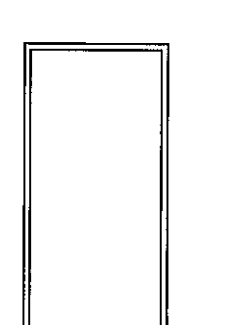
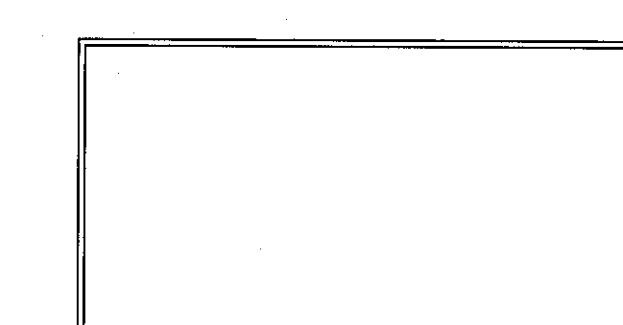
24. AN APPROVED SEISMIC GAS SHUTOFF VALVE WILL BE INSTALLED ON THE FUEL GAS LINE ON THE DOWN STREAM SIDE OF THE UTILITY METER AND BE RIFIDLY CONNECTED TO THE EXTERIOR OF THE BUILDING OR STRUCTURE CONTAINING THE FUEL GAS PIPING.



TRESHOLD AT ENTRY TYP.



TYPE A TYPE B



TYPE A TYPE B TYPE C TYPE D TYPE E TYPE F

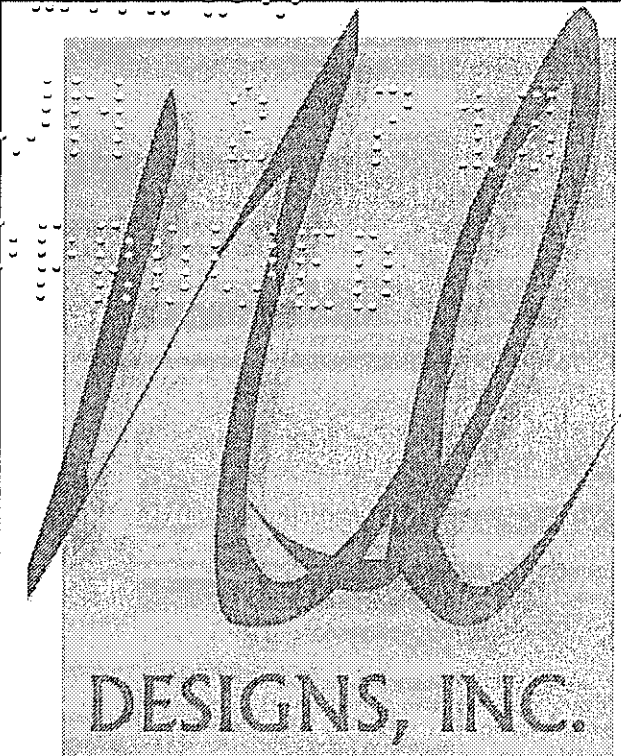
DOOR SCHEDULE

QTY	ID	WIDTH/HEIGHT	LOCATION	TYPE	THICK	FINISH	FRAME	REMARKS
1	D1	16'-0"x8'-0"	GARAGE	A				
1	D2	4'-0"x8'-0"	MAIN ENTRY	B				
1	D3	13'-6"x8'-0"	FAMILY ROOM	C				
1	D4	2'-8"x8'-0"	STORAGE	B				
1	D5	2'-8"x8'-0"	LAUNDRY	D				
1	D6	2'-8"x8'-0"	GARAGE	D				
1	D7	2'-8"x8'-0"	POWDER	D				
1	D8	2'-8"x8'-0"	PANTRY	D				
1	D9	7'-8"x8'-0"	BEDROOM #3	E				
1	D10	8'-0"x8'-0"	BEDROOM #4	E				
1	D11	12'-0"x8'-0"	MASTER BEDROOM	E				
1	D12	3'-0"x8'-0"	MASTER BEDROOM	D				
1	D13	3'-0"x8'-0"	W.I.C.	D				
1	D14	3'-0"x8'-0"	MASTER BATHROOM	D				
1	D15	2'-8"x8'-0"	MASTER BATHROOM	D				
1	D16	6'-0"x8'-0"	BEDROOM #2 CLOSET	F				
1	D17	2'-8"x8'-0"	BEDROOM #2	D				
1	D18	3'-0"x8'-0"	BEDROOM #4	D				
1	D19	2'-8"x8'-0"	CLOSET	D				
1	D20	2'-8"x8'-0"	BATHROOM #4	D				
1	D21	3'-0"x8'-0"	BEDROOM #3	D				
1	D22	2'-8"x8'-0"	BEDROOM #3 CLOSET	D				
1	D23	2'-8"x8'-0"	BATHROOM #3	D				
1	D24	2'-8"x8'-0"	BEDROOM #1	F				
1	D25	2'-8"x8'-0"	BEDROOM #1 CLOSET	F				
1	D26	2'-8"x8'-0"	BATHROOM #1	D				

WINDOW SCHEDULE

QTY	ID	WIDTH/HEIGHT	LOCATION	TYPE	GLAZING	FRAME MATL	FRAME FIN	REMARKS
1	W1	6'-0"x1'-6"	BATHROOM #1	A	DUAL			
1	W2	2'-0"x3'-0"	BATHROOM #1	B	DUAL			
1	W3	2'-6"x8'-0"	BEDROOM #1	B	DUAL			
1	W4	2'-8"x5'-0"	BEDROOM #1	B	DUAL			
1	W5	12'-0"x8'-0"	DINING ROOM	A	DUAL			
1	W6	11'-0"x8'-0"	DINING ROOM	A	DUAL			
1	W7	9'-0"x8'-0"	FAMILY ROOM	A	DUAL			
1	W8	2'-6"x4'-6"	FAMILY ROOM	B	DUAL			
1	W9	2'-6"x4'-6"	FAMILY ROOM	B	DUAL			
1	W10	6'-0"x4'-6"	KITCHEN	A	DUAL			
1	W11	3'-0"x3'-0"	POWDER	B	DUAL			
1	W12	6'-0"x1'-6"	BEDROOM #4	B	DUAL			
1	W13	2'-0"x1'-6"	BATHROOM #4	C	DUAL			
1	W14	2'-0"x8'-0"	STAIRS	B	DUAL			
1	W15	8'-0"x8'-0"	STAIRS	D	DUAL			
1	W16	4'-0"x8'-0"	STAIRS	C	DUAL			
1	W17	NOT USED	NOT USED	DUAL				
1	W18	4'-0"x4'-0"	MASTER BATHROOM	A	DUAL			
1	W19	6'-0"x4'-0"	MASTER BATHROOM	B	DUAL			
1	W20	3'-0"x4'-6"	MASTER BEDROOM	B	DUAL			
1	W21	3'-0"x4'-6"	MASTER BEDROOM	B	DUAL			
1	W22	3'-0"x3'-0"	BATHROOM #2	B	DUAL			
1	W23	8'-0"x4'-6"	BEDROOM #2	B	DUAL			
1	W24	2'-0"x3'-0"	BATHROOM #3	B	DUAL			
1	W25	2'-0"x4'-6"	BEDROOM #3	B	DUAL			
1	W26	2'-0"x4'-6"	BEDROOM #3	B	DUAL			

- GL - GLASS
- AL - ALUMINUM
- ST - STEEL
- SC - SOLID CORE
- TEM - TEMPERED
- MIR - MIRROR
- AN - ANODIZE
- PG - PAINT GLOSS
- V.I.F. - VERIFY IN FIELD



RESIDENTIAL, COMMERCIAL & INTERIOR DESIGN SERVICES

NATHALIE GISPAN 818.915.4119
ERAN GISPAN 818.915.4118

OFFICE 818.789.6439
FAX 818.789.6941

www.NEDESIGNSINC.COM

General Notes

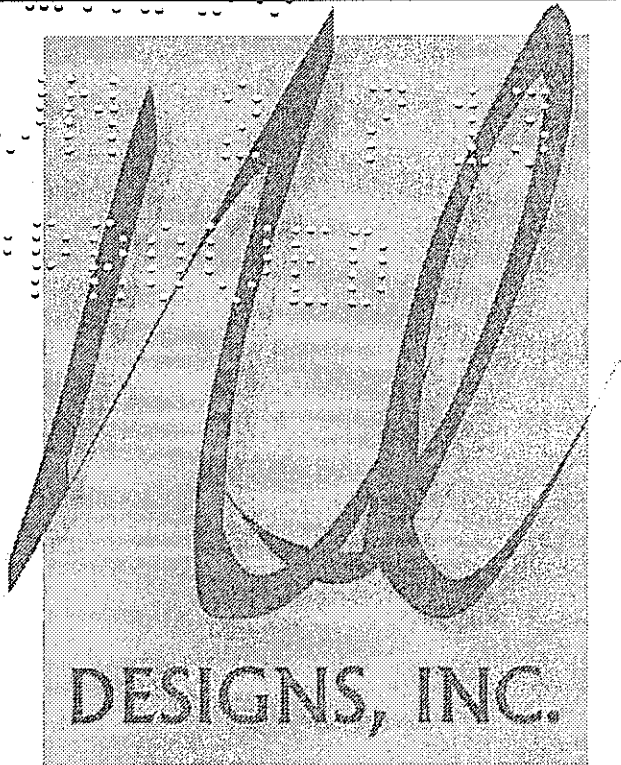
RESTRICTIONS: THESE PLANS AND INCORPORATED DESIGNS EMBODIED THEREON ARE THE PROPERTY OF N.E. DESIGNS INC. THE USE OF THESE PLANS ARE RESTRICTED TO THE ORIGINAL SITE AND OWNER FOR WHICH THEY WERE PREPARED. PUBLICATION AND REPRODUCTION IS RESTRICTED TO SUCH USE. PUBLICATION AND REPRODUCTION BY ANY METHOD, IN WHOLE OR IN PART, IS PROHIBITED. TITLE TO THE PLANS AND DESIGNS REMAIN WITH N.E. DESIGNS INC. VISUAL CONTACT WITH THEM CONSTITUTE APPROVAL WITH THESE RESTRICTIONS.

No.	Revision/Issue	Date

Project
901 N. LAUREL AVE.
LOS ANGELES, CA 90046

NEW 2 STORY HOUSE
Drawing Title
DOORS AND WINDOWS
SCHEDULE

Project	12-553	Sheet	A7.0
Date	02-04-13		
Scale	N.T.S.		



RESIDENTIAL, COMMERCIAL & INTERIOR DESIGN SERVICES

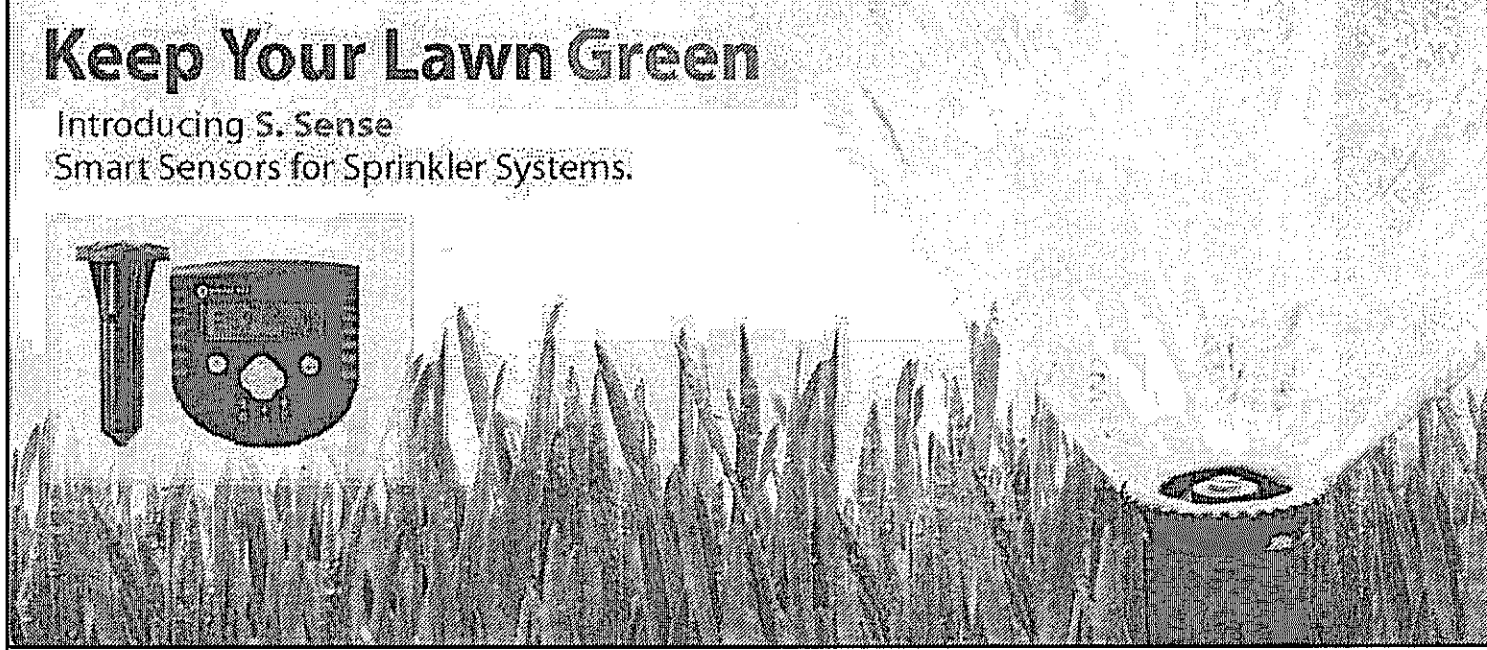
NATHALIE GISPAN 818.915.4119
ERAN GISPAN 818.915.4118

OFFICE 818.789.6439
FAX 818.789.6941

www.NEDESIGNSINC.COM

General Notes

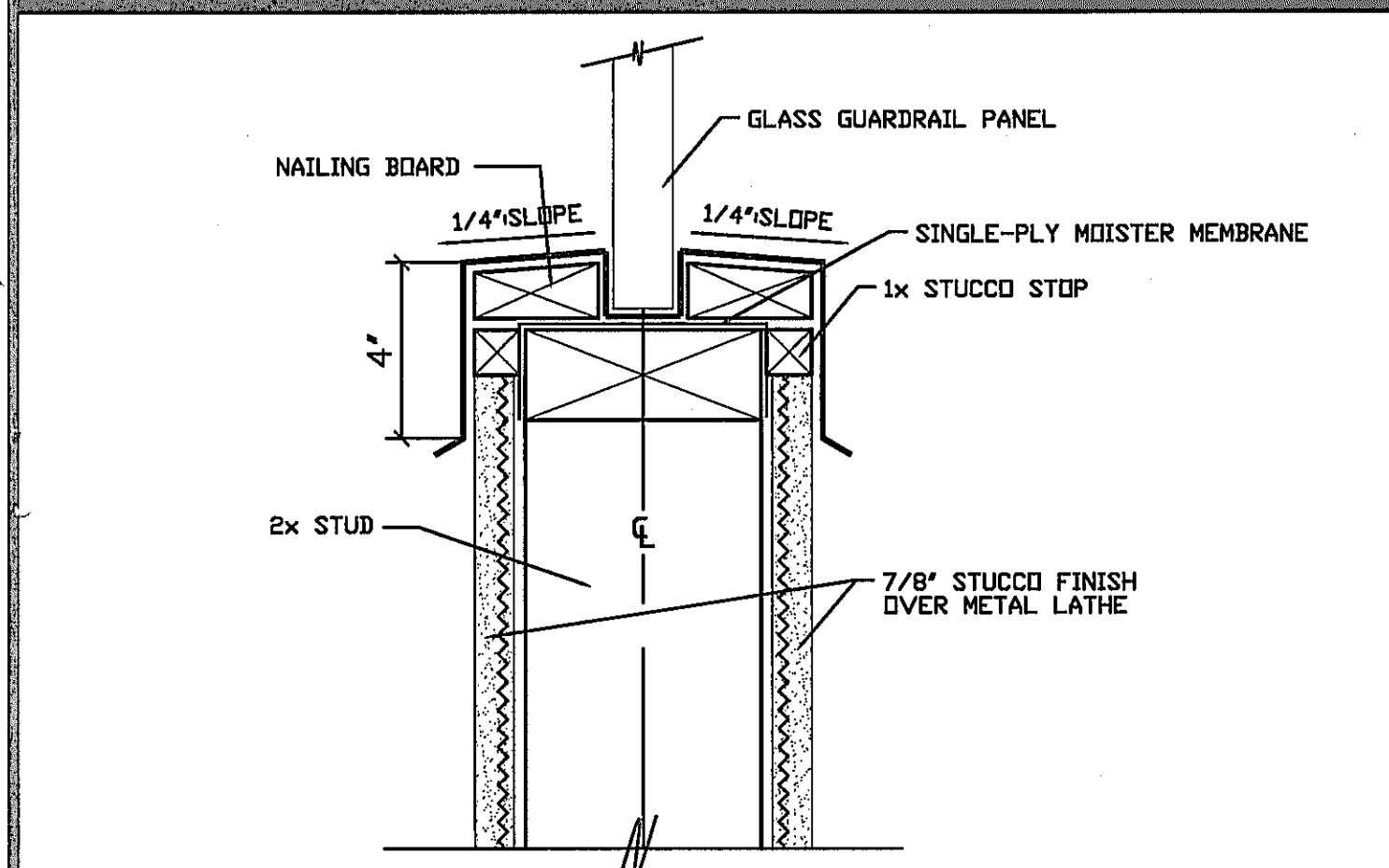
RESTRICTIONS: THESE PLANS AND INCORPORATED DESIGNS EMBODIED THEREON ARE THE PROPERTY OF N.E. DESIGNS INC. THE USE OF THESE PLANS ARE RESTRICTED TO THE ORIGINAL SITE AND OWNER FOR WHICH THEY WERE PREPARED. PUBLICATION AND REPRODUCTION IS RESTRICTED TO SUCH USE. PUBLICATION AND REPRODUCTION BY ANY METHOD, IN WHOLE OR IN PART, IS PROHIBITED. TITLE TO THE PLANS AND DESIGNS REMAIN WITH N.E. DESIGNS INC. VISUAL CONTACT WITH THEM CONSTITUTE APPROVAL WITH THESE RESTRICTIONS.



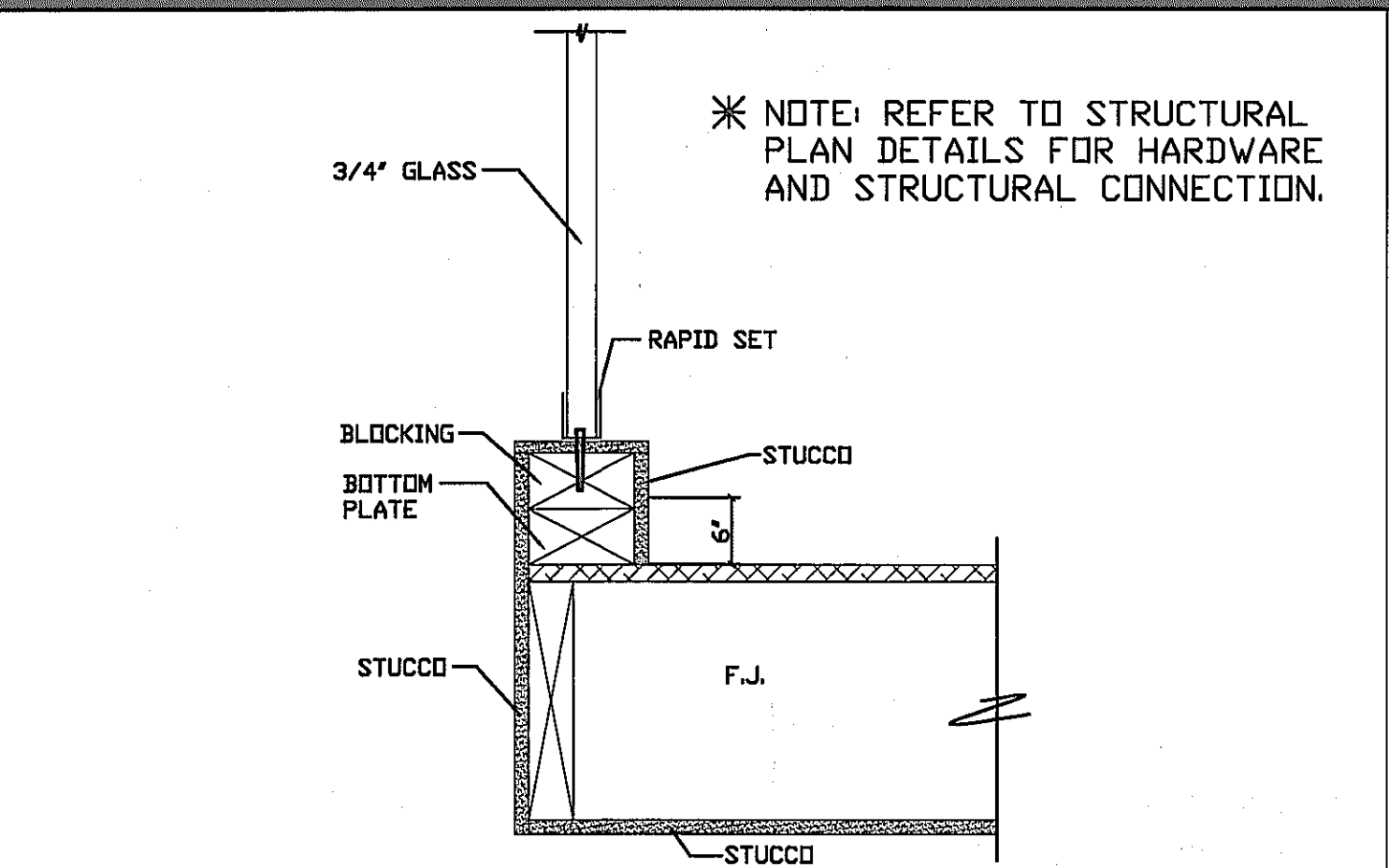
Receiver Technical Specifications
Works with all standard 24AC controllers
Automatic and Manual (Bypass) Operation
Large LCD with 64x128 pixel graphics for status and control
Easy-to-use six push button controls
Multiple language capabilities (English and Spanish)
Built-in memory maintains programming information in the absence of AC power (no battery required)
Durable plastic case (indoor use)
Unconditional four month warranty
Radio Frequency: 2.4GHz Spread Spectrum
Radio Range: 200ft Line-of-Sight (extendable)
Power Requirements: 24 V ac, 0.50 amps
Maximum common line current: 24 V ac, 1.8 amp (including master)

Common line operation: normally closed
Operating temperature: 32°F to 130°F (0°C to 54°C)
Sensor Technical Specifications
Sensors: Soil moisture and temperature
Max Range: 200 feet line of sight (extendable)
Power: 3 AAA alkaline batteries (1 year operation)
Operating Temp: 32°F to 130°F (0°C to 54°C)

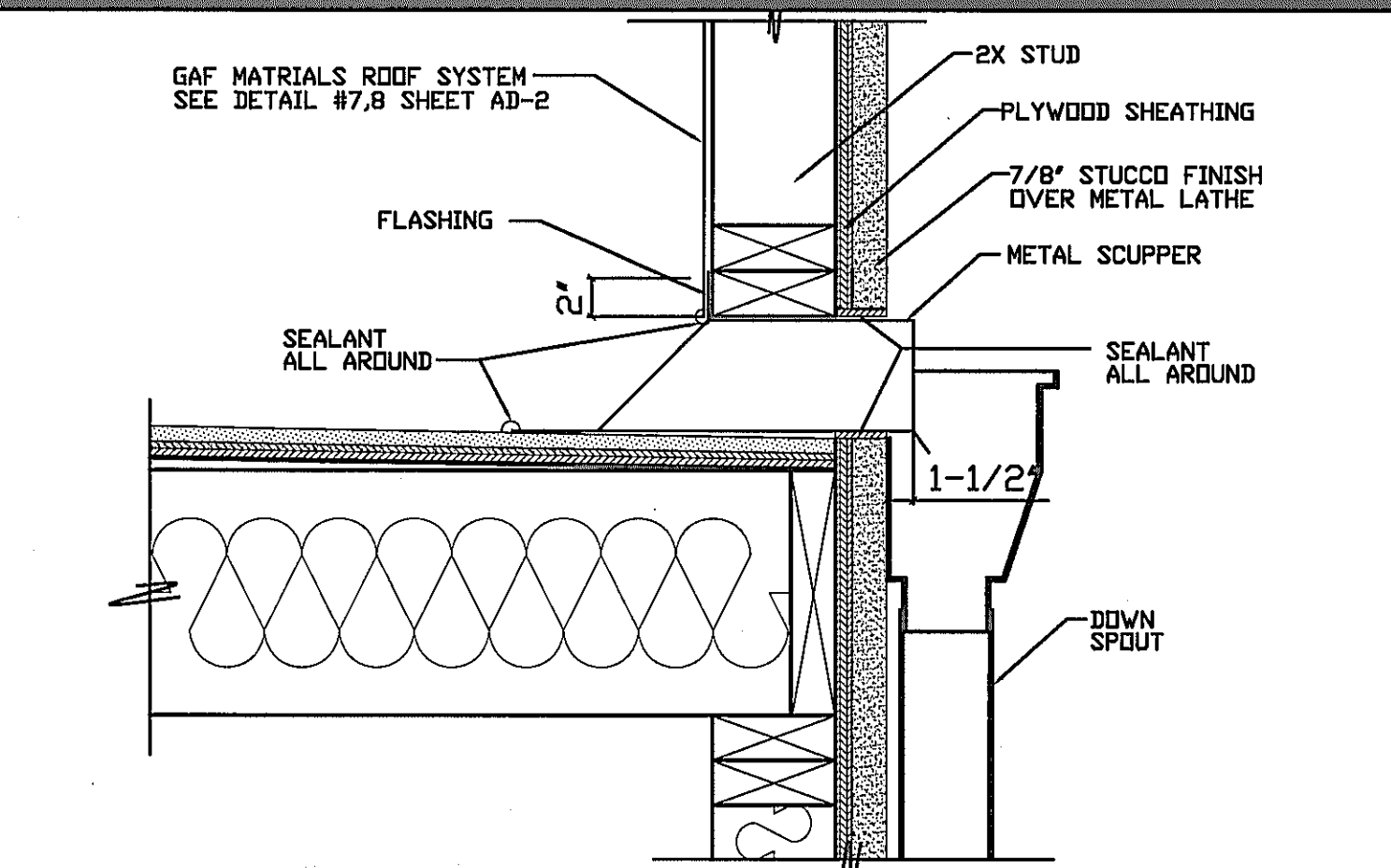
*NOTE: OWNER TO PROVIDE SMART SENSOR SPRINKLER SYSTEM PER SPECIFICATIONS OR ITS EQUIVALENT.
Department of Building and Safety
Green Building Division



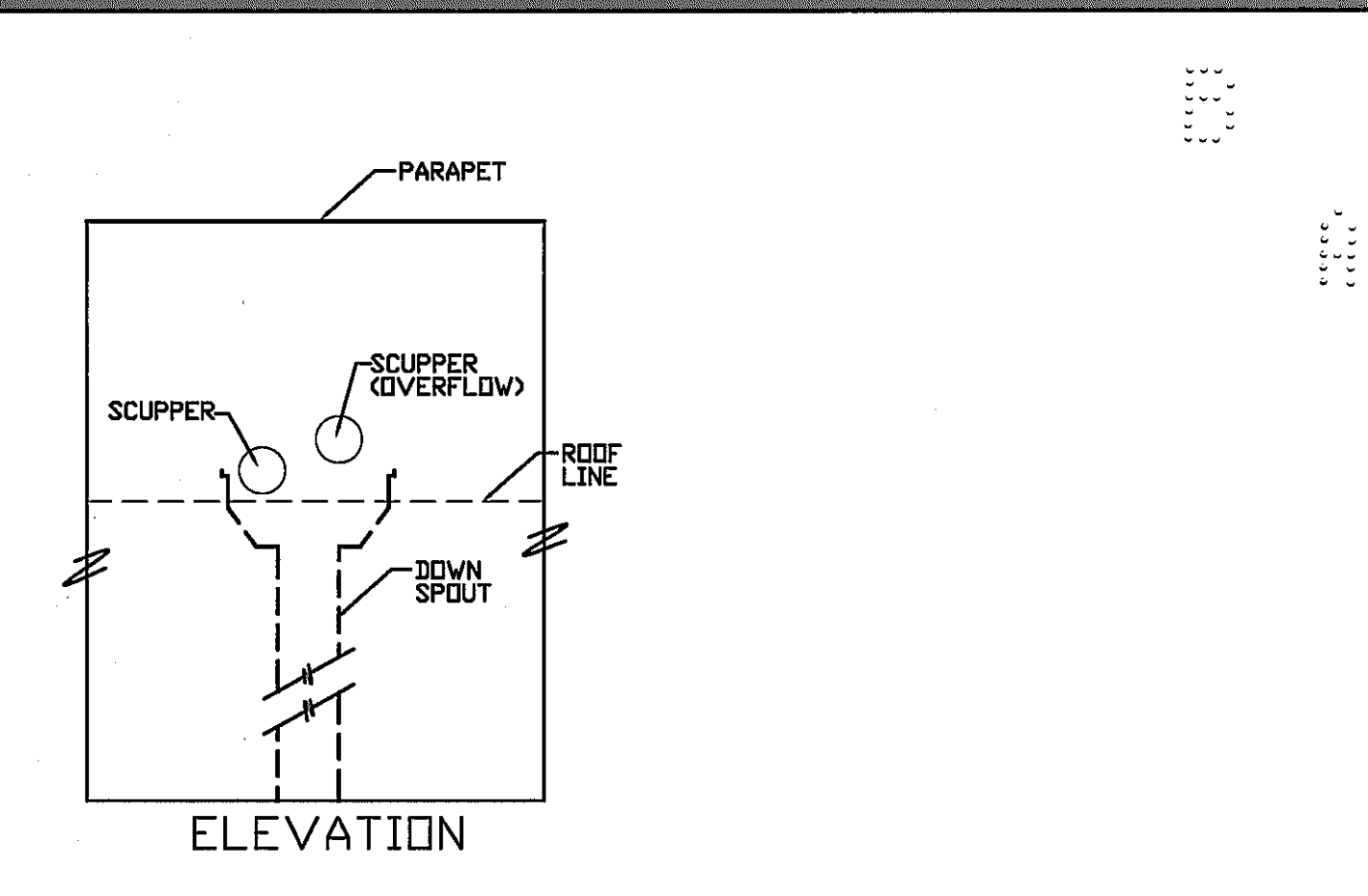
1 SCALE: N.T.S. FLASHING AT GUARDRAIL PANEL



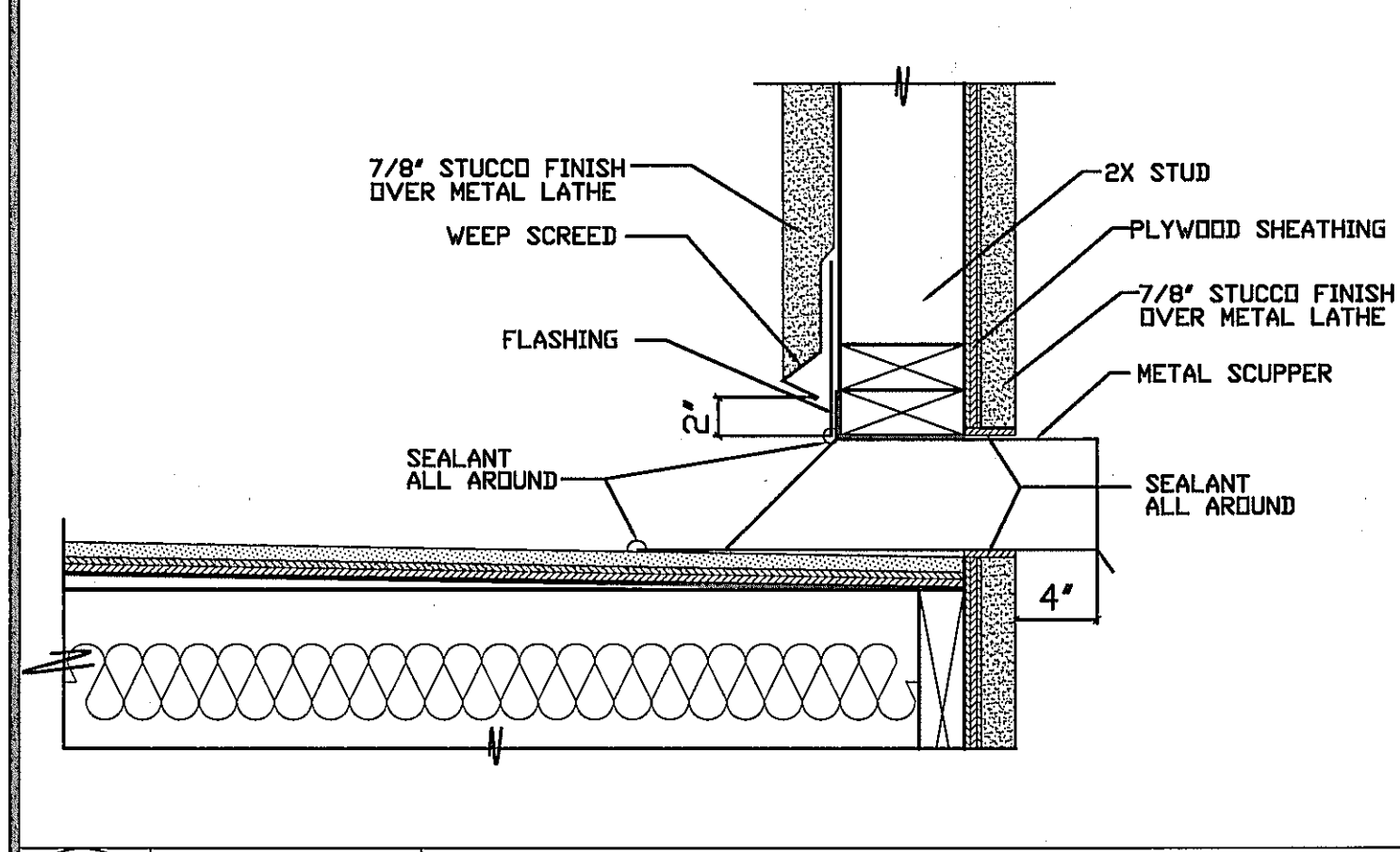
5 SCALE: N.T.S. GLASS GUARDRAIL CURB DETAIL



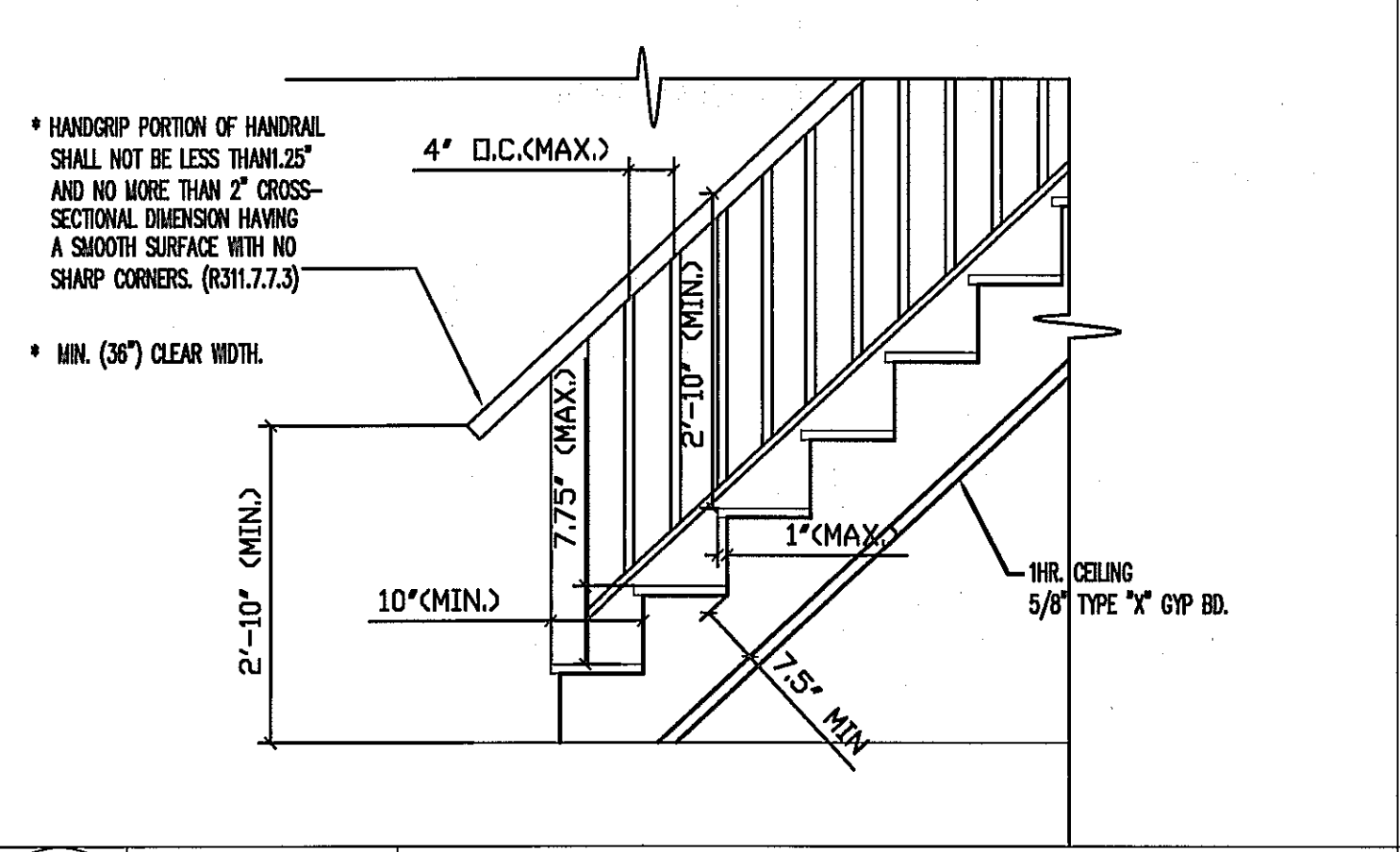
9 SCALE: N.T.S. DOWN SPOUT DETAIL



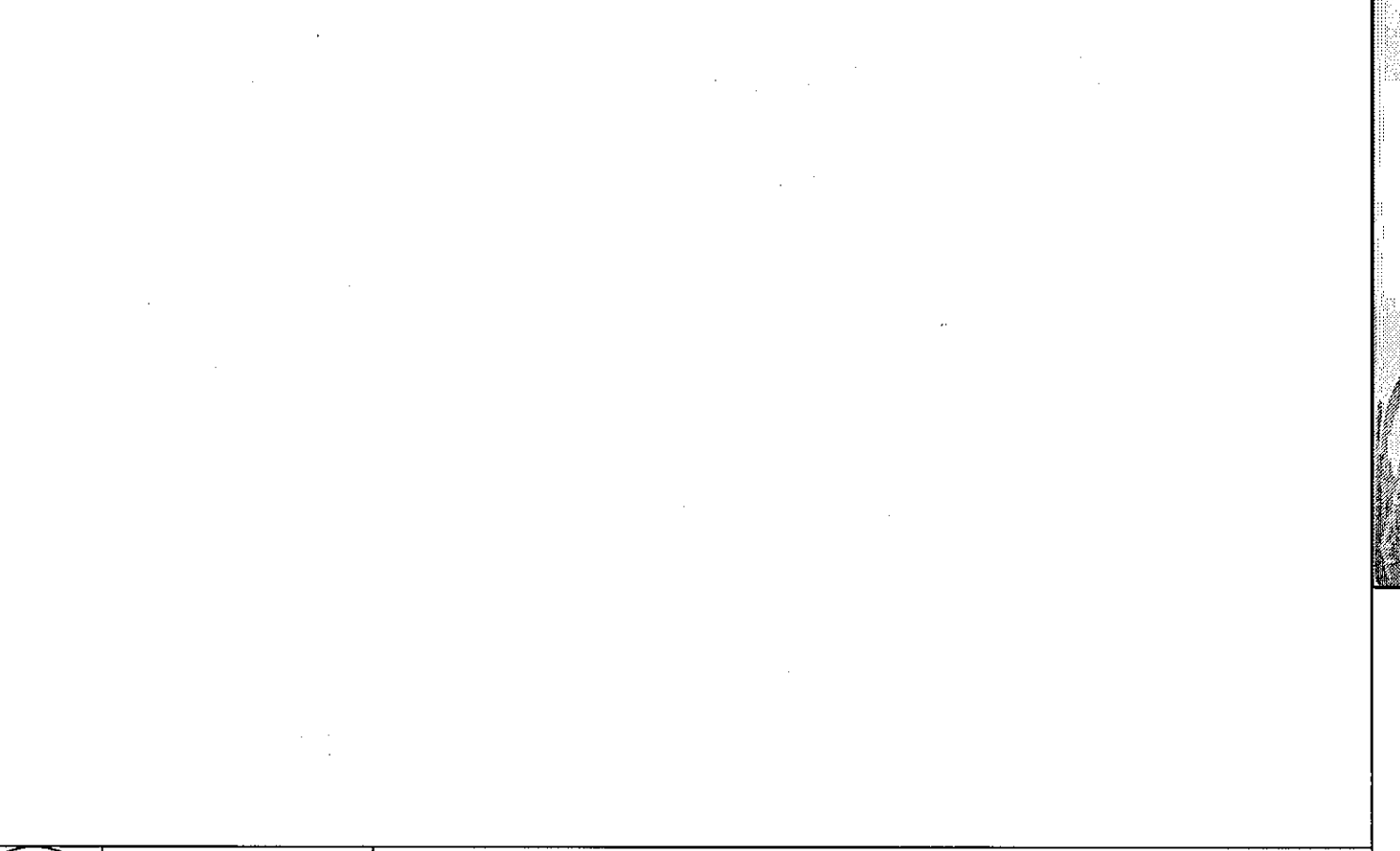
13 SCALE: N.T.S. SMART SENSOR SPRINKLER SYSTEM SPECIFICATION



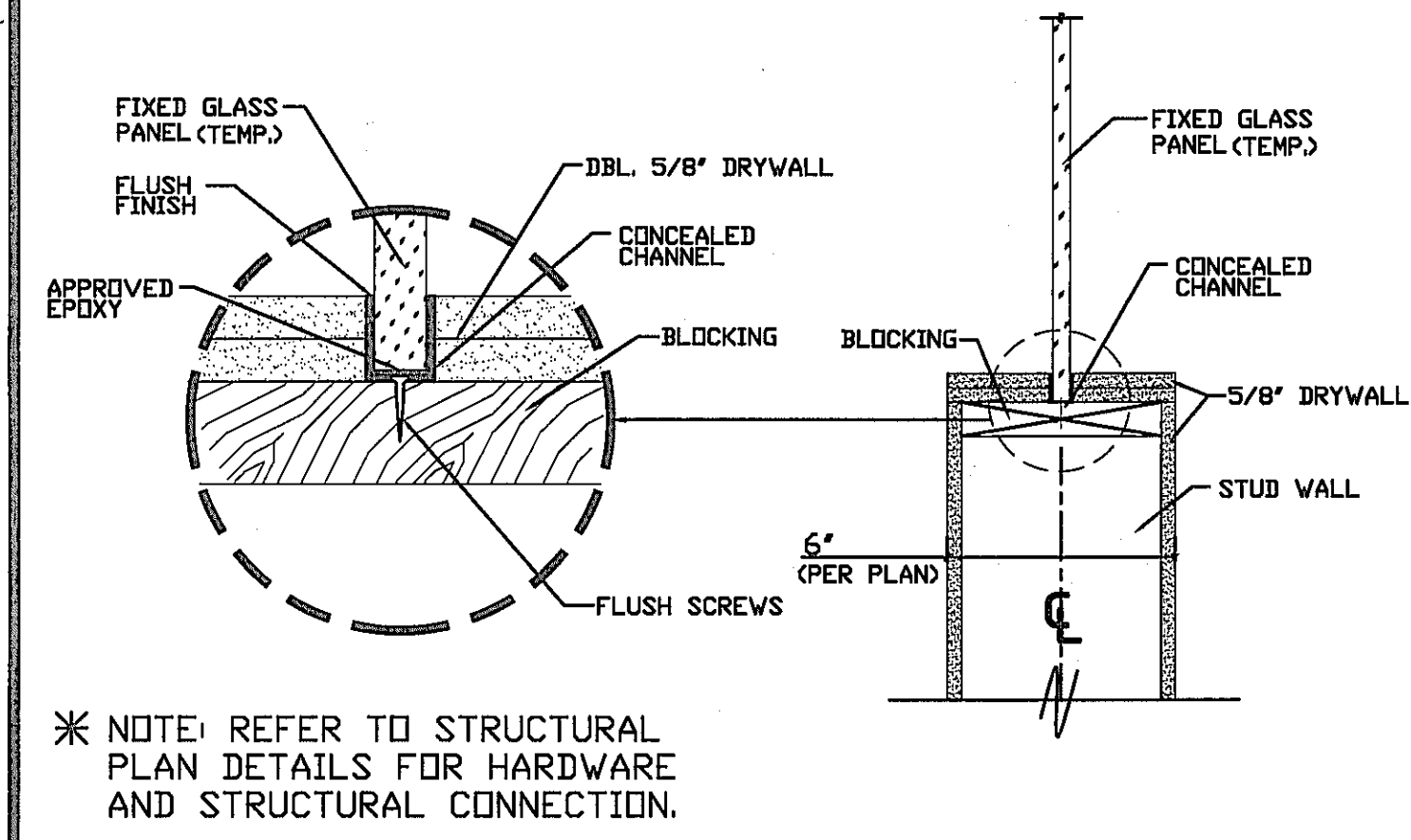
2 SCALE: N.T.S. SCUPPER



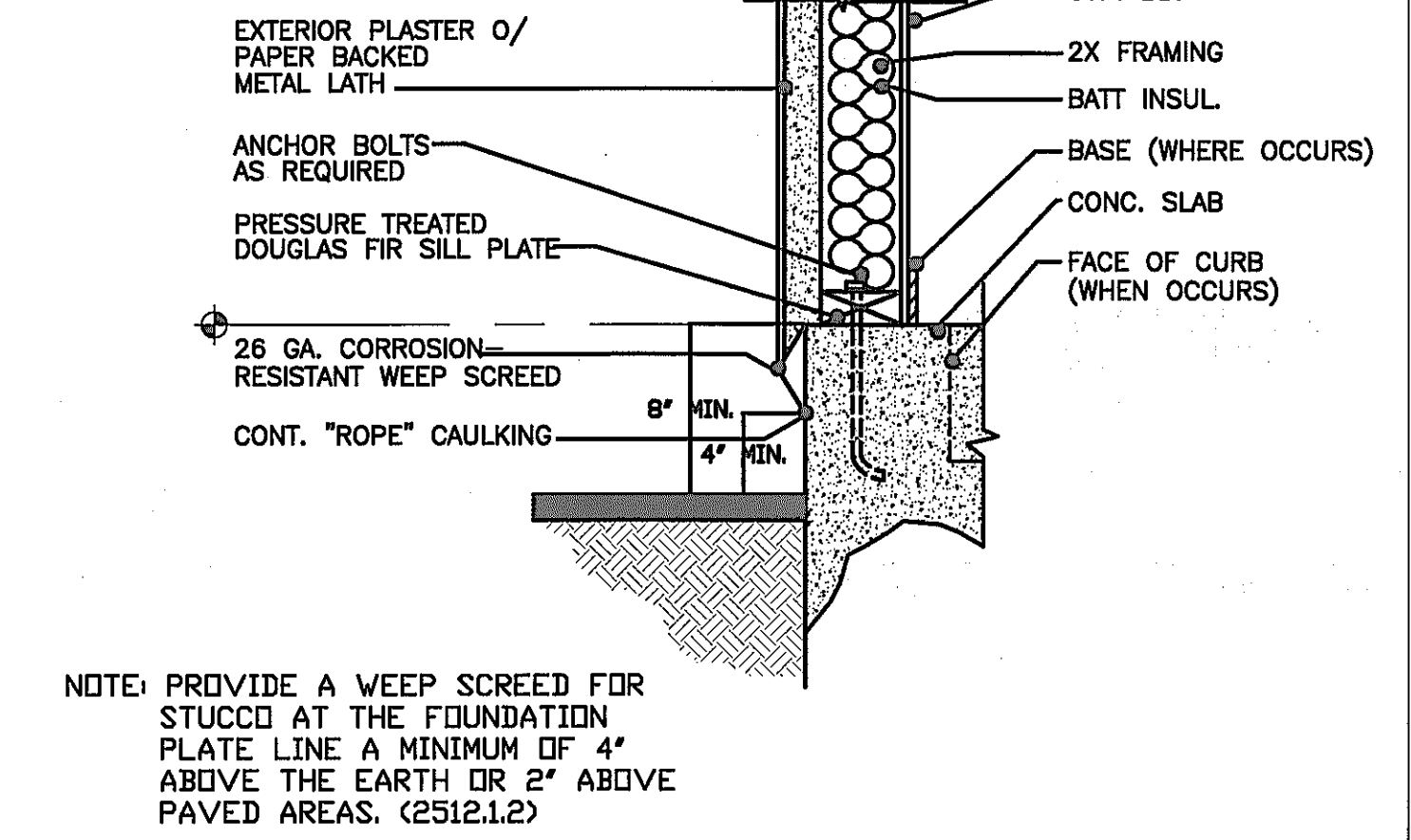
6 SCALE: N.T.S. WEEP SCREED DETAIL



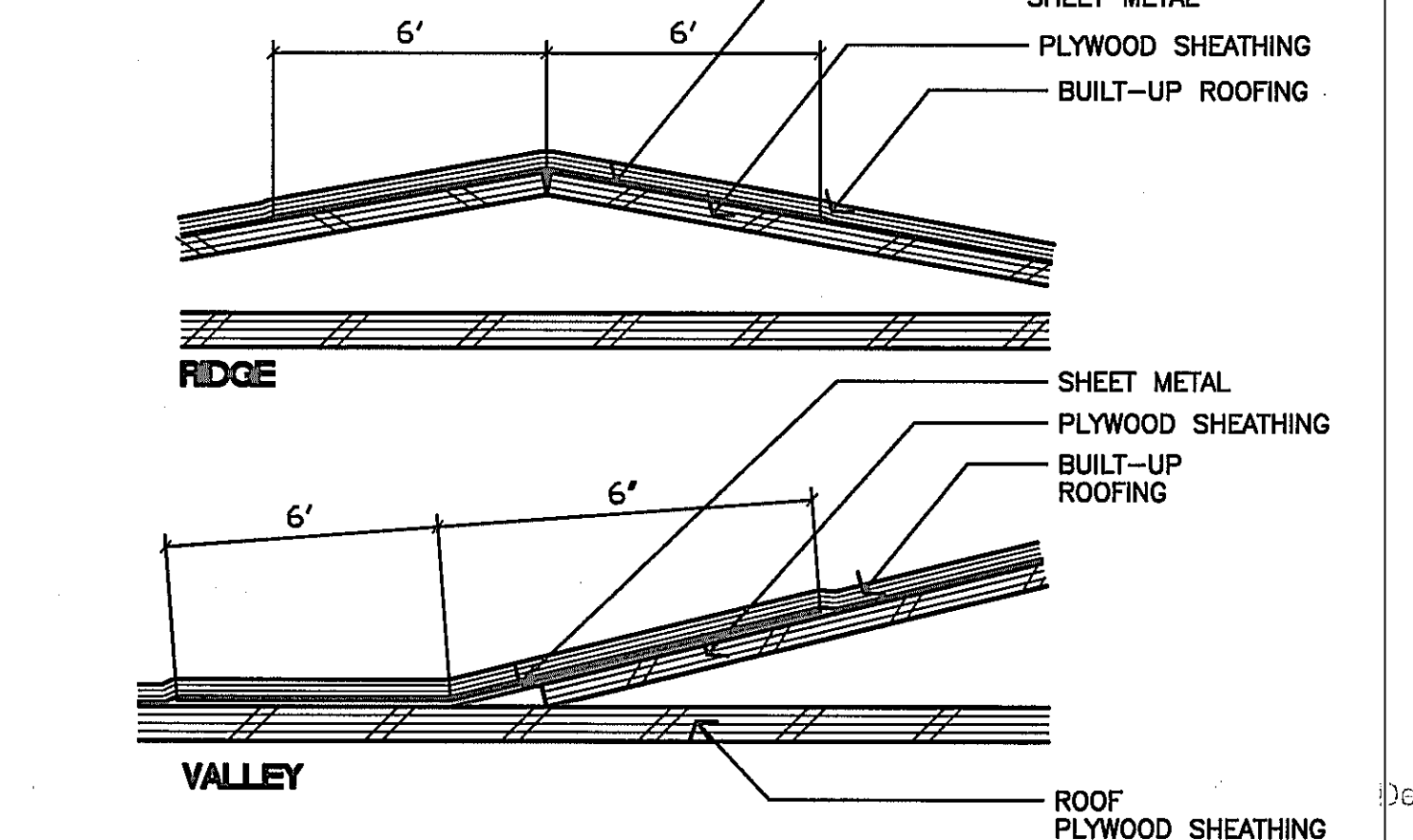
10 SCALE: N.T.S. NOT USED



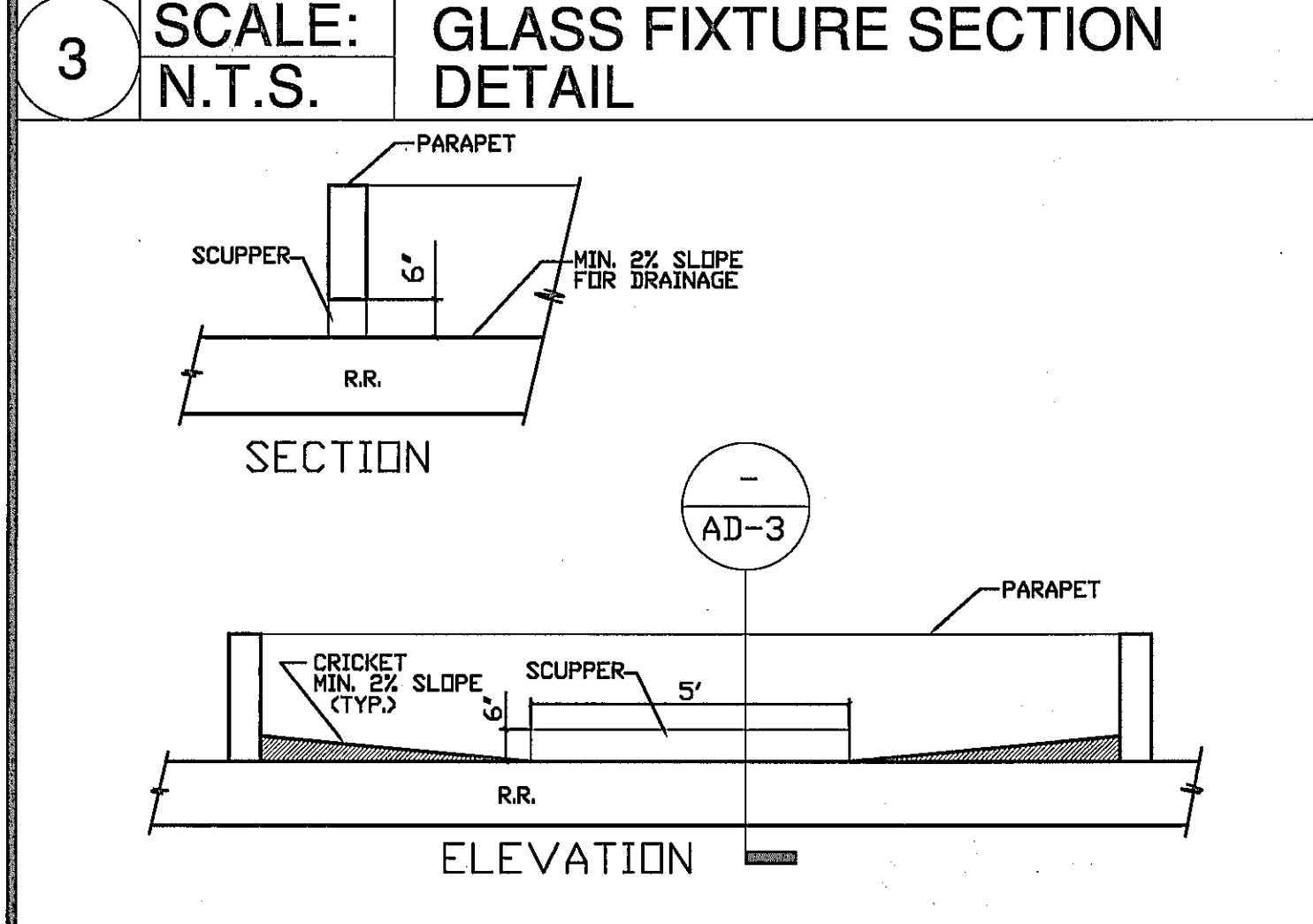
3 SCALE: N.T.S. GLASS FIXTURE SECTION DETAIL



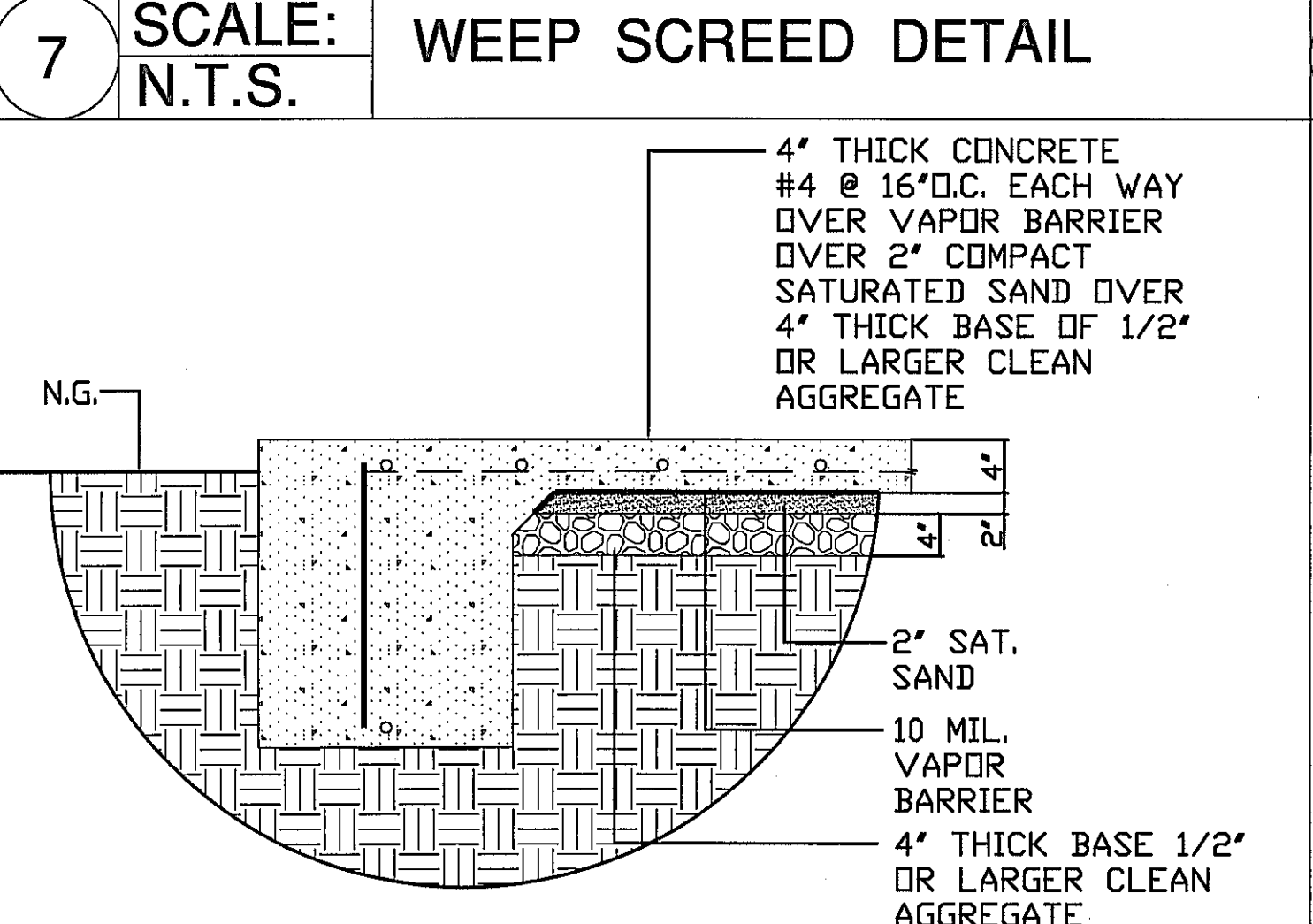
7 SCALE: N.T.S. WEEP SCREED DETAIL



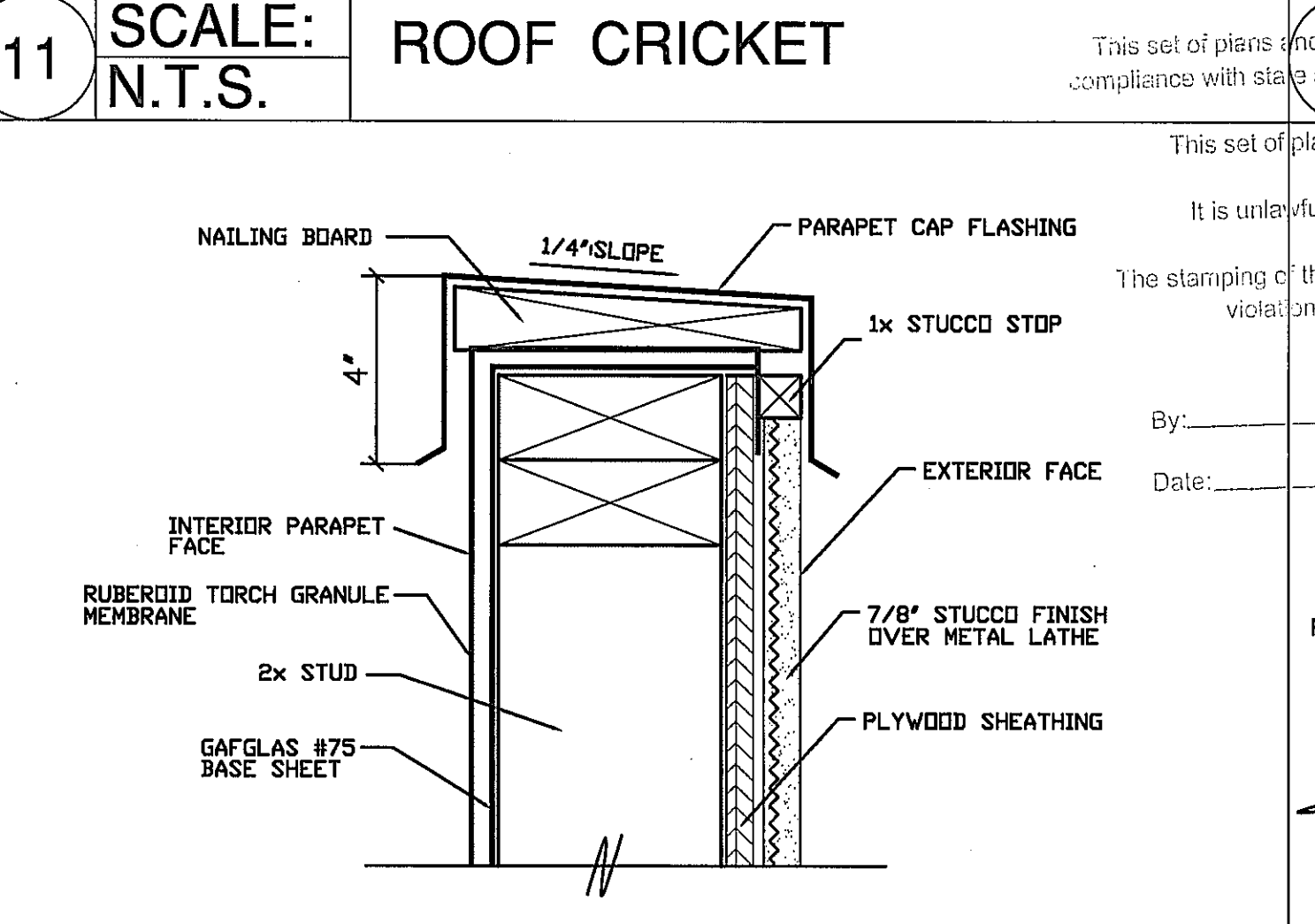
11 SCALE: N.T.S. ROOF CRICKET



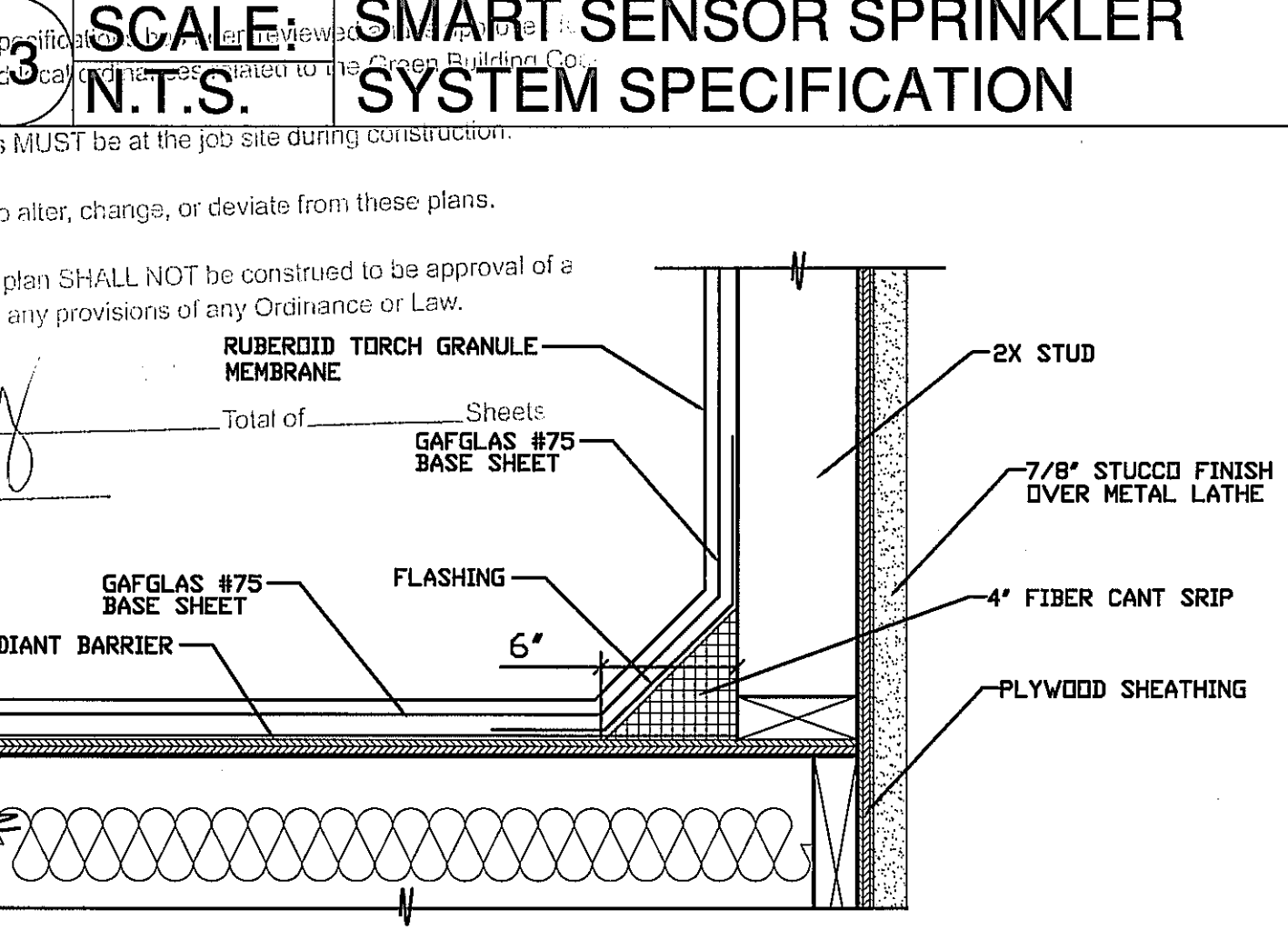
4 SCALE: N.T.S. SCUPPER DETAIL



8 SCALE: N.T.S. FOUNDATION DETAIL



12 SCALE: N.T.S. PARAPET WALL CAP



14 SCALE: N.T.S. PARAPET FLASHING DETAIL

No.	Revision/Issue	Date

Project
901 N. LAUREL AVE.
LOS ANGELES, CA 90046

NEW 2 STORY HOUSE
Drawing Title
DETAILS

Project	12-553	Sheet	AD-1
Date	02-04-13		
Scale	N.T.S.		

STRUCTURAL OBSERVATION SPECIFICATIONS

DESIGNATION OF STRUCTURAL OBSERVER



Los Angeles Regional Uniform Code Program
Committee 1-3: Structural Observation



STRUCTURAL OBSERVATION PROGRAM AND DESIGNATION OF THE STRUCTURAL OBSERVER

PROJECT ADDRESS: 901 Laurel Cyn., LA, CA 90046 permit no. _____

Description of Work: Two Story S.F.R.

Owner: L.I. Investments, LLC Architect: None Engineer: Ross Downey

STRUCTURAL OBSERVATION (only checked items are required)			
Firm or Individual to be responsible for the Structural Observation: Name: HRD Engineering, Ross Downey Phone: (818) 764-2245 Calif. Registration: SE 2628			
FOUNDATION	WALL	FRAME	DIAPHRAGM
<input checked="" type="checkbox"/> Footing, Stem Walls, Piers	<input type="checkbox"/> Concrete	<input checked="" type="checkbox"/> Steel Moment Frame	<input type="checkbox"/> Concrete
<input type="checkbox"/> Mat Foundation	<input type="checkbox"/> Masonry	<input type="checkbox"/> Steel Braced Frame	<input type="checkbox"/> Steel Deck
<input type="checkbox"/> Caisson, Piles, Grade Beams	<input checked="" type="checkbox"/> Wood	<input type="checkbox"/> Concrete Moment Frame	<input checked="" type="checkbox"/> Wood
<input type="checkbox"/> Stepp'g, Retaining Foundation, Hillside Special Anchors	<input type="checkbox"/> Others:	<input type="checkbox"/> Masonry Wall Frame	<input type="checkbox"/> Others:
<input type="checkbox"/> Others:		<input checked="" type="checkbox"/> Others: Cant. Col.	

DECLARATION BY OWNER
I, the Owner of the Project, declare that the above listed firm or individual is hired by me to be the Structural Observer.

Signature _____ Date _____

DECLARATION BY architect or engineer of record (required if the Structural Observer is different from the Architect or Engineer of Record)
I, the Architect or Engineer of record for the project, declare that the above listed firm or individual is designated by me to be responsible for the Structural Observation.

Signature _____ License No. _____ Date _____

CITY OF LOS ANGELES DEPARTMENT OF BUILDING & SAFETY GENERAL NOTES FOR STRUCTURAL OBSERVATION

- STRUCTURAL OBSERVATION IS REQUIRED FOR THE STRUCTURAL SYSTEM IN ACCORDANCE WITH MGD 110. structural observation is the visual observation of the ELEMENTS AND CONNECTIONS OF THE STRUCTURAL SYSTEM AT SIGNIFICANT CONSTRUCTION STAGES AND THE COMPLETED STRUCTURE FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND SPECIFICATIONS. STRUCTURAL OBSERVATION DOES NOT WAIVE THE RESPONSIBILITY FOR THE INSPECTIN REQUIRED OF THE BUILDING INSPECTOR OR THE DEPUTY INSPECTOR.
- THE OWNER SHALL EMPLOY A CIVIL OR STRUCTURAL ENGINEER OR ARCHITECT TO PERFORM THE STRUCTURAL OBSERVATION. THE ENGINEER OR ARCHITECT SHALL BE REGISTERED OR LICENSED IN THE STATE OF CALIFORNIA. THE DEPARTMENT OF BUILDING AND SAFETY RECOMMENDS THE USE OF THE ENGINEER OR ARCHITECT RESPONSIBLE FOR THE STRUCTURAL DESIGN WHEN THEY ARE INDEPENDENT OF THE CONTRACTOR.
- THE STRUCTURAL OBSERVER SHALL PROVIDE EVIDENCE OF EMPLOYMENT BY THE OWNER. A LETTER FROM THE OWNER OR A COPY OF THE AGREEMENT FOR SERVICES SHALL BE SENT TO THE BUILDING BEFORE THE FIRST SITE VISIT. THE STRUCTURAL OBSERVER SHALL ALSO INFORM THE OWNER OF THE REQUIREMENTS FOR A RECONSTRUCTION MEETING AND SHALL PRESIDE OVER THIS MEETING.
- THE OWNER OR OWNER'S REPRESENTATIVE SHALL COORDINATE AND CALL FOR A MEETING BETWEEN THE ENGINEER OR ARCHITECT RESPONSIBLE FOR THE STRUCTURAL DESIGN, STRUCTURAL OBSERVER, CONTRACTOR, AFFECTED SUBCONTRACTORS AND DEPUTY INSPECTORS. THE PURPOSE OF THE MEETING SHALL BE TO IDENTIFY THE MAJOR STRUCTURAL ELEMENTS AND CONNECTIONS THAT AFFECT THE VERTICAL AND LATERAL LOAD SYSTEMS OF THE STRUCTURE AND TO REVIEW SCHEDULING OF THE REQUIRED OBSERVATIONS. A RECORD OF THE MEETING SHALL BE INCLUDED IN THE FIRST OBSERVATION REPORT SUBMITTED TO THE BUILDING INSPECTOR.
- THE STRUCTURAL OBSERVER SHALL PERFORM SITE VISITS AT THOSE STEPS IN THE PROGRESS OF THE WORK THAT ALLOW FOR CORRECTION OF DEFICIENCIES WITHOUT SUBSTANTIAL EFFORT OR UNCOVERING OF THE WORK INVOLVED. AT A MINIMUM, THE FOLLOWING SIGNIFICANT CONSTRUCTION STAGES REQUIRE A SITE VISIT AND AN OBSERVATION REPORT FROM THE STRUCTURAL OBSERVER:

CONSTRUCTION STAGES	ELEMENTS TO BE OBSERVED
a) PRE-CONSTRUCTION	/ REVIEW S.O. (STRUCTURAL DESIGNER, CONTRACTOR, SUBS AND DEPUTY(S) MUST ATTEND)
b) FOUNDATION	/ AFTER FOOTINGS DUG/REINF. PLACED, BEFORE CONC. POURED
c) FRAMING	/ AFTER ALL FRAMING IS INSTALLED, BEFORE FINAL
d) SHEAR WALL NAILING	/ AFTER INSTALLED
e) EDGE DIST. A.B./HD'S	/ AFTER INSTALLED
e) FINAL	/ AFTER ALL STRUCTURAL / ROUGH IS INSTALLED
- THE STRUCTURAL OBSERVER SHALL PREPARE A REPORT ON THE DEPARTMENT FORM B & S 261 FOR EACH SIGNIFICANT STAGE OF CONSTRUCTION OBSERVED. THE ORIGINAL OF THE OBSERVATION REPORT SHALL BE SENT TO THE BUILDING INSPECTORS OFFICE AND SHALL BE SIGNED AND SEALED (NET STAMP) BY THE RESPONSIBLE STRUCTURAL OBSERVER. ONE COPY OF THE OBSERVATION REPORT SHALL BE ATTACHED TO THE APPROVED PLANS. THE COPY ATTACHED TO THE PLANS NEED NOT BE SEALED BUT SHALL BE SIGNED BY THE RESPONSIBLE STRUCTURAL OBSERVER OR THE DESIGNEE. COPIES OF THE REPORT SHALL ALSO BE GIVEN TO THE OWNER, CONTRACTOR, AND DEPUTY INSPECTOR.
- A FINAL OBSERVATION REPORT MUST BE SUBMITTED WHICH SHOWS THAT ALL OBSERVED DEFICIENCIES WERE RESOLVED AND THE STRUCTURAL SYSTEM GENERALLY CONFORMS WITH THE APPROVED PLANS AND SPECIFICATIONS. THE DEPARTMENT OF BUILDING AND SAFETY WILL NOT ACCEPT THE STRUCTURAL WORK WITHOUT THIS FINAL OBSERVATION REPORT AND THE CORRECTION OF SPECIFIC DEFICIENCIES NOTED DURING NORMAL BUILDING AND DEPUTY INSPECTION.

INSPECTION GROUP NAME _____
STREET ADDRESS _____
COMMUNITY OF LA, CA, ZIP CODE _____
- THE STRUCTURAL OBSERVER SHALL SEND THE ORIGINAL OBSERVATION REPORT TO THE FOLLOWING OFFICE:

WHEN THE OWNER ELECTS TO CHANGE THE STRUCTURAL OBSERVER OF RECORD THE OWNER SHALL:

 - NOTIFY THE BUILDING INSPECTOR IN WRITING BEFORE THE NEXT INSPECTION,
 - CALL AN ADDITIONAL PRE-CONSTRUCTION MEETING, AND
 - FURNISH THE REPLACEMENT STRUCTURAL OBSERVER WITH A COPY OF ALL PREVIOUS OBSERVATION REPORTS.
 THE REPLACEMENT OBSERVER SHALL APPROVE THE CORRECTION OF THE ORIGINAL DEFICIENCIES UNLESS OTHERWISE APPROVED BY PLAN CHECK SUPERVISION. THE DEPARTMENT SHALL BE TO CORRECT ANY PROPERLY NOTED DEFICIENCIES WITHOUT CONSIDERATION OF THEIR SOURCE.
- THE ENGINEER OR ARCHITECT OF RECORD SHALL DEVELOP ALL CHANGES RELATING TO THE STRUCTURAL SYSTEMS. THE BUILDING DEPARTMENT SHALL REVIEW AND APPROVE ALL CHANGES TO THE APPROVED PLANS AND SPECIFICATIONS.

* STRUCTURAL DESIGNER IS THE ENGINEER OF RECORD, ROSS DOWNEY

RESPONSIBILITIES

- OWNER:** - SHALL EMPLOY S.O. ENGINEER. S.O. ENGINEER TO BE ENGINEER OF RECORD UNLESS NOT POSSIBLE OR PRACTICAL.
- SHALL CALL FOR AND COORDINATE PRE-CONST. MEETING.
 - SHALL NOTIFY CITY INSPECTOR FOR ANY S.O. ENGR. CHANGES (IN WRITING).
 - SHALL CALL FOR AND COORDINATE NEW PRE-CONST. MEETING FOR NEW S.O. ENGR. WHEN A CHANGE OCCURS.
 - SHALL PROVIDE NEW S.O. ENGR. WITH ALL RECORDED COPIES OF ALL S.O. REPORTS.
 - SHALL CONSULT S.O. ENGR. AND ENGR. OF RECORD ON ALL DISCHARGES OF DEPUTY INSPECTORS. DISCHARGE MUST BE JUSTIFIED (DISCHARGE FOR DOING HIS/HER JOB IS NOT AN ACCEPTABLE REASON). REPLACEMENT MUST BE APPROVED BY THE ENGR. OF RECORD.
- STRUCTURAL DESIGNER:** - SHALL ATTEND PRE-CONSTRUCTION MEETING WHETHER OR NOT HE/SHE IS THE S.O. ENGR.
- SHALL REMAIN RESPONSIBLE PARTY FOR ALL CHANGES EVEN IF NOT S.O. ENGINEER.

- S.O. ENGR.:** - SHALL SUBMIT VERIFICATION OF EMPLOYMENT BY OWNER TO INSPECTOR AT BEGINNING OF PROJECT BEFORE FIRST MEETING.
- SHALL PRESIDE OVER PRE-CONST. AND ALL OTHER MEETINGS.
 - SHALL MAKE A RECORD OF ALL MEETINGS TO INCLUDE ATTENDEES OF THE PRE-CONSTRUCTION MEETING.
 - SHALL PROVIDE REPORT OF THE PRE-CONST. MEETING.
 - SHALL MAKE SITE VISIT AT ALL SIGNIFICANT STAGES OF CONSTRUCTION EVEN IF NOT LISTED IN CONST. STAGES ABOVE.
 - SHALL IDENTIFY ALL GROSS ERRORS AND OMISSIONS IN THE MAJOR ELEMENTS AND CONNECTIONS OF THE STRUCTURE.
 - SHALL PREPARE ALL REPORTS ON CITY FORM (B45 261).
 - SHALL MAIL S.O. REPORTS TO CITY INSPECTOR.
 - SHALL ATTACH FIELD COPY TO APPROVED PLANS.
 - SHALL DISTRIBUTE COPIES OF S.O. REPORT TO OWNER, CONTRACTOR AND DEPUTY (WHEN APPROPRIATE).
- CONTRACTOR:** - SHALL ATTEND THE PRE-CONSTRUCTION MEETING ALONG WITH ALL PARTIES OF HIS/HER BUSINESS THAT MAY WORK ON PROJECT IN CAPACITY OF SUPERVISOR, FOREMAN OR SUPER INTENDENT

- SHALL CONSTRUCT STRUCTURAL SYSTEM AND MAKE STRUCTURAL CHANGES ONLY AFTER THEY ARE APPROVED IN WRITING BY ENGR. OF RECORD (CHANGES PROPOSED BY THE S.O. ENGR. IF DIFFERENT FROM ENGR. OF RECORD DOES NOT CONSTITUTE APPROVAL).
 - SHALL NOTIFY S.O. ENGR. WHEN DESIGNATED STAGES OCCUR(S).
 - SHALL STOP WORK PROGRESS UNTIL REG. SITE VISIT, REPORT AND CITY INSPECTION IS COMPLETED SATISFACTORILY.
 - SHALL CORRECT ALL DEFICIENCIES NOTED BY S.O. ENGR. AND/OR ENGINEER OF RECORD.
 - SHALL CONSULT OWNER, S.O. ENGR. AND ENGR. OF RECORD ON ALL DISCHARGES OF DEPUTY INSPECTORS. DISCHARGE MUST BE JUSTIFIED (DISCHARGE FOR DOING HIS/HER JOB IS NOT AN ACCEPTABLE REASON). REPLACEMENT MUST BE APPROVED BY THE ENGR. OF RECORD.
- DEPUTY INSPECTOR:** - SHALL ATTEND PRE-CONSTRUCTION MEETING(S).
- SHALL CONTINUOUSLY INSPECT STRUCTURAL SYSTEM FOR COMPLIANCE WITH THE APPROVED PLANS.
 - SHALL BE RESPONSIBLE TO THE S.O. ENGR. (AND OWNER) NOT

- THE CONTRACTOR, IF DISCHARGED BY CONTRACTOR AND/OR OWNER BEFORE HIS/HER WORK IS COMPLETED, DEPUTY INSPECTOR SHALL CONTACT S.O. ENGR. AND/OR ENGR. OF RECORD FOR FINAL APPROVAL OF DISCHARGE (SEE NOTES UNDER OWNER AND AT LEFT FOR ADDITIONAL INFORMATION).
- SHALL IDENTIFY ALL ERRORS AND OMISSIONS TO THE S.O. ENGR. AND ENGR. OF RECORD (IF DIFFERENT) IN ADDITION TO CONTRACTOR.
- CITY INSPECTOR:** - SHALL INSPECT PROJECT FOR COMPLIANCE WITH THE APPROVED PLANS.
- SHALL INSPECT TO VERIFY THAT ALL REPORTED DEFICIENCIES REPORTED ARE CORRECTED.
 - SHALL REFER ALL DISPUTED DEFICIENCIES TO PLAN CHECK SUPERVISOR.
 - SHALL NOTIFY ARCH. AND/OR ENGR. FOR ALL CODE VIOLATIONS WHETHER OR NOT REPORTED AS DEFICIENCIES.
 - SHALL NOTIFY CONTRACTOR WHEN AN ADDITIONAL S.O. SITE VISIT IS REQ.
 - SHALL ALLOW WORK TO PROGRESS IF REPORTED DEFICIENCIES ARE CORRECTED AND MINOR IN NATURE WITH OUT FOLLOW-UP S.O. WHEN CITY INSPECTOR HAS VERIFIED THE CORRECTIONS BY INSPECTION.
 - SHALL APPROVE FINAL ONLY AFTER FINAL S.O. REPORT IS RECEIVED FROM S.O. ENGR. INDICATING "FINAL" (OR IF WRITTEN LETTER).

REVISIONS _____ BY _____

THESE PLANS DRAWINGS, DESIGNS AND SPECIFICATIONS ARE THE SOLE PROPERTY OF HRD ENGINEERING FOR THE EXCLUSIVE USE OF ITS CLIENTS FOR THE SPECIFIC PROJECT WITH APPROVAL BY THE CLIENT. ANY REUSE, REPRODUCTION, TRANSFER OF USE, NOT PERMITTED WITHOUT WRITTEN APPROVAL. COPYRIGHT © HRD ENGINEERING.

HRD ENGINEERING
STRUCTURAL DESIGN AND DRAFTING
7463 VARNA AVE., SECOND FLOOR
N. HOLLYWOOD, CA 91605
PHONE NO. (805) 431-5415 - FAX NO. (805) 888-4840

STRUCTURAL OBSERVATION

TWO STORY SINGLE FAMILY RESIDENCE
901 LAUREL CYN.
LA, CA 90046 FOR:
L.I. INVESTMENTS, LLC



Date 12-18-12
Scale
Drawn hrd
Job
Sheet
SO-1.1
of

STANDARD QUALITY ASSURANCE PLAN

I. PURPOSE

This Standard Quality Assurance (QA) Plan provides specifications, procedures, and illustrative details to comply with the requirements of the 2002 Los Angeles Building Code. The Quality Assurance requirements described in this Standard QA Plan are intended to promote public safety and welfare by standardizing inspections, tests, and all other applicable measures that ensure substantial compliance with the code performance objectives of steel moment frame connections.

II. HOW TO USE THIS STANDARD QA PLAN

To use this Standard QA Plan, follow the seven steps outlined below:

- 1. Standard QA Plan shall be attached to and made part of the structural Plans.
2. Engineer of Record shall identify the type of steel moment frame to be used in the building or structure by placing an "X" mark in the box below:

- SPECIAL MOMENT FRAME (SMF)
INTERMEDIATE MOMENT FRAME (IMF)
ORDINARY MOMENT FRAME (OMF)

- 3. All of the Quality Assurance components listed in this Standard QA Plan shall be applied to special moment frames, intermediate moment frames, and ordinary moment frames, unless noted otherwise herein.
4. Engineer of Record shall place the following note on the structural plans stating "All specifications, tables, and notes in the City of Los Angeles' Standard Quality Assurance Plan for Steel Moment Frames shall be part of these approved structural plans."
5. Engineer of Record shall sign and stamp the Standard QA Plan in the box provided at the lower right corner of each sheet.
6. Organize all reports to be reviewed and submitted to the City Building Inspector as required by Table 1 on Sheet 2.
7. Any deviations in the quality assurance program from this Standard Plan shall require a separate written approval by the Engineer of Record and the Department. The procedures, specifications and illustrative details as described in this Standard QA Plan shall not exempt the Engineer of Record from using engineering judgment in determining the suitability of applying this Standard QA Plan to any welded connection.

III. GENERAL REQUIREMENTS

- 1. Codes
The design and construction of steel moment frames shall be in compliance with the following codes:
a. 2002 City of Los Angeles Building Code,
b. AISC Seismic Provisions for Structural Steel Buildings, Part I (LRFD) and Part III (ASD), dated May 21, 2002, and
c. AWS D1.1/D1.1M:2002 Structural Welding Code - Steel.

- 2. Material Specifications
a. Structural steel shall comply with UBC Standard 22-1 and the following ASTM standard specifications:
i. Wide flange shapes.....ASTM A572 (50), A913 (50), A992
ii. Continuity, doubler and column base plates, shear tabs.....ASTM A36
iii. Anchor bolts at column base plates.....ASTM F1554
iv. Fabricate and erect structural steel in compliance with either the 2001 Edition of AISC "Load and Resistance Factor Design Specification for Structural Steel Buildings" or 1989 Edition of the AISC "Allowable Stress Design Specification for Structural Steel Buildings."
b. High strength bolts shall comply with the following requirements and ASTM standard specifications:
i. High strength bolts, nuts, and washers.....ASTM A325, A490
ii. Shall be installed in accordance with the "Specifications for Structural Joints using ASTM A325 or A490 Bolts."
iii. Shall be tightened to a snug tight condition that is at least the minimum proper tension and verified using a calibrated tension measuring device.
iv. Shall be slip critical high strength bolts.
v. All faying surfaces of connections with high strength fasteners shall be prepared as required for Class A per the 2002 AISC Seismic Provision, Section 7.2.
c. Filler metal properties and specifications shall be as follows:
i. Electrodes shall be of a low-hydrogen type conforming to AWS specifications as referenced in Table 7 on Sheet 2.
ii. Filler metals shall be classified for nominal 70 ksi tensile strength.
iii. The maximum permitted electrode diameter shall be per Table 5 on Sheet 2.
iv. Filler metals shall have a minimum Charpy V-Notch (CVN) toughness of 20 ft-lbf at -20°F using AWS A5 classification test methods.
v. The use of intermixed welds shall not occur unless it can be demonstrated by testing in accordance with AWS D1.1/D1.1M:2002 Section 4.
vi. The parameters established by the electrode manufacturer shall be reflected in the WPS.
d. Other materials not listed in UBC Standard 22-1 or LABC Chapter 35 are not permitted without specific approval from the Department. Steel having dual ASTM designation shall be clearly identified on each specific plan detail.

- 3. Welding Processes
Structural welding shall be limited to the Shielded Metal Arc Welding or Flux Cored Arc Welding processes.

- 4. Base Metal Repairs or Restorations
Any repair or restoration of base metal shall comply with all of the following:
a. AWS D1.1/D1.1M:2002, Section 5.26, and ASTM A6/A6M-02, Section 9.2, 9.3, 9.4 and 9.5,

- b. Engineer of Record shall review and approve the WPS for repair procedures prior to welding,
c. All welding shall be performed using low-hydrogen process or with SMAW using low-hydrogen electrodes,
d. Provide continuous visual inspection by the Deputy Inspector, and
e. Provide non-destructive testing.

- 5. Deviations From the Standard Quality Assurance Plan
When deviations from the Standard QA Plan are made, comply with all of the following:
a. Deviations from this Standard QA Plan shall be approved by the Deputy Inspector and City Building Inspector prior to commencement of work.
b. Alternate procedures, specifications, or details shall be reviewed and approved by the Engineer of Record.
c. Supplemental testing and additional specifications may be required to approve alternate procedures, specifications, or details.
d. Conformance with all applicable provisions of the 2002 LABC and AWS D1.1/D1.1M:2002 is required.

IV. QUALITY ASSURANCE

- 1. Certification
a. Inspectors shall be LADBS Certified Deputy Inspectors per LABC Section 1701.2 and Information Bulletin P/BC 2002-035 "Regulations Regarding Registration for Deputy and Controlled Activities Inspection." Employment shall be in accordance with Information Bulletin P/BC 2002-034 "Employment and Duties of a Registered Deputy Inspector."
b. Welders shall be LADBS Certified Welders for the Structural Steel classification per LABC Section 1701.18.1, 2205.10, and Information Bulletin P/BC 2002-045 "Welder Certification Rules and Regulations."
c. Shop welds shall be performed in an LADBS certified Fabricator's Shop per LAMC Section 96.204(g) and Information Bulletin P/BC 2002-042 "Application for Approval as Fabricator."
d. Technicians performing NDT shall be certified for Level II in accordance with ASNT SNT-TC-1A 2001 Edition by a Testing Agency approved per LAMC Section 98.0503 and Information Bulletin P/BC 2002-058 "Guidelines for Recognition of Testing Agencies."

- 2. Pre-Construction Meeting
a. The Owner (or owner's representative) shall arrange a pre-construction meeting(s) with the Engineer of Record (or Structural Observer designated by the Engineer of Record), the Contractor (or affected Sub-Contractor), and the Deputy Inspector to discuss and review welding procedures, bolting procedures, and inspection requirements for all welding and bolting operations.
b. The City Building Inspector shall be notified of such meeting(s) and may participate at his/her discretion.
c. Meeting record(s) shall be included in the first report submitted to the City Building Inspector.

- 3. Structural Observation
Structural observation shall be performed in accordance with Information Bulletin P/BC 2002-024. The Structural Observer shall:
a. Perform structural observation listed in Table 6 on Sheet 2.
b. Perform structural observation prior to placement of decking, covering by fireproofing, encasement in concrete or placement of other finishes.
c. Submit observation report(s) to the City Building Inspector at each stage observed and upon completion of the structural system.
d. State in the report that the steel moment frame system substantially conforms with the approved structural plans and specifications.
e. Use the Department's Structural Observation Report Form to report all observations. Structural Observation Report Form can be obtained at www.ladbs.org, keyword "Information Bulletin P/BC 2002-024".

- 4. Deputy Inspection
The following are the basic Quality Assurance responsibilities of the Deputy Inspectors:
a. Arrive on the job in sufficient time to verify the permit information, check for prior inspections and/or approvals by the City Building Inspector or previous Deputy Inspectors, check the quality of all materials and become familiar with the approved structural plans and specifications.
b. Verify that structural steel delivered is from a fabricator currently licensed by the Department.
c. Identify material from an offsite fabricator in accordance with LABC Section 2203 and compare to the approved plans and specifications.
d. Verify that each steel piece is labeled with the approved fabricator's shop name and license number.
e. Visual check shop welds, joint preparation, faying surfaces, indent stamps and color codes of high strength steel, excessive mill scale or lamination, and dimensional conformity with the approved plans.
f. Ensure that welding complies with AWS D1.1/D1.1M:2002.
g. Inspect, before any welding begins, joint preparation, fit-up, condition of surfaces to be welded, storage and use of electrodes, current license of all welders, and voltage/amperage of welding machines.
h. Measure voltage/amperages near the arc with a hand held calibrated averaging type meter. The meter shall be calibrated not less than once a year. This equipment shall be used by the Fabricator, Erector, and Deputy Inspector.
i. During welding operation, provide continuous inspection particularly on multiple pass welds to assure that each pass has been prepared correctly, preheat and interpass temperatures are maintained and that finished welds shall be the correct size and without rejectable discontinuities.
j. Verify type and size of bolts and washers, check mill certificates, and verify faying surfaces are free of burrs, scale, rust, grease or anything that may inhibit full contact.
k. Verify connections involving high strength bolts and welds are fabricated and erected in a sequence specified by the Engineer of Record.
l. Verify high strength bolts are not welded or damaged by preheating.
m. Verify washers are always installed with all bolts, except A-490 bolts which require washers under both elements.
n. Verify that any deficiency noted in the Structural Observation Report Form has been corrected.

- o. Verify the Engineer of Record has approved the written welding procedure specification (WPS) prepared by the Fabricator or Erector. The WPS shall include the following:
i. All applicable code requirements, this Standard Plan, and any other information necessary to produce the welds.
ii. List the applicable base metal types and thicknesses.
iii. List the welding joint details, including joint type, weld type, joint geometry, and applicable dimensions. Individual weld passes shall be identified in sketches and numbered to identify the sequence of their deposition (see Detail 13 on Sheet 3 for example). The sketches shall identify the maximum layer thicknesses and bead widths. In no case shall layer thicknesses exceed 1/4 inch nor shall the maximum bead widths exceed 5/8 inch.
iv. List the welding processes.
v. Specify the required welding positions.
vi. List the filler metal per AWS D1.1 for electrode specification and classification (see Table 7 on Sheet 2), as well as information regarding shielding material to be used.
vii. Indicate the minimum preheat and interpass temperatures (see Table 4 on Sheet 2) and post weld heat treatment.
viii. List all applicable electrical characteristics for the welding process employed. WPS shall clearly indicate the specific values required for each welding pass. These electrical characteristics shall include at minimum the following:
(1) Electrode diameter (see Table 5 on Sheet 2),
(2) Type of current, and acceptable ranges of current measured in amperage,
(3) Voltage,
(4) Travel speed (range), and
(5) Amperage, voltage and electrode extension (as applicable)
shall be within the filler metal manufacturer's recommendations.
ix. A copy of the electrode manufacturer's technical information with ID # listed shall be attached to the WPS.
p. Weld joints not conforming to Chapter 3 of AWS D1.1/D1.1M:2002 must be tested by an approved testing agency and accepted by both the Engineer of Record and the Department's Material Control Section before the weld is performed. Material Control Section can be contacted at:

LADBS Material Control Section
221 N. Figueroa St., Suite 1540
Los Angeles, CA 90012
(213) 482-0380 or 1-888-LA-BUILD

- q. Notify the Contractor, Engineer of Record, and City Building Inspector of any deviations or non-compliance with the approved WPS, plans or specifications.
r. "Deputy Inspection Report Form B-94" shall be submitted on a weekly basis to the City Building Inspector and Engineer of Record, unless determined otherwise by the City Building Inspector.
s. During the execution of the work, the Deputy Inspector shall not undertake or engage in any other task or occupation which will interfere with the proper performance of the duties of such inspection.

- 5. Electrode Storage and Atmospheric Exposure
a. Electrodes are considered to be exposed to the atmosphere if:
i. the manufacturer's sealed electrode containers or packaging are opened or damaged, or
ii. outside of baking or storage ovens.
b. Modification or lubrication of electrodes are not permitted.
c. Drying of electrodes in baking or storage ovens are permitted as recommended by the manufacturer.
d. Electrodes shall be identified to facilitate monitoring of total atmospheric exposure time.
e. Storage and atmospheric exposure of AWS A5.1-91/A5.5-96 low-hydrogen SMAW electrodes shall be in accordance with AWS D1.1/D1.1M:2002 Section 5.3.2.
f. FCAW electrodes not consumed within 24 hrs of accumulated atmospheric exposure time shall not be used. Manufacturer's recommendations that show that drying effectively removes moisture and restores electrodes to their designated diffusible hydrogen levels are permitted.
g. FCAW electrode welding suspended more than 8 hrs shall be removed from the machines and stored in an electrode wire baking or storage oven maintained at a temperature between 250° and 550°F, or as recommended by the electrode manufacturer.

- 6. Plastic Hinging Zone Protection
a. The plastic hinging zone shall be identified diagrammatically, as illustrated in Detail 15 on Sheet 3, on the structural plans by the Engineer of Record.
b. The Engineer of Record and Contractor shall be responsible for reviewing shop drawings of ALL relevant trades to ensure compliance. This shall be discussed and documented in pre-construction meetings.
c. The Contractor shall be responsible for developing a program to ensure that all workers on the project, including their subcontractors, are aware of and understand this requirement. Failure to comply with these requirements may cause the replacement of steel.
d. Plastic hinging zones shall be defined by permanent markings such as paint or ink.
e. A note, as illustrated in Detail 15 on Sheet 3, shall be prominently placed on the structural plans (general note sheet and adjacent to moment frame detail) and the construction documents of ALL trades.
f. Welded, bolted, screwed, or shot-in (powder driven) attachments for perimeter edge angles, shear studs, exterior facades, partitions, duct work, piping, or other connections shall not be permitted within the plastic hinging zones.
g. Any penetrations or damage from temporary welded attachments within the plastic hinging zones shall be repaired as required by the Engineer of Record.
h. Initially, the plastic hinging zone "Warning Sign", as illustrated in Detail 15 on Sheet 3, may be temporary. However, the temporary "Warning Sign" shall be replaced by a permanent "Warning Sign" before project completion. This sign and identification of the plastic hinging zone shall be maintained during construction; and may require repair after operations such as fireproofing.

- i. Signs shall be affixed to the beam and located within the plastic hinging zone. The City Building Inspector may accept alternate methods of attaching the "Warning Sign" to the plastic hinging zone.
7. Additional Charpy V-Notch Toughness (not required for OMF)
Welds at the locations indicated below shall be made with filler metal having a CVN toughness of 20 ft-lbf at -20°F AND 40 ft-lbf at 70°F as determined by test procedure prescribed in the AISC Seismic Provisions, Appendix X "Weld Metal / Welding Procedure Specification Toughness Verification Test."
a. Beam flanges to columns,
b. Single plate shear connections to columns,
c. Beam webs to columns, and
d. Column splices.

- 8. Non-Destructive Testing (NDT) Requirements
a. The minimum non-destructive testing at each weld joints or parts shall be conducted at the locations and frequencies as specified in Table 2 and Table 3 on Sheet 2 respectively.
b. A copy of each NDT report shall be provided to the Contractor, Engineer of Record, Deputy Inspector, and City Building Inspector with the following information:
i. Document the accepted and rejected welds, parts, or joints.
ii. Identify the tested weld by piece mark and location in the piece.
iii. Identify the tested weld location in the structure.
c. NDT Technician shall perform the following tasks:
i. Coordinate the NDT scope and schedule with the Deputy Inspector.
ii. Perform NDT in a timely manner, so as not to hinder construction work, and to detect welding problems soon after occurrence so that corrective measures will be taken by the Contractor.
iii. Mark the inspected and accepted welds, parts, and joints with a distinguishing mark or die stamp.

- d. Reduction Rate for NDT
i. The rate of UT testing on CJP groove welds may be reduced if approved by the Engineer of Record and the Department. The NDT rate for an individual welder or welding operator may be reduced to 25%, provided the reject rate is demonstrated to be 5% or less of the welds tested for the welder or welding operator. A sampling of at least 40 completed welds for a job shall be made for such reduction evaluation.
ii. The rate of MT testing on CJP groove welds may be reduced if approved by the Engineer of Record and the Department. The MT rate for an individual welder or welding operator may be reduced to 10%, provided the reject rate is demonstrated to be 5% or less of the welds tested for the welder or welding operator. A sampling of at least 20 completed welds for a job shall be made for such reduction evaluation. This reduction is not permitted on welds in the k-area, at repair sites, weld tab and backing removal sites and access holes.
iii. Reject rate shall mean the number of welds containing rejectable defects divided by the number of welds completed.

- 9. Documentations
The reports listed in Table 1 on Sheet 2 shall be submitted to the City Building Inspector.

V. WELDING PROCEDURES

- 1. Bottom Beam Flange Moment Connection Welding
Welding the bottom flange to the column flange shall be completed in the flat welding position with the following sequence:
a. Start welding from Side A (one side of the beam) with a maximum 1/4 inch thick root pass beyond the center of the joint on Side B (other side of the beam), reaching past the beam web through the weld access hole.
b. After the arc is initiated, electrode travel shall progress toward the edge of the Side A beam flange, terminating on the Side A weld tab.
c. The Side A root pass, and the root pass deposit on Side B, shall be thoroughly cleaned to allow the Deputy Inspector to verify that the resulting bead profile is suitable for obtaining good fusion by the subsequent root pass to be initiated from Side B. If the profile is not conducive to good fusion, the start of the first root pass shall be ground, gouged, chipped, or otherwise prepared to ensure adequate profile to achieve fusion.
d. Complete the root pass on Side B before any other weld passes are performed.
e. The arc shall be initiated at the start of the first Side A root pass, and electrode travel shall progress toward the edge of the Side B beam flange, terminating on the Side B weld tab.
f. The above sequence shall be repeated for subsequent weld layers, and each weld layer shall be completed on both sides of the joint before a new layer is deposited. The order of operations (Side A, then Side B, or vice versa) is not restricted and may vary for each weld layer. Weld passes shall be placed in horizontal layers. Each pass shall be thoroughly cleaned of slag and wire brushed. Each pass shall be visually inspected by the Deputy Inspector, as described above in Step (c).
2. Sequence for Welding at Multiple Locations
When welding occur at multiple locations of welded steel moment frame connections, the following sequence shall be followed:
a. Weld both top and bottom beam flanges prior to any supplemental welding to the beam web or shear tab.
b. Engineer of Record shall review and approve all field welding sequences prior to the start of work.
c. Field welding of web shear plates with bolts shall occur after field welding of beam flanges to column flange.
d. High strength bolts shall be in the snug tight condition prior to welding.
e. High strength bolts shall be fully tensioned upon completion of all welding activities.
3. Welding Technique
a. Stringer beads shall be used during all welding operations. Maximum bead width, bead thickness, and layer thickness shall be considered. Weaving is not permitted, except when the WPS approved by the Engineer of Record limits electrode oscillation transverse to the weld axis to a maximum of:

- i. 3d for 1G/1F, 2G/2F, and 4G/4F weld positions, or
ii. 5d for the 3G/3f position, where d = electrode diameter.
b. Welding layers should progress from the face of the column flange outward toward the groove face of the beam flange as illustrated in Detail 13 on Sheet 3.
4. Preheat and Interpass Temperature
a. The minimum preheat and interpass temperature requirements in Table 4 of Sheet 2 shall be observed. Special attention shall be given to AWS D1.1/D1.1M:2002 Section 3.5.1 and Section 5.6 for the thickness of the base metal to be welded.
b. Preheat and all subsequent interpass temperatures shall be maintained during the welding operation for a distance at least equal to the thickness of the thicker welded part, but not less than 3", in all directions from the point of welding.
c. Where plates are of different thickness, the higher minimum preheat and interpass temperature requirements of the thicker plate shall govern.
d. Maximum preheat and interpass temperature shall not exceed the lesser of:
i. 550°F, or
ii. The maximum temperature recommended by the manufacturer.

- 5. Post Weld Heat Treatment
Post weld heat treatment may reduce cracking tendencies due to possible hydrogen embrittlement. Post weld heat treatment shall be provided as follows:
a. Apply heat in the 400°F to 600°F range after completion of welding.
b. Complying with the conditions of AWS D1.1/D1.1M:2002 Section 3.14 and Section 5.8.
c. Alternatively, the use of insulating blankets after the completion of welding is permitted to control the cooling of the connection to ambient temperature.

VI. WELDING AND FABRICATION DETAILS

- 1. Base Metal Joint Preparation
a. Base metal preparation shall be in comply with AWS D1.1/D1.1M:2002 Section 5.15.
b. All beam flange to column flange welds are to be made with an AWS prequalified CJP groove welded joint detail.
c. Bevel, fit-up and detail tolerances shall be as required by the selected prequalified welded joint detail.
d. Whenever possible, use the AWS prequalified CJP groove welded joint detail as illustrated in Detail 14 on Sheet 3 and the following:
i. Use single bevel CJP groove welds made with a 30° groove angle or double bevel CJP groove welds when flange thickness exceed 1-1/2 inch.
ii. "As Fit-Up" and "As Detailed" shall be the maximum tolerances.
iii. Meet all prequalified WPS variables in Table 5 on Sheet 2.

- 2. Weld Access Hole
a. Where weld access holes are provided, they shall be detailed as illustrated in Detail 12 on Sheet 3.
b. Notches and gouges shall be repaired following a WPS approved by the Engineer of Record.
c. Weld access holes shall be prepared by grinding to a suitable finish in accordance with AISC LRFD Specification Section J1.6 and provided with a minimum radius of 3/8 inch as illustrated in Detail 12 on Sheet 3.

- 3. Backing Bar
a. Backing bar used in connections with a CJP groove weld of beam flange to column flange shall be removed except that top flange backing bar attached to the column by a continuous fillet weld on the edge below the CJP groove weld need not be removed.
b. Following removal of backing bar, the root pass shall be backgrounded to sound weld metal, and back welded. A reinforcing fillet weld with a minimum leg size of 5/16 inch or the root opening plus 1/16 inch, whichever is larger, shall be provided. The reinforcing fillet weld need not be ground.
c. When backing bar is other than AWS D1.1/D1.1M:2002 Table 3.1 and Section 5.2.2 approved base metal is used, the following shall apply:
i. Ceramic, flux or glass tape may be used provided the manufacturer's recommendations are followed.
ii. When a non-metallic backing bar is used, the WPS and the Welder shall be qualified using the type of backing bar intended for welding.
iii. Nonferrous metallic (e.g. copper) backing materials are not permitted.

- 4. Weld Tab
a. Weld tabs shall be aligned parallel to the joint preparation.
b. No weld dams are allowed.
c. Weld tabs shall extend beyond the edge of the joint a minimum distance equal to the part thickness, but not less than one inch.
d. Weld tab shall be removed upon completion of the welded joint as follows:
i. No more than 1/8 inch beyond the edge of the joint shall remain; except at continuity plate where up to 1/4 inch is acceptable.
ii. Edges of the weld tab shall be finished to a surface roughness value of 500 micro inch or better. Grinding to a flush condition is not required.
e. Gouges and notches are not permitted. The transitional slope of any area where gouges and notches have been removed shall not exceed 1:5.
f. Material removed by grinding that extends more than 1/16 inch below the surface of the base metal shall be filled with weld metal. The contour of the weld at the ends shall provide a smooth transition, free of notches and sharp corners.

- 5. Continuity Plate
a. Continuity plates shall be detailed as illustrated in Detail 11 on Sheet 3.
b. The weld attaching the continuity plate to the column flange shall be as follows:
i. Use a CJP groove weld for the full length of the groove preparation.
ii. When backing bars are omitted, the root shall be backgrounded and back welded.
iii. When backing bars are used and remain in place, backing bars shall be attached to the column flanges with a reinforcing fillet weld.
iv. Fillet weld shall not be used to connect backing bars to continuity plates.
v. The fillet weld size need not exceed the minimum size requirements of AWS D1.1/D1.1M:2002 Table 5.8.

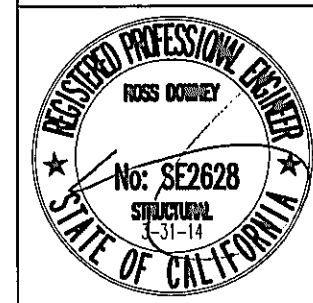
SITE ADDRESS:

OWNER:

STANDARD QUALITY ASSURANCE PLAN
For Steel Moment Frames

The specifications and illustrative details presented in this Standard Quality Assurance Plan have been prepared in accordance with recognized engineering principles and are for general information only. This Standard Quality Assurance Plan is not intended to constitute a contract. It is the responsibility of the Engineer of Record to ensure that the project is completed in accordance with the applicable codes and regulations. The Engineer of Record shall be responsible for the application of all of the specifications and illustrative details to the project. The Engineer of Record shall be responsible for the interpretation of the Standard Quality Assurance Plan. The Engineer of Record shall be responsible for the interpretation of the Standard Quality Assurance Plan. The Engineer of Record shall be responsible for the interpretation of the Standard Quality Assurance Plan.

TWO STORY SINGLE FAMILY RESIDENCE
901 LAUREL C'YVN
LA, CA 90046 FOR:
L.I. INVESTMENTS, LLC



Engineer of Record

Date: 02/28/2005

Scale: Not to Scale

Sheet:

Sheet 1 of 3 (Cont. S1.3B)

Date: 11-27-12

Scale:

Drawn: hrd

Job:

Sheet: S-13A

of

Table with 2 columns: REVISIONS, BY. Contains revision history information.

THESE PLANS DRAWINGS, DESIGNS AND SPECIFICATIONS ARE THE SOLE PROPERTY OF HRD ENGINEERING FOR THE EXCLUSIVE USE OF ITS CLIENTS FOR THE SPECIFIC PROJECT WITH APPROVAL BY SPECIFIC WRITTEN AUTHORITY. NO TRANSFER OF USE, NOT PERMITTED WITHOUT WRITTEN APPROVAL. COPYRIGHT © HRD ENGINEERING.

HRD ENGINEERING
STRUCTURAL DESIGN AND DRAFTING
7463 VARNA AVE., SECOND FLOOR
N. HOLLYWOOD, CA 91605
PHONE NO. (805) 431-5415 - FAX NO. (805) 888-4840

CONTINUED FROM SHEET 1

- c. Weld terminations near the end of the column flange tips may be completed using weld tabs as follows:
- Weld tabs may be steel or nonfusible material.
 - Weld terminations near the radius of the column need not be made using weld tabs. The use of small nonfusible weld tabs to assist in weld terminations is permitted.
 - Weld tabs shall be removed following completion of welding.
- d. Continuity plates may be welded to the column web with groove welds, fillet welds, or a combination of the two. Fillet welds shall terminate a minimum distance of 1/4 inch from each end of the joint.
6. Doubler Plate
Web doubler plates, as illustrated in Detail 2, 3, or 4 on Sheet 3, shall be welded using either Detail 5, 6, or 7 on Sheet 3.
7. Requirements for "k" Area
Welds shall terminate short of the "k" area for continuity plates as illustrated in Detail 11 on Sheet 3.
- VII. EXEMPTIONS
- Reduction from certain Quality Assurance components of this Standard Quality Assurance Plan, as listed in Part VII Item 2, are permitted for the following buildings or structures:
 - One or two family dwellings not more than 1 story in height and 2,500 sq ft of floor area,
 - Buildings or structures accessory to residential uses (such as carport, storage, garage), and
 - Miscellaneous structures (such as walkway, canopy, patio cover, gazebo, storage rack).
 - Buildings or structures, as listed in Part VII Item 1, are exempt from providing the following Quality Assurance components:
 - Electrode Storage and Atmospheric Exposure, Part IV, Item 5(f) and 5(g),
 - Plastic Hinging Zone Protection, Part IV, Item 6,
 - Additional CVN Notch Toughness Testing, Part IV, Item 7,
 - Non-Destructive Testing, Part IV, Item 8,
 - Preheat and Interpass Temperature, Part V, Item 4, or
 - Post Weld Heat Treatment, Part V, Item 5.

Table 7. PREQUALIFIED BASE METAL - FILLER METAL COMBINATIONS FOR MATCHING STRENGTH (1, 2, 3, 4)

BASE METAL		FILLER METAL		
Group	Steel Specification	Welding Process	AWS Electrode Specification	Electrode Classification
I	ASTM A36 < 3/4 in.	SMAW	A5.1	E70XX
			A5.5 (6)	E70XX-X
		FCAW	A5.20 (6)	E70XT-X, E7XT-XM
			A5.29 (6)	E70XTX-X, E7XTX-XM
II	ASTM A36 s 3/4 in. ASTM A572 Grade 50 ASTM A913 Grade 50 ASTM A992	SMAW	A5.1	E7015, E7016, E7018, E7028
			A5.5 (6)	E70XX-X
		FCAW	A5.20 (6)	E70XT-X, E7XT-XM
			A5.29 (6)	E70XTX-X, E7XTX-XM
RELATIONSHIP	BASE METAL(S)	FILLER METAL STRENGTH RELATIONSHIP REQUIRED		
Matching	Any steel to itself or any steel to another in the same group	Any filler metal listed in the same group		
	Any steel in one group to any steel in another	Any filler metal listed for a lower strength group [SMAW electrodes shall be the low-hydrogen classification]		
Under-Matching	Any steel to any steel to any group			

- NOTES:
- The base metal/filler metal strength relationships above shall be used to determine whether matching or under-matching filler metals are required. Refer to AWS D1.1/D1.1M:2002, Section 3.3.
 - Preheating of joints involving base metals of different groups shall be in conformance with the requirements applicable to the higher strength group.
 - When welds are to be stress-relieved, the deposited weld metal shall not exceed 0.05 percent vanadium.
 - Adapted with permission from the AWS D1.1 Committee on Structural Welding, Structural Welding Code - Steel, AWS D1.1/D1.1M: 2002, Miami: American Welding Society, Table 3.1.
 - FCAW electrodes with the -2, -2M, -3, -4, -7, -10, -11, -13, -14, G, -GS suffix shall be excluded and electrodes with the -11 suffix shall be excluded for thicknesses greater than 1/2 in.
 - Filler metals of alloy group B3, B3L, B4, B4L, B5, B5L, B6, B6L, B7, B7L, B8, B8L, B9, or any BXH grade in AWS A5.5 or A5.29 are not prequalified for use in the as-weld condition.

Table 5. PREQUALIFIED WPS REQUIREMENTS (1, 2, 3)

VARIABLE	POSITION OF WELD	WELD TYPE	SMAW	FCAW
Maximum Electrode Diameter	Flat (F)	Fillet (4)	5/16 in.	1/8 in.
		Groove (4)	1/4 in.	
		Root Pass	3/16 in.	
	Horizontal (H)	Fillet	1/4 in.	1/8 in.
		Groove	3/16 in.	
	Vertical (V)	All	3/16 in.	3/32 in.
Overhead (OH)		All	3/16 in.	5/64 in.
Maximum Current	All	Fillet		
	All	Groove weld root pass with opening	Within the range of recommended operation by the filler metal manufacturer and a WPS approved by engineer of record.	Within the range of recommended operation by the filler metal manufacturer and a WPS approved by engineer of record.
		Groove weld root pass without opening		
		Groove weld fill passes		
		Groove weld cap pass		
	Maximum Root Pass Thickness (6)	Flat (F)		3/8 in.
Horizontal (H)		All	5/16 in.	5/16 in.
Vertical (V)			1/2 in.	1/2 in.
Overhead (OH)			5/16 in.	5/16 in.
Maximum Fill Pass Thickness	All	All	3/16 in.	1/4 in.
	Maximum Single Pass Fillet Weld Size	Flat (F)		3/8 in.
Horizontal (H)		Fillet	5/16 in.	3/8 in.
Vertical (V)			1/2 in.	1/2 in.
Overhead (OH)			5/16 in.	5/16 in.
Maximum Single Pass Layer Width	All	Root opening >1/2 in.	Not applicable.	Split layers
		Any layer of width w		(6)

- NOTES:
- Applicable provisions of AWS D1.1/D1.1M:2002 Section 3 "Prequalification of WPSs" must be maintained for prequalified status of SMAW and FCAW WPSs.
 - Refer to Detail 13 on Sheet 3 for diagram of weld pass sequence.
 - Adapted with permission from the AWS D1.1 Committee on Structural Welding, Structural Welding Code - Steel, AWS D1.1/D1.1M: 2002, Miami: American Welding Society, Table 3.7.
 - Except root passes.
 - See AWS D1.1/D1.1M:2002, Section 3.7.2, for width-to-depth limitations.
 - In the F, H, or OH positions for nontubulars, split layers when the layer width w > 5/8 inch. In the V position for nontubulars or the 5G or 6G for tubulars, split layers when the width w > 1 inch.

Table 6. STRUCTURAL OBSERVATION CHECKLIST

STRUCTURAL OBSERVATION PROGRAM (Steel Moment Frame for Seismic Application)	
<input type="checkbox"/>	Orientation and placement of connected components.
<input type="checkbox"/>	Removal of backing bars, as required on the plans.
<input type="checkbox"/>	Removal of runoff tabs, as required on the plans.
<input type="checkbox"/>	Presence of continuity plates, as required on the plans.
<input type="checkbox"/>	Presence of doubler plates, as required on the plans.
<input type="checkbox"/>	Configuration and finish of weld access holes, if applicable.
<input type="checkbox"/>	Contour of RBS profile, if applicable.
<input type="checkbox"/>	Verify that no welded attachments occur in the plastic hinging region.
<input type="checkbox"/>	Review NDT and deputy inspection reports for general compliance.

- NOTES:
- Weld qualities shall be verified by the Deputy Inspector.
 - The structural observations listed in this Table are in addition to the structural observations that may be required on the structural plans.

Table 1. REPORTS TO BE SUBMITTED TO THE CITY BUILDING INSPECTOR

PREPARED BY	TYPE OF REPORT
1. Structural Observer(s)	<input type="checkbox"/> Structural Observation Reports
2. Deputy Inspector(s)	<input type="checkbox"/> Deputy Inspection Reports
3. NDT Technician(s)	<input type="checkbox"/> Non-Destructive Testing Reports

Table 2. NON-DESTRUCTIVE TEST LOCATIONS

REQUIRED LOCATIONS	OMF	IMF	SMF
1. CJP Groove Weld Ultrasonic test shall be performed on all CJP groove welds in materials 5/16 inch (8 mm) thick or greater. In addition, magnetic particle test shall be performed on all beam-to-column CJP groove welds.	B	A	A
2. "k" Area When welding of doubler plates, continuity plates, or stiffeners has been performed in the k-area, the web shall be tested for cracks using magnetic particle testing. The magnetic particle test area shall include the k-area base metal within 3 in. (75 mm) of the weld.	C	B	B
3. Beam Cope and Access Hole At welded splices and connections, thermally cut surfaces of beam copes and access holes shall be tested using magnetic particle testing, when the flange thickness exceeds 1-1/2 in. (38 mm) for rolled shapes.	C	B	B
4. Reduced Beam Section Repair Magnetic particle testing shall be performed on any weld and adjacent area of the RBS plastic hinge region that has been repaired by welding, or on the base metal of the RBS plastic hinge region if a sharp notch has been removed by grinding.	B	B	A
5. Base Metal Lamellar Tearing and Laminations at CJP Groove Weld Base metal thicker than 1-1/2 in. (38 mm) shall be ultrasonically tested for discontinuities behind and adjacent to the fusion line when the base metal is loaded in tension in the through thickness direction in tee and corner joints and the connected material is greater than 3/4 in. (19 mm). Any base metal discontinuities found within 1/4 of the steel surface shall be accepted or rejected on the basis of criteria of AWS D1.1 Table 6.2, where t is the thickness of the part subjected to the through-thickness strain.	B	B	A
6. End of Weld at Weld Tab Removal Site Magnetic particle testing shall be performed on the end of welds from which the weld tabs have been removed, except for continuity plate weld tabs.	C	B	B
7. PJP Groove Weld Ultrasonic testing shall be performed on PJP groove welds used in column splices with an effective throat of 3/4 in. (19.1 mm) thick or greater.	C	B	A

NOTE: A, B, and C are the frequencies of non-destructive tests listed in Table 3.

Table 3. NON-DESTRUCTIVE TEST FREQUENCY

	Frequency Designation		
	A	B	C
Ultrasonic Testing (UT)	100% of joints	50% of joints	25% of joints
Magnetic Particle Testing (MT)	50% of joints	25% of joints	Not Required

- NOTES:
- Refer to Table 2 for locations of non-destructive testing.
 - Rate of non-destructive testing may be reduced as permitted in Sheet 1, Part IV, Item 8(d).

Table 4. PREQUALIFIED MINIMUM PREHEAT AND INTERPASS TEMPERATURE

STEEL SPECIFICATION	WELDING PROCESS	THICKNESS OF THICKEST PART AT POINT OF WELDING (in.)	MINIMUM PREHEAT AND INTERPASS TEMPERATURE (°F)
ASTM A36 ASTM A572 Grade 50 ASTM A913 Grade 50 ASTM A992	SMAW with low-hydrogen electrodes, FCAW	1/8 to 3/4 incl.	32
		Over 3/4 to 1-1/2 incl.	50
		Over 1-1/2 to 2-1/2 incl.	150
		Over 2-1/2	225

- NOTES:
- Surfaces to be welded and surfaces adjacent to welds shall be free of moisture pursuant to AWS D1.1/D1.1M:2002 Section 5.15. Use a higher preheat temperature from this Table to remove moisture.
 - Adapted with permission from the AWS D1.1 Committee on Structural Welding, Structural Welding Code - Steel, AWS D1.1/D1.1M: 2002, Miami: American Welding Society, Table 3.2.

SITE ADDRESS:
OWNER:

STANDARD QUALITY ASSURANCE PLAN
For Steel Moment Frames

The specifications and illustrative details presented in this Standard Quality Assurance Plan are intended to provide a minimum level of quality assurance for steel moment-resisting frames. This Standard Quality Assurance Plan should not be used or relied upon for any specific application without modification by the Engineer or Architect of Record. By signing and sealing this Standard Quality Assurance Plan, the Engineer or Architect of Record assumes full responsibility for the design and construction of the project. The Engineer or Architect of Record shall be responsible for the selection and use of any of the specifications and illustrative details contained in this Standard Quality Assurance Plan and all liability arising from such use.

Engineer of Record
Date: 02/28/2005
Scale: Not to Scale
Sheet: Sheet 2 of 3 (Cont. S1.3C)

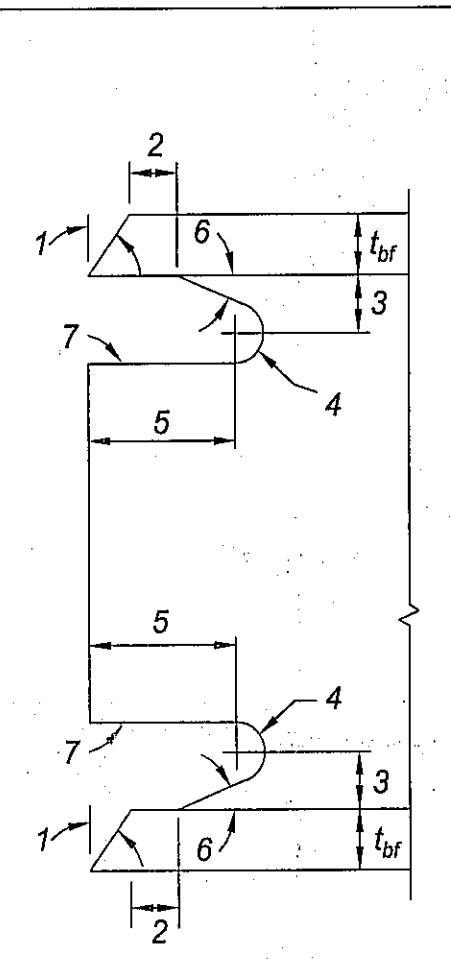
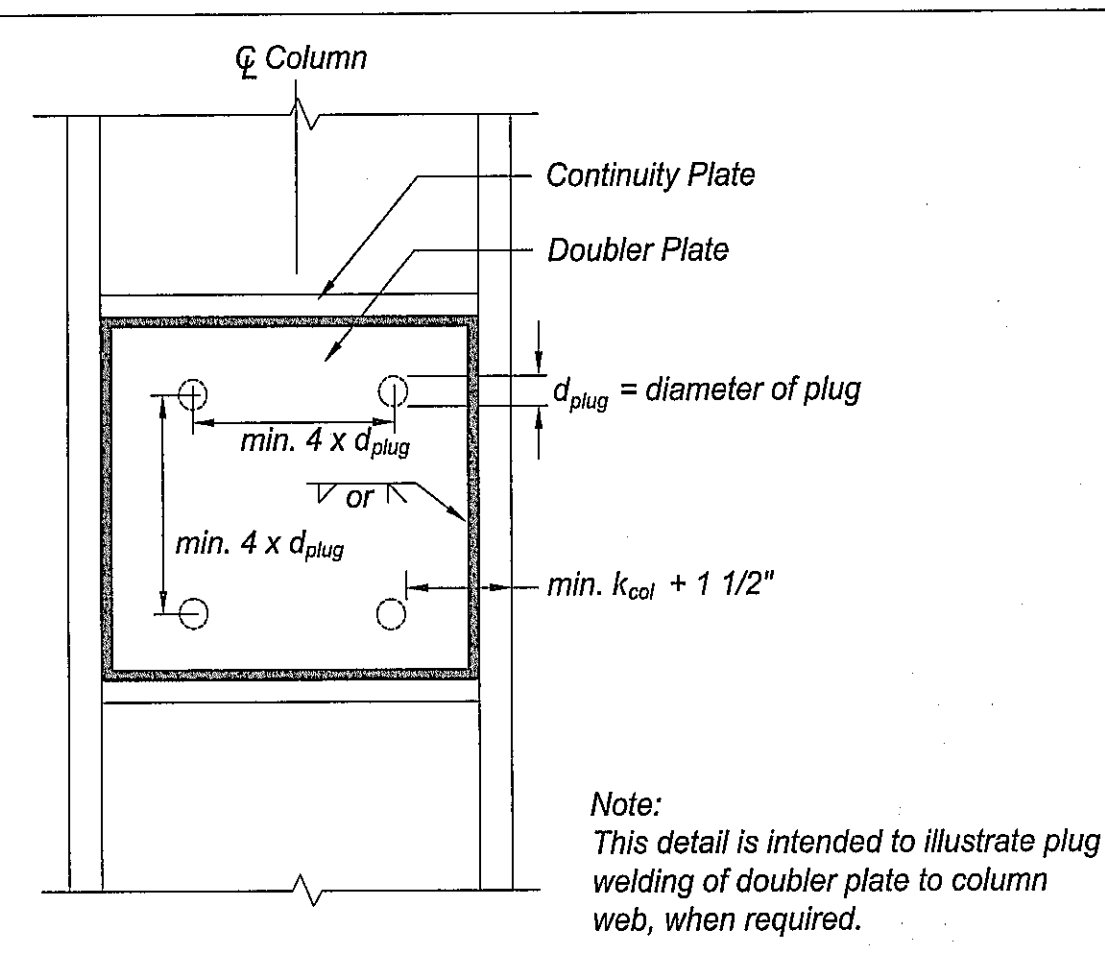
REVISIONS BY

HRD ENGINEERING
STRUCTURAL DESIGN AND DRAFTING
7463 VARNA AVE., SECOND FLOOR
N. HOLLYWOOD, CA 91605
PHONE NO. (805) 431-5415 - FAX NO. (805) 889-4840

TWO STORY SINGLE FAMILY RESIDENCE
901 LAUREL CYN.
LA, CA 90046 FOR:
L.I. INVESTMENTS, LLC

REGISTERED PROFESSIONAL ENGINEER
NO. SC2628
STATE OF CALIFORNIA
1984

Date: 02/28/2005
Scale: Not to Scale
Drawn: hrd
Job: S-1.3B
Sheet: S-1.3B



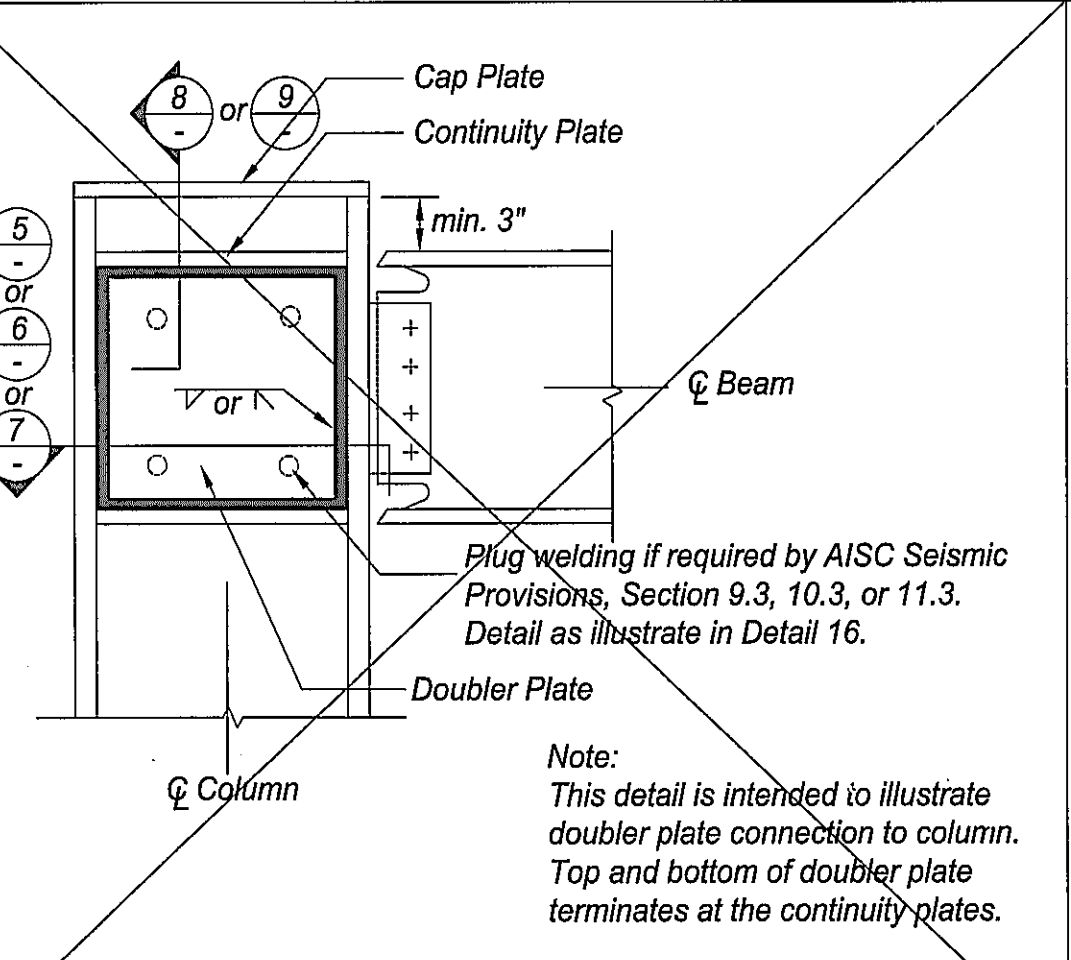
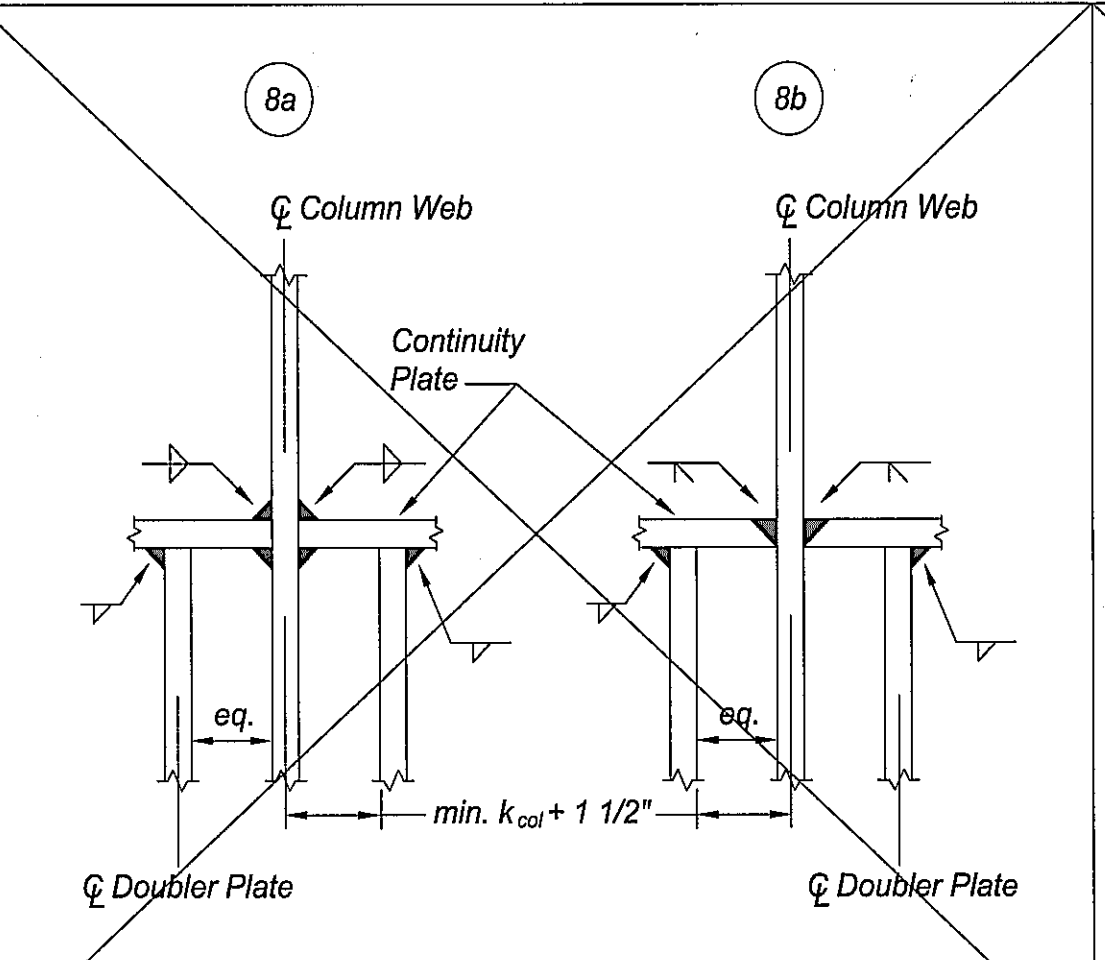
Notes:

- Bevel as required by a selected AWS prequalified CJP groove welded joint detail.
- Larger of t_{br} or $1/2"$ (plus $1/2 t_{br}$, or minus $1/4 t_{br}$).
- $3/4 t_{br}$ to t_{br} , $3/4"$ minimum ($\pm 1/4"$).
- $3/8"$ minimum radius (plus not limited, or minus 0).
- $3 t_{br}$ ($\pm 1/2"$).
- Tolerances shall not accumulate to the extent that the angle of the access hole cut to the flange surface exceed 25° .
- Weld access hole shall be ground smooth to a surface roughness value not to exceed 500 micro inch; and shall be free of notches and gouges.

Plug Welding of Doubler Plate to Column **Detail 16**
Scale: Not to Scale

Weld Access Hole Detail **Detail 12**
Scale: Not to Scale

Doubler Plate Welds to Continuity Plate **Detail 8**
Scale: Not to Scale



TYPICAL SHEET 3 GENERAL NOTES:

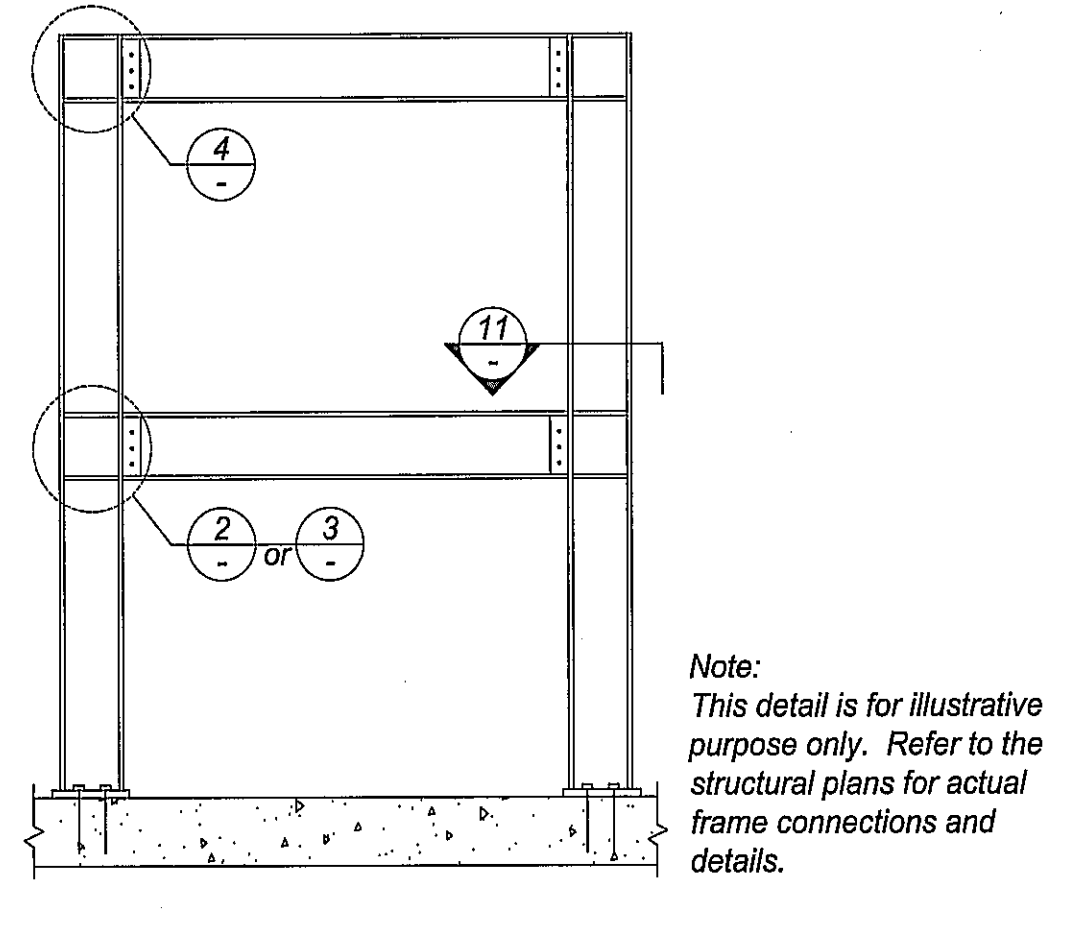
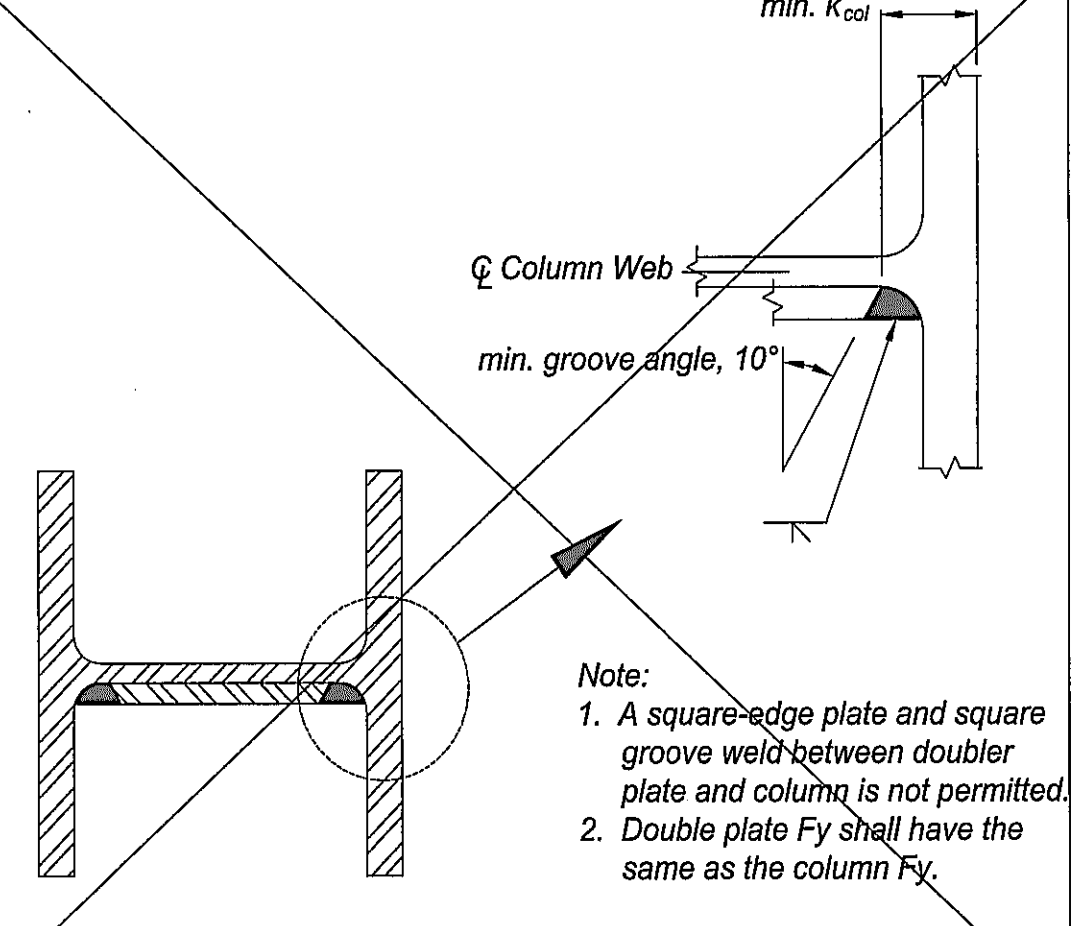
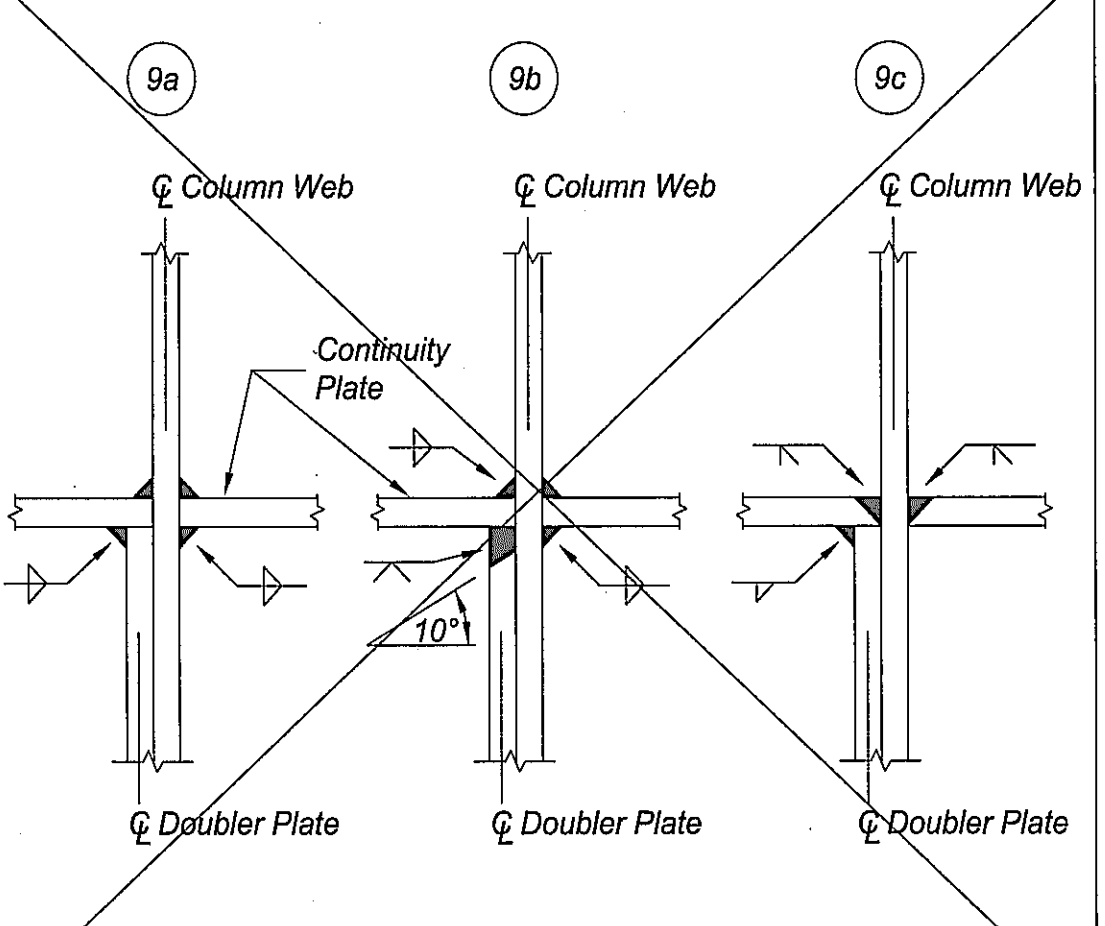
- The size of columns or beams, continuity or doubler plate thickness, size and length of fillet welds, beam to column moment connections, steel column frame to foundation connections, and length of plastic hinging zone shall be determined by the Engineer or Architect of Record.
- The Engineer or Architect of Record shall appropriately dimension and specify the minimum detailing information illustrated on this Sheet to the structural plans. The details illustrated herein shall not be used to substitute for actual structural details.
- The details provided herein are intended to illustrate the minimum fabrication and welding details that shall be reflected on the structural plans and details.

SITE ADDRESS:
OWNER:



Notes:

- For maximum fill pass thickness, see Table 5 on Sheet 2.
- For moment connection at bottom beam flange to column flange, the maximum root pass thickness shall not exceed $1/4"$ per Part V, Item 1. For welded connection at all other locations, see Table 5 on Sheet 2 for maximum root pass thickness.
- Root Opening, R, and Groove Angle, a, shall be per selected AWS prequalified CJP groove welded joint detail.
- Welding pass numbers are shown diagrammatically to indicate sequence. Quantity of passes may vary due to depth and/or position of weld. This figure is intended to illustrate recommended welding sequence for FCAW and SMAW welding process only.



STANDARD QUALITY ASSURANCE PLAN
For Steel Moment Frames

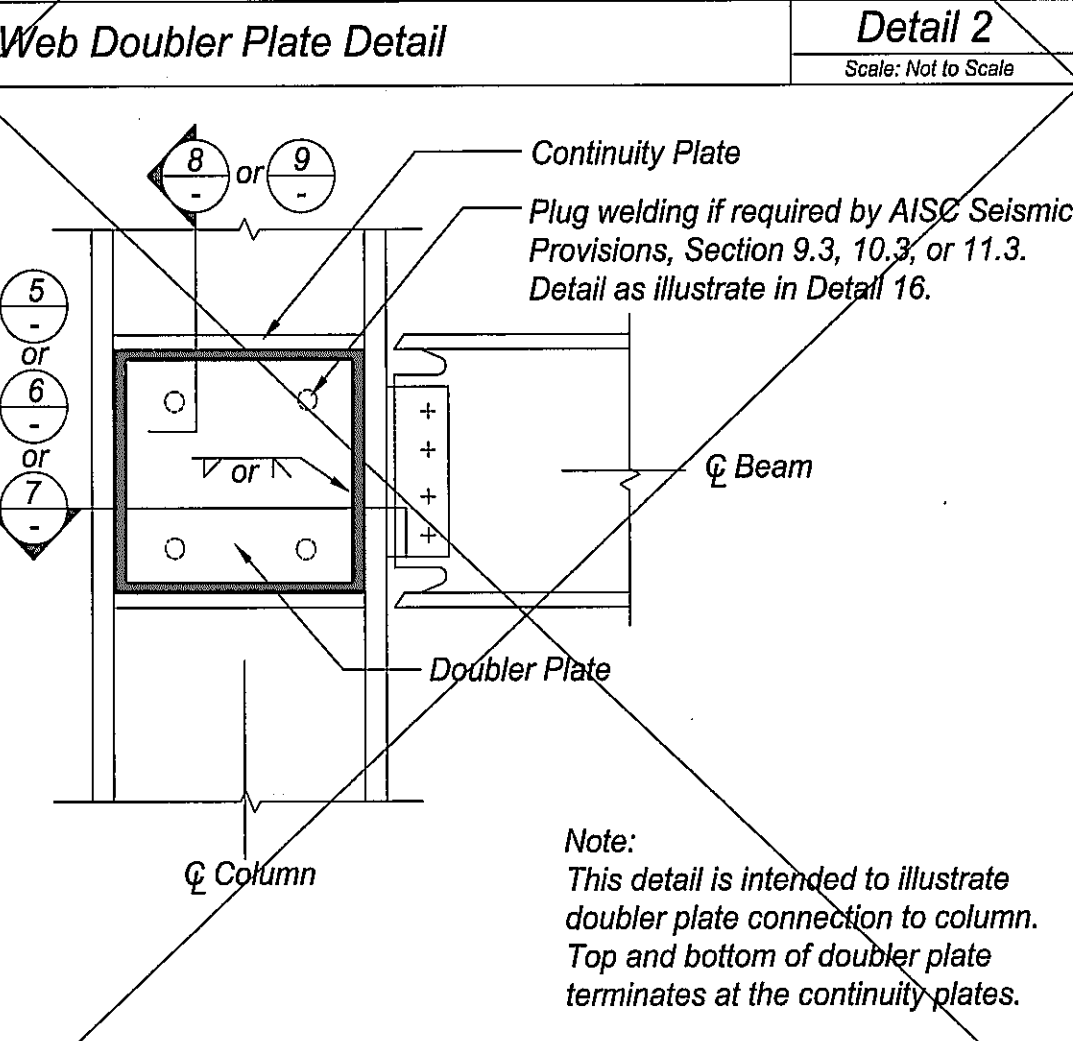
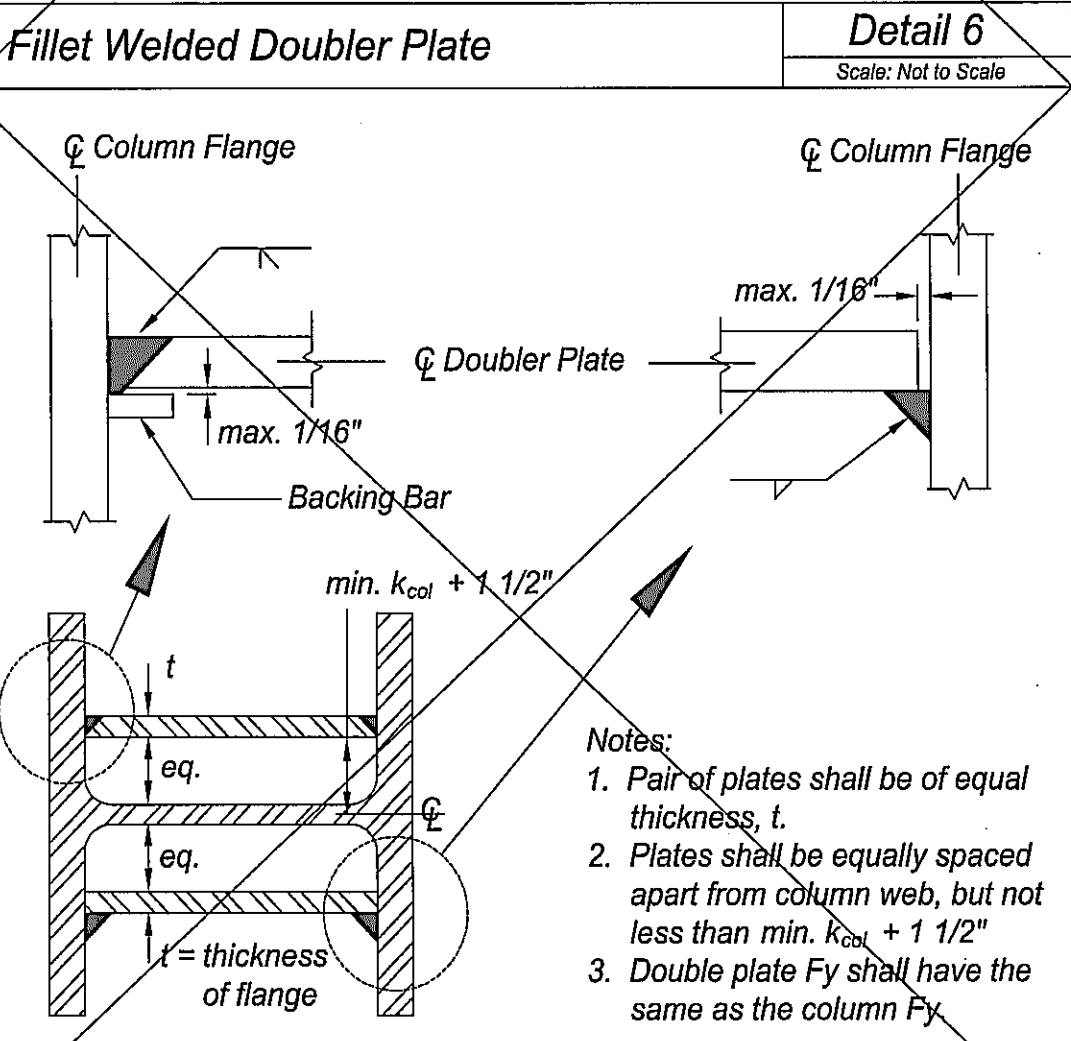
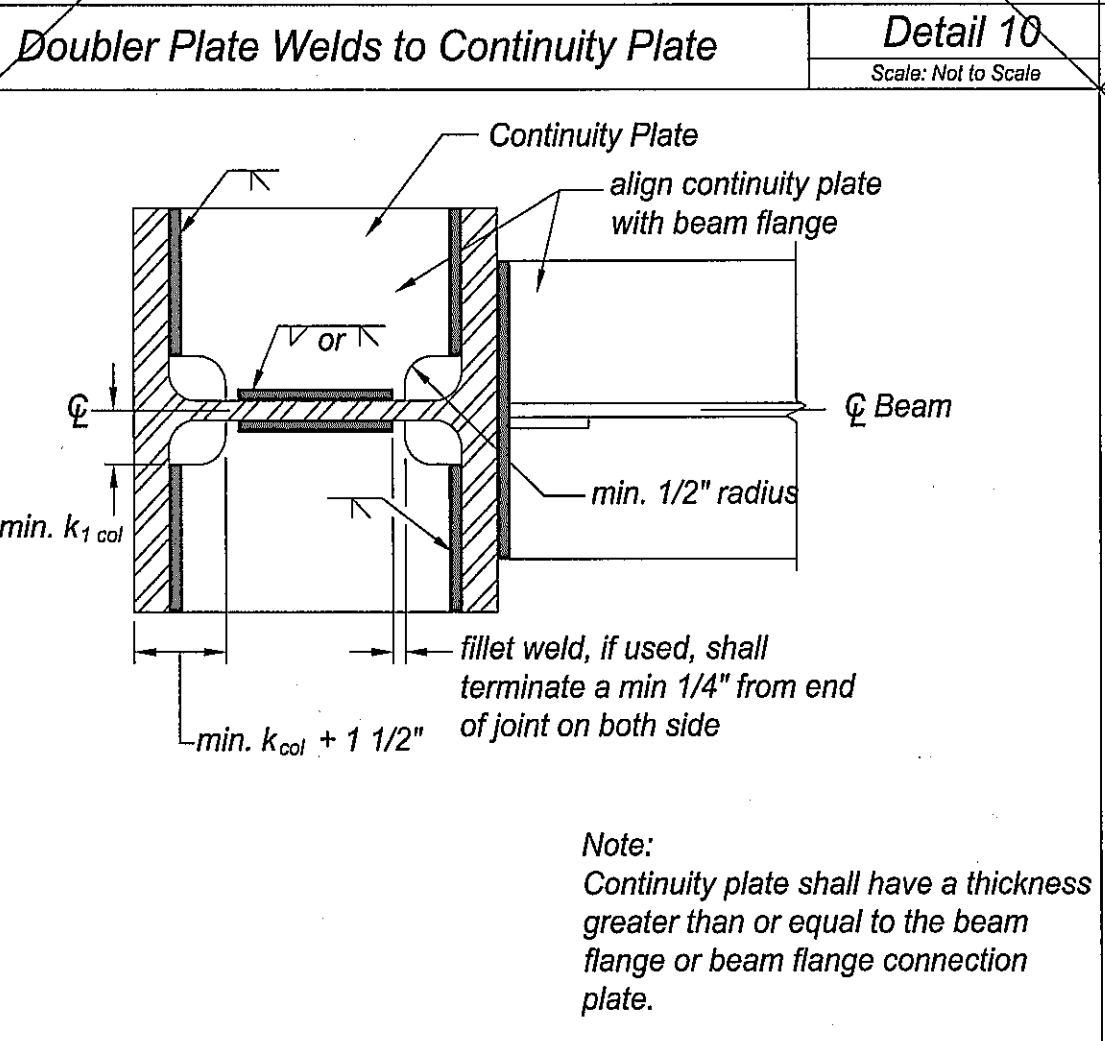
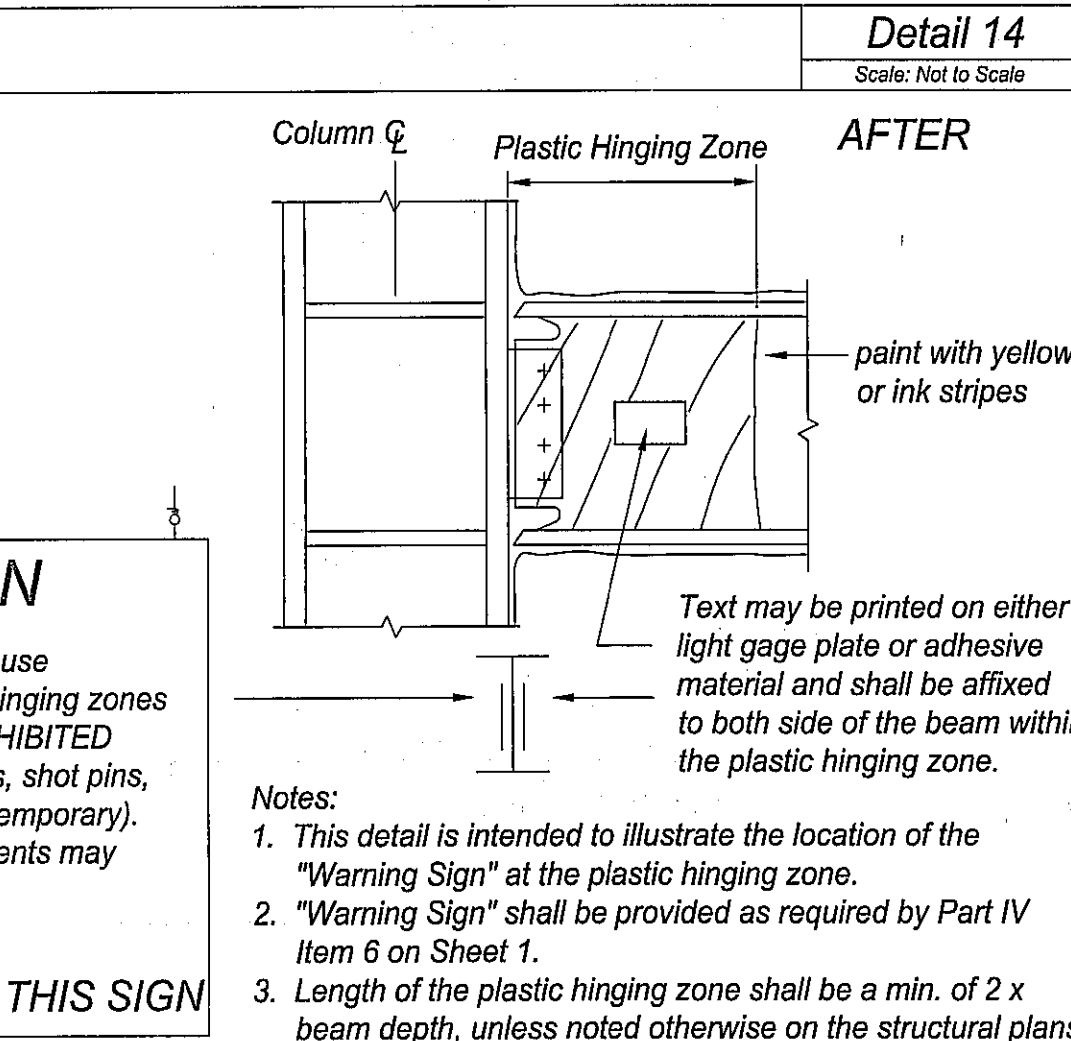
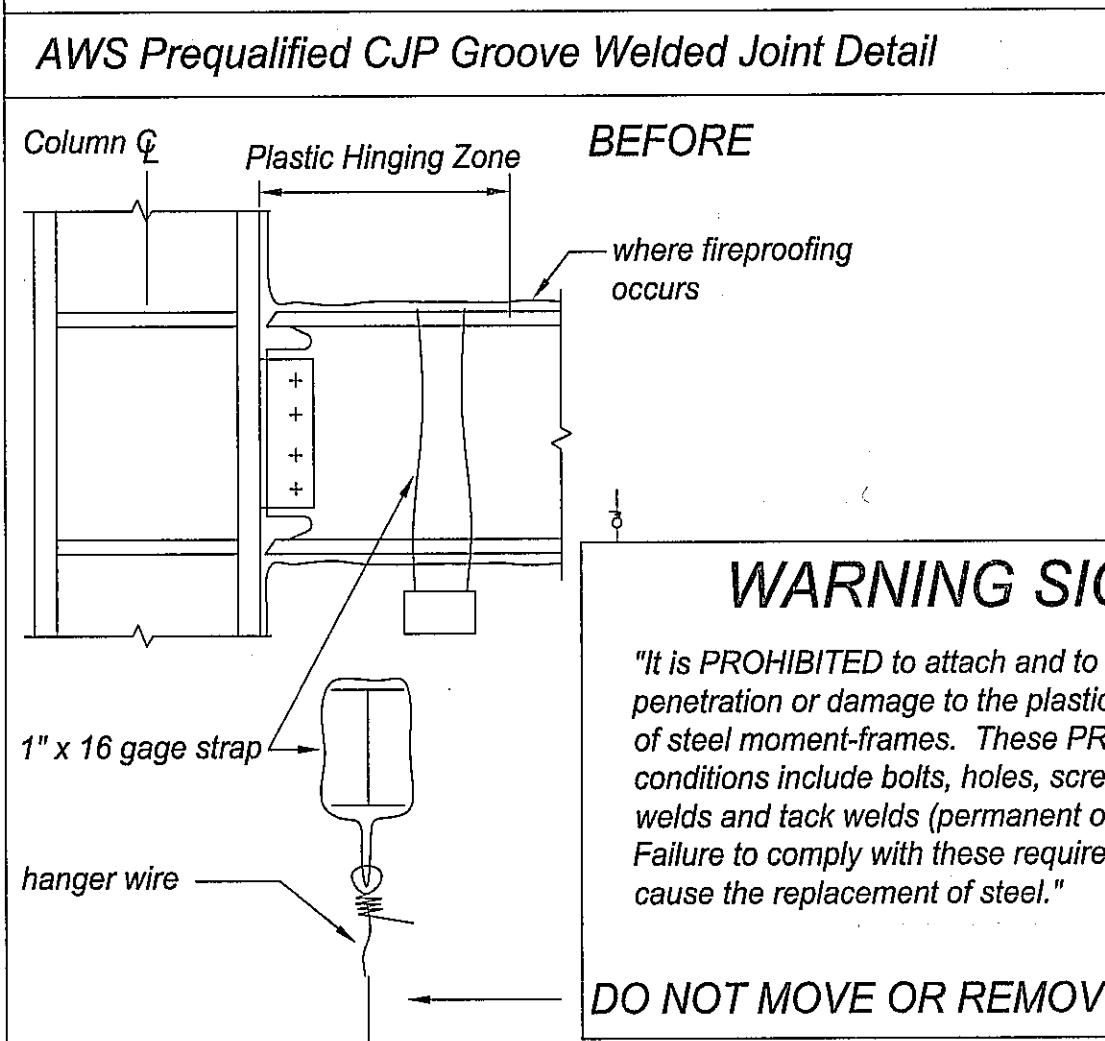
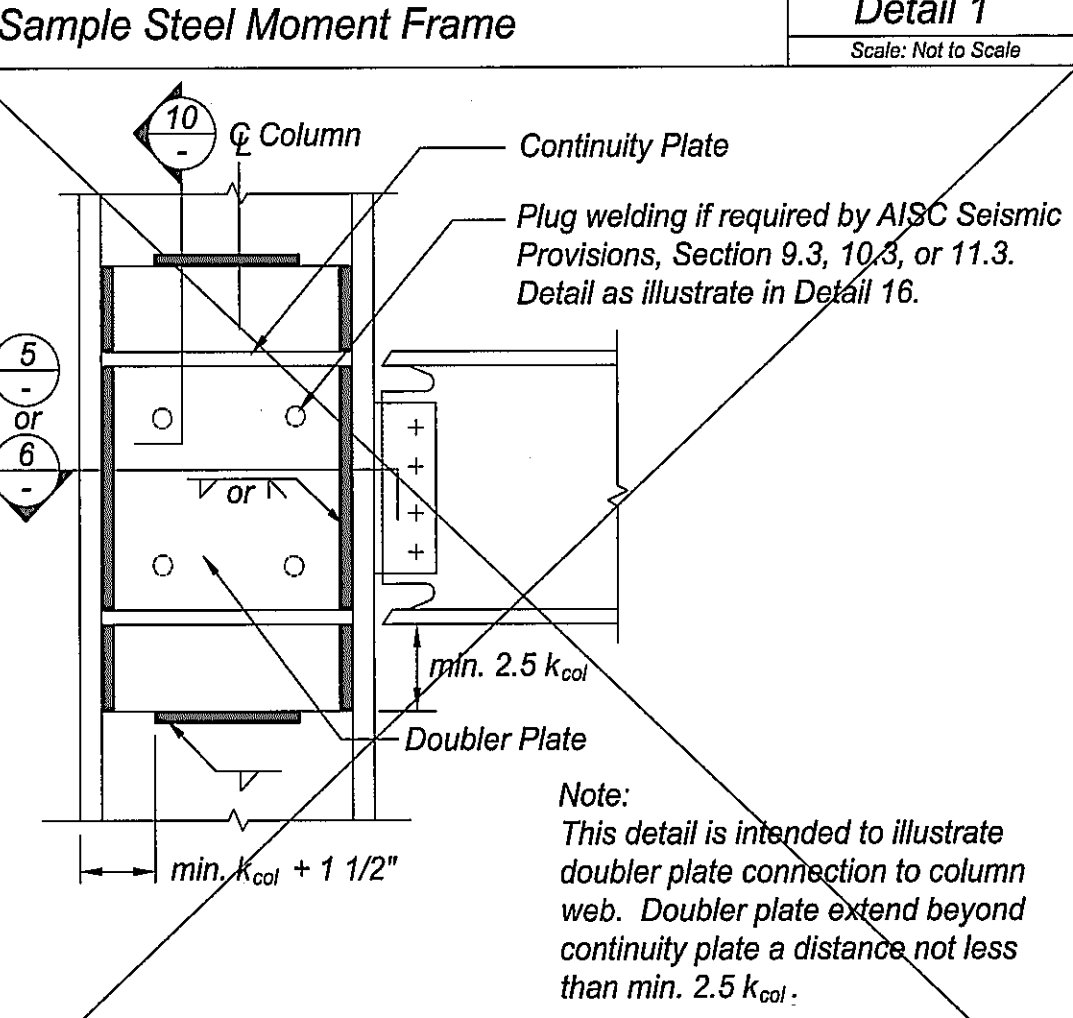
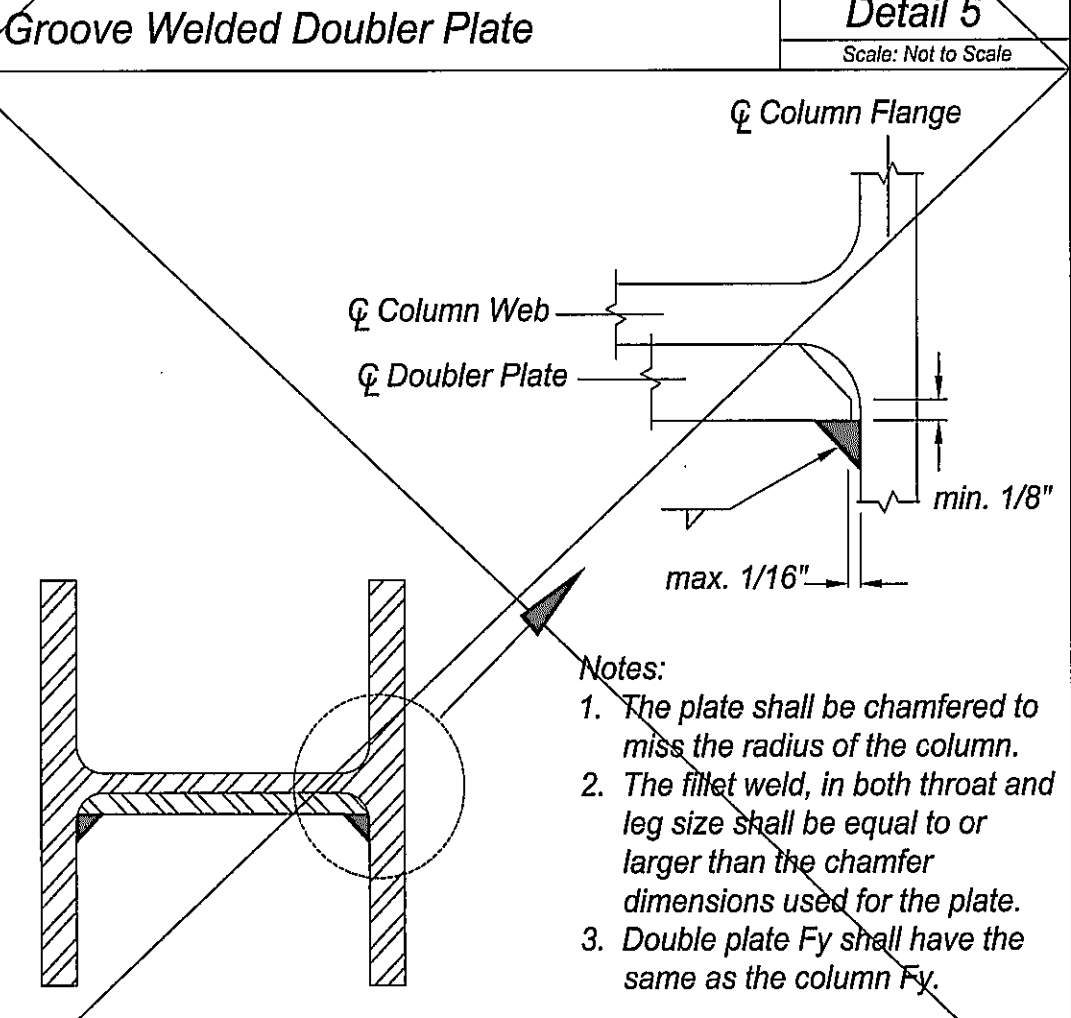
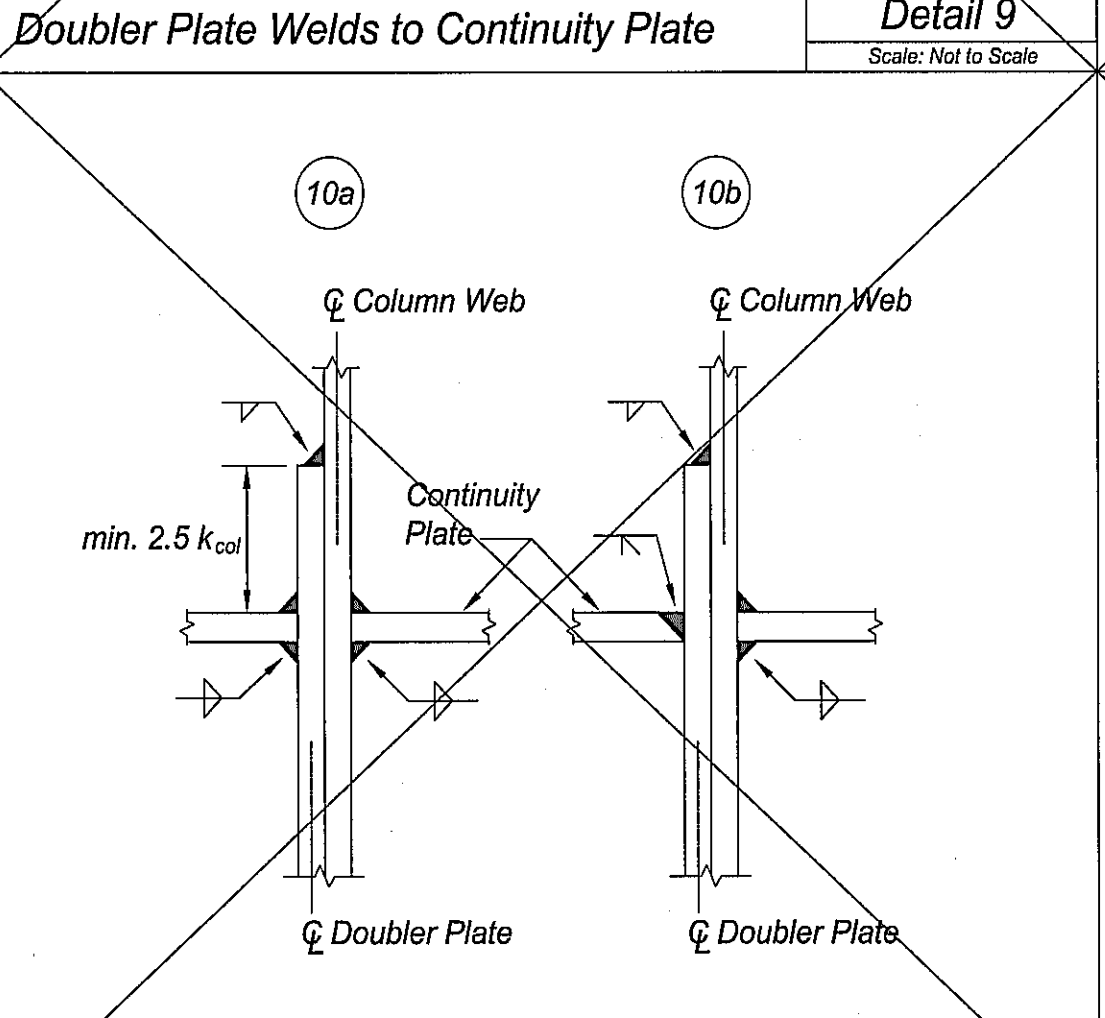
TOLERANCES

R = +1/16, -0	+1/4, -1/16
a = +10°, -0°	+10°, -5°

Welding Process	Joint Designation	Base Metal Thickness (U = unlimited)		Groove Preparation		Allowed Welding Positions	Gas Shielding for FCAW
		T ₁	T ₂	Root Opening	Groove Angle		
SMAW	TC-U4a	U	U	R = 1/4 a = 45°	All	F, V, OH	-
FCAW	TC-U4a-GF	U	U	R = 3/8 a = 30°	All	F	Required
				R = 3/16 a = 30°	All	F	Not required
				R = 3/8 a = 30°	All	F	Not required
				R = 1/4 a = 45°	All	F	Not required

Notes:

- Groove welds in corner and T-joints of cyclically loaded structures shall be reinforced with fillet welds equal to $T_1/4$, but need not exceed $3/8$ inch.
- For corner joints, the outside groove preparation may be in either or both members, provided the basic groove configuration is not changed and adequate edge distance is maintained to support the welding operations without excessive edge melting.
- Adapted with permission from the AWS D1.1 Committee on Structural Welding, Structural Welding Code - Steel, AWS D1.1/D1.1M: 2002, Miami: American Welding Society, Figure 3.4 pg. 92 (top left).

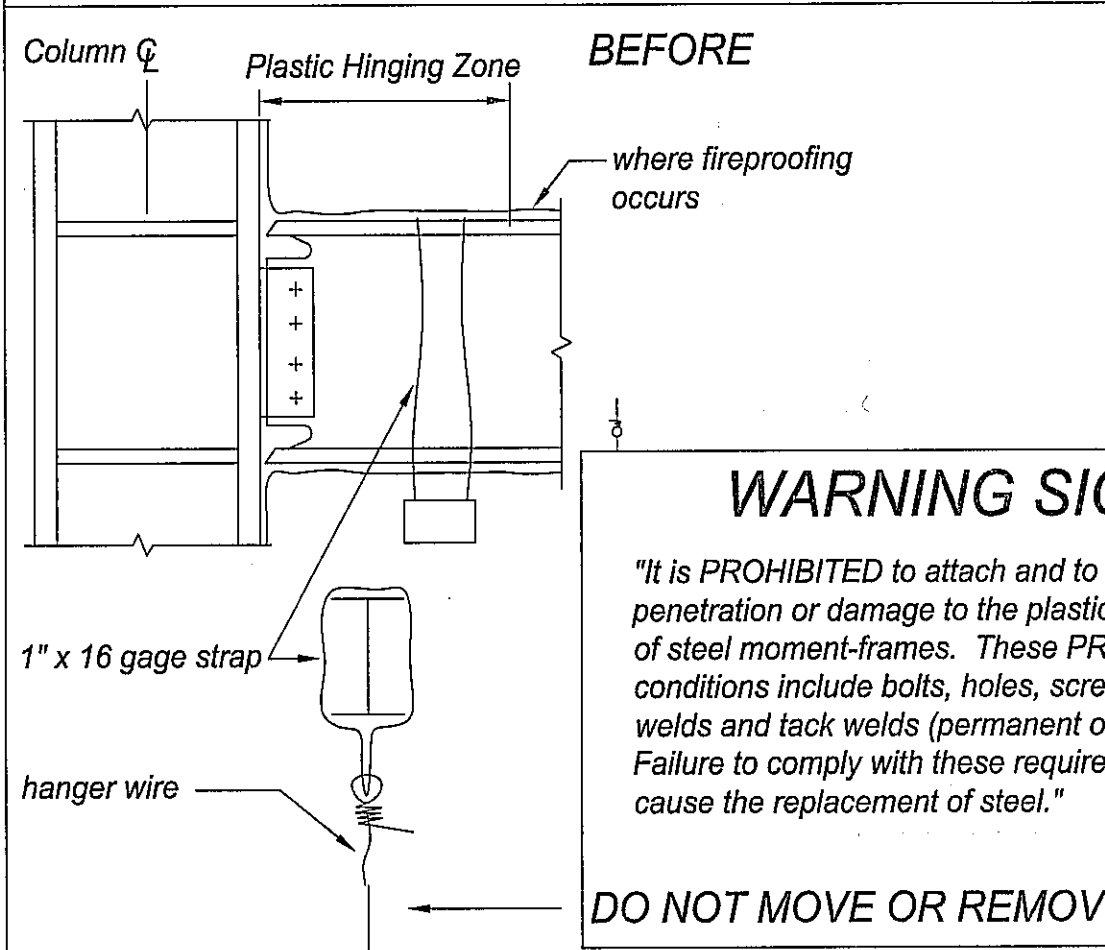


Warning Sign at Plastic Hinging Zone **Detail 15**
Scale: Not to Scale

Continuity Plate Detail **Detail 11**
Scale: Not to Scale

Groove or Fillet Welded Doubler Plate **Detail 7**
Scale: Not to Scale

Web Doubler Plate Detail **Detail 3**
Scale: Not to Scale



Notes:

- This detail is intended to illustrate the location of the "Warning Sign" at the plastic hinging zone.
- "Warning Sign" shall be provided as required by Part IV Item 6 on Sheet 1.
- Length of the plastic hinging zone shall be a min. of 2 x beam depth, unless noted otherwise on the structural plans.

Notes:

- The plate shall be chamfered to miss the radius of the column.
- The fillet weld, in both throat and leg size shall be equal to or larger than the chamfer dimensions used for the plate.
- Double plate Fy shall have the same as the column Fy.

Notes:

- Pair of plates shall be of equal thickness, t.
- Plates shall be equally spaced apart from column web, but not less than min. $k_{col} + 1/2"$
- Double plate Fy shall have the same as the column Fy.

Notes:

- This detail is intended to illustrate doubler plate connection to column. Top and bottom of doubler plate terminates at the continuity plates.

STANDARD QUALITY ASSURANCE PLAN
For Steel Moment Frames

THESE PLANS DRAWINGS, DESIGNS AND SPECIFICATIONS ARE THE SOLE PROPERTY OF HRD ENGINEERING AND DRAFTING. NO PART OF THESE PLANS DRAWINGS, DESIGNS AND SPECIFICATIONS SHALL BE REPRODUCED, COPIED, TRANSMITTED, OR IN ANY MANNER DISCLOSED TO ANY OTHER PERSON WITHOUT THE WRITTEN APPROVAL OF HRD ENGINEERING. COPYRIGHT © HRD ENGINEERING.

HRD ENGINEERING
STRUCTURAL DESIGN AND DRAFTING
7463 VARNA AVE., SECOND FLOOR
N. HOLLYWOOD, CA 91605
PHONE NO. (805) 431-5415 - FAX NO. (805) 888-4840

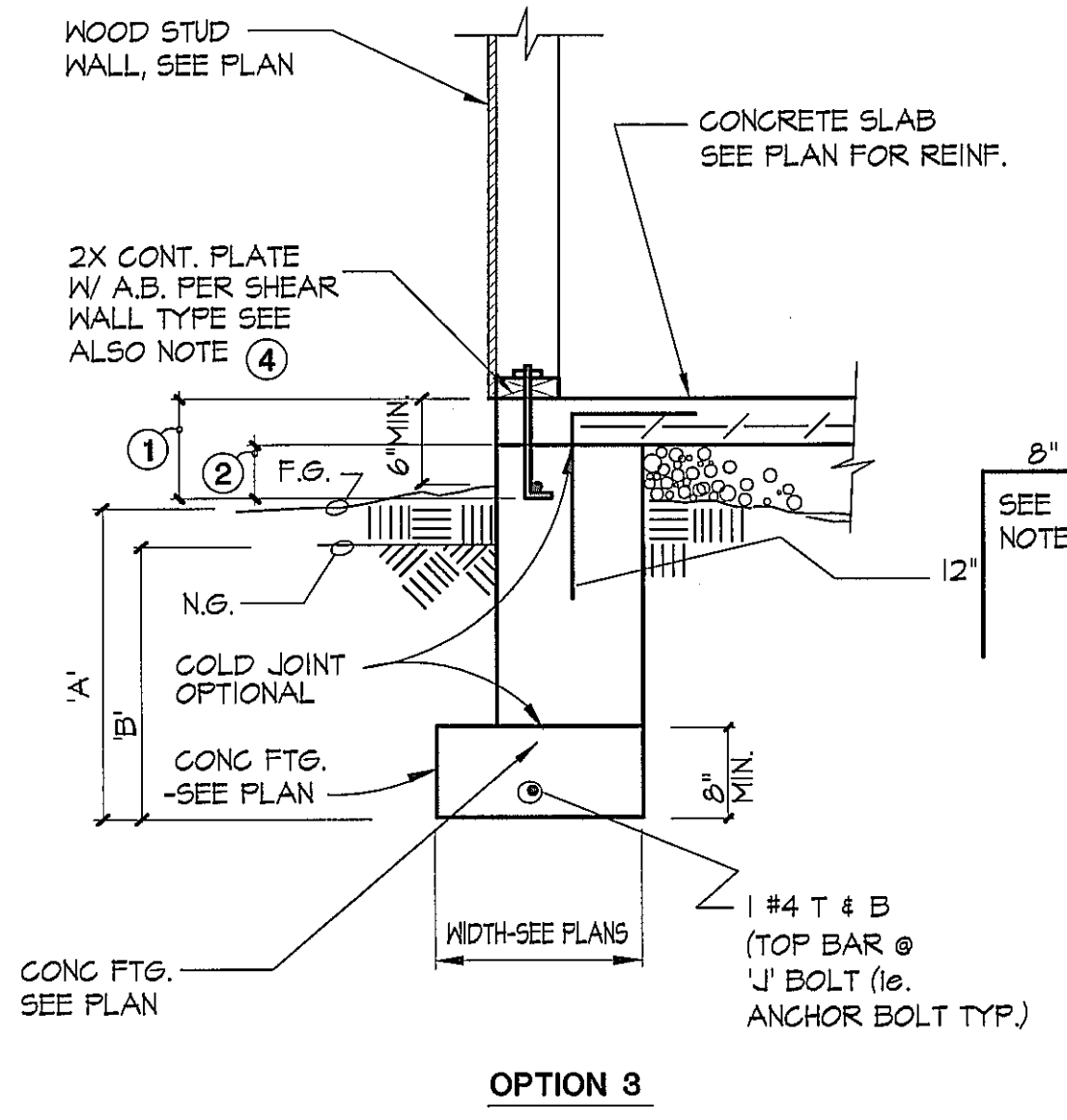
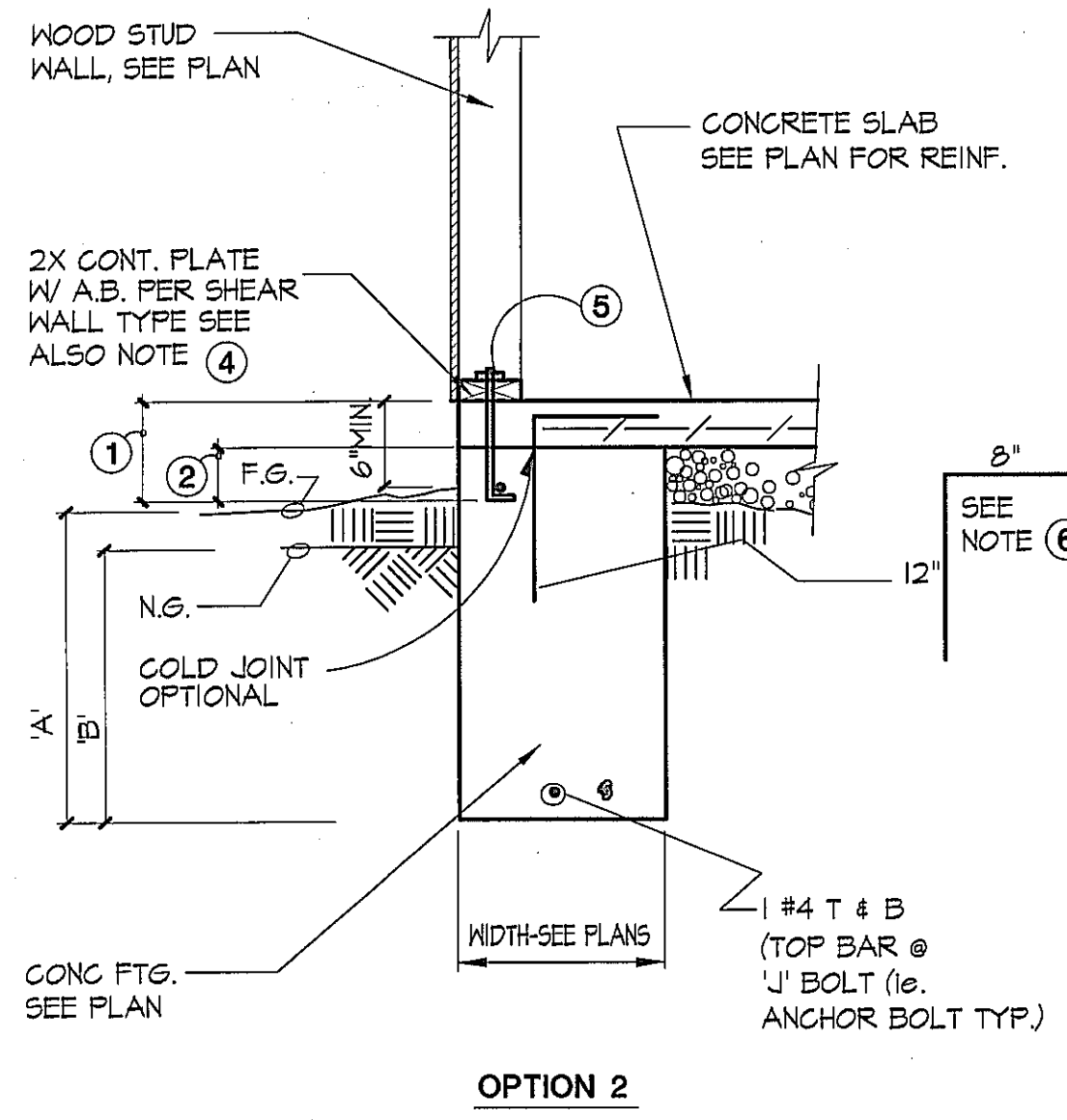
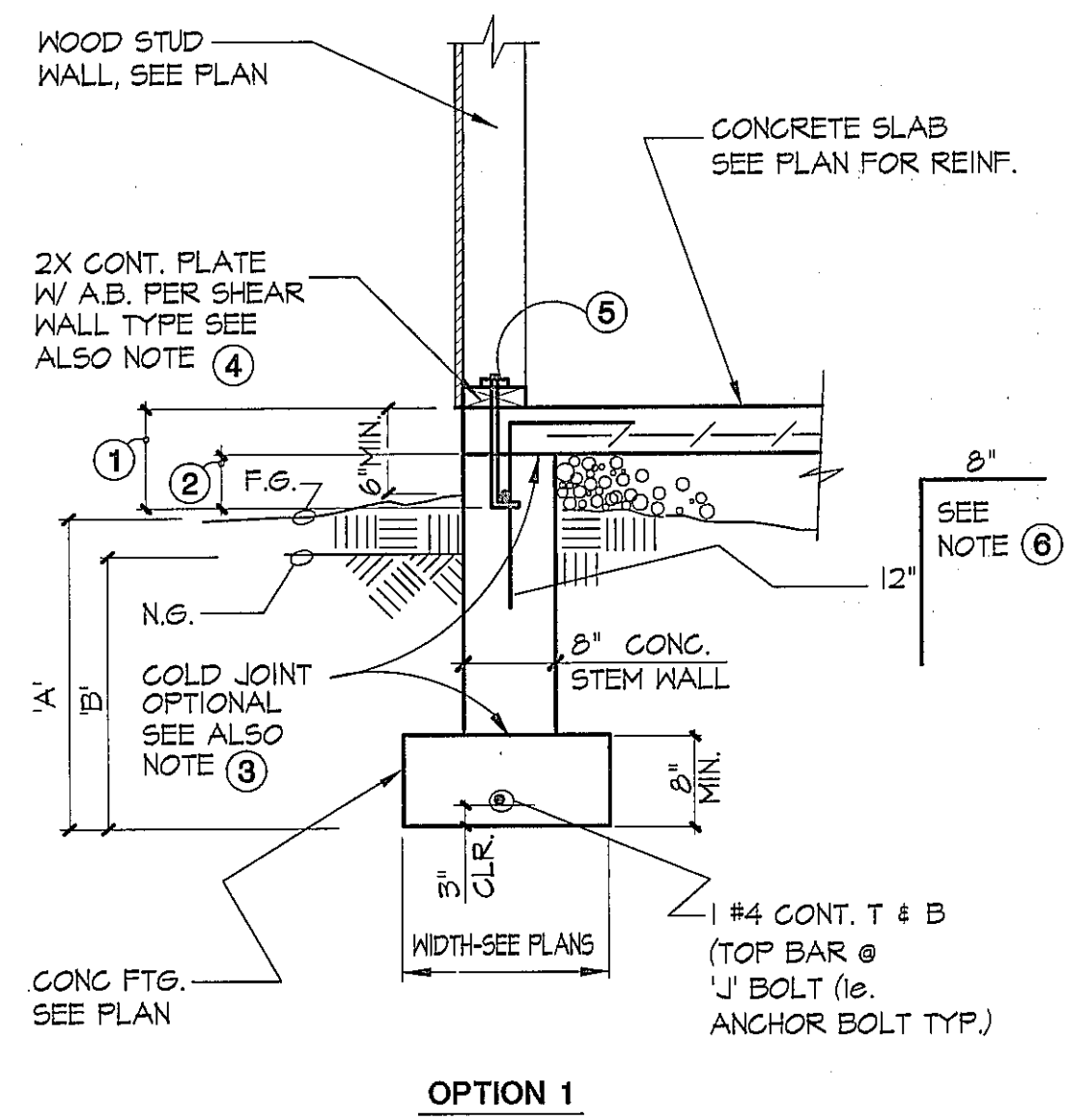
Engineer of Record

Date: 02/28/2005
Scale: Not to Scale
Sheet: **Sheet 3 of 3**

Date: 11-27-12
Scale:
Drawn: hrd
Job:
Sheet:
S-13C

TWO STORY SINGLE FAMILY RESIDENCE
901 LAUREL CYN.
LA, CA 90046 FOR:
LI. INVESTMENTS, LLC

REGISTERED PROFESSIONAL ENGINEER
No. SE2628
STATE OF CALIFORNIA



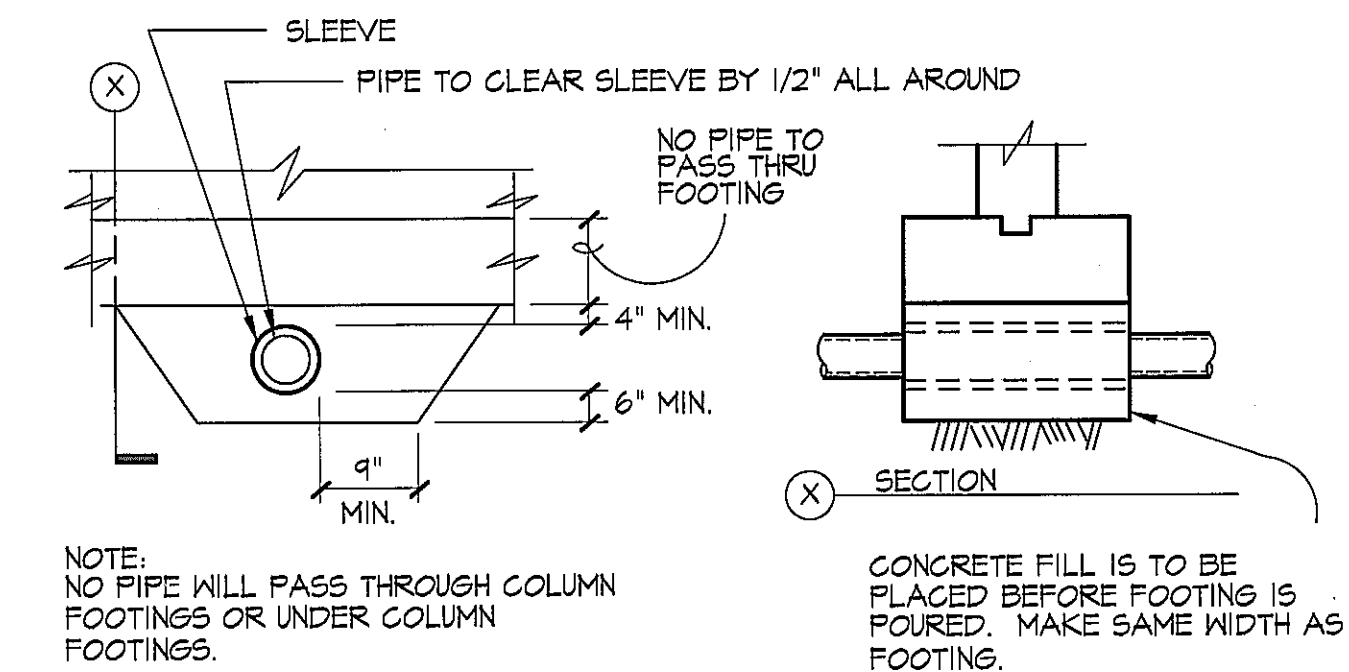
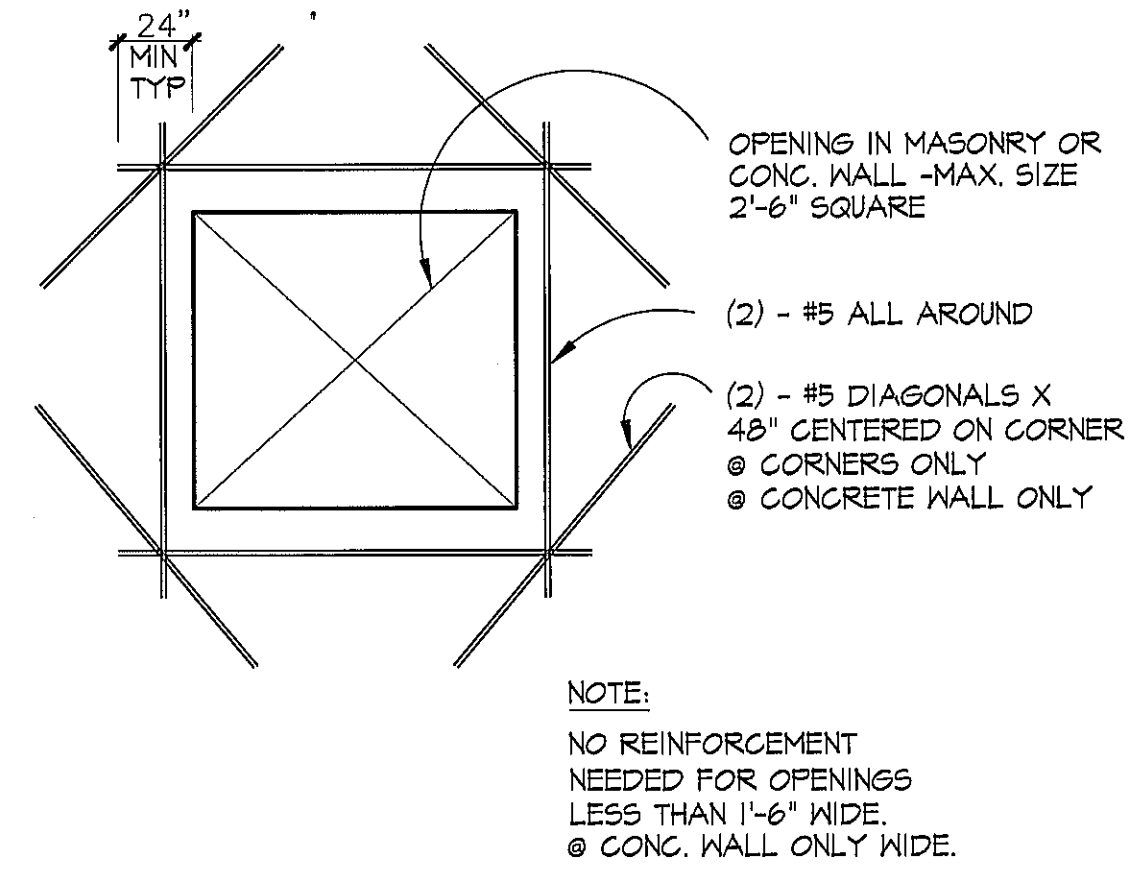
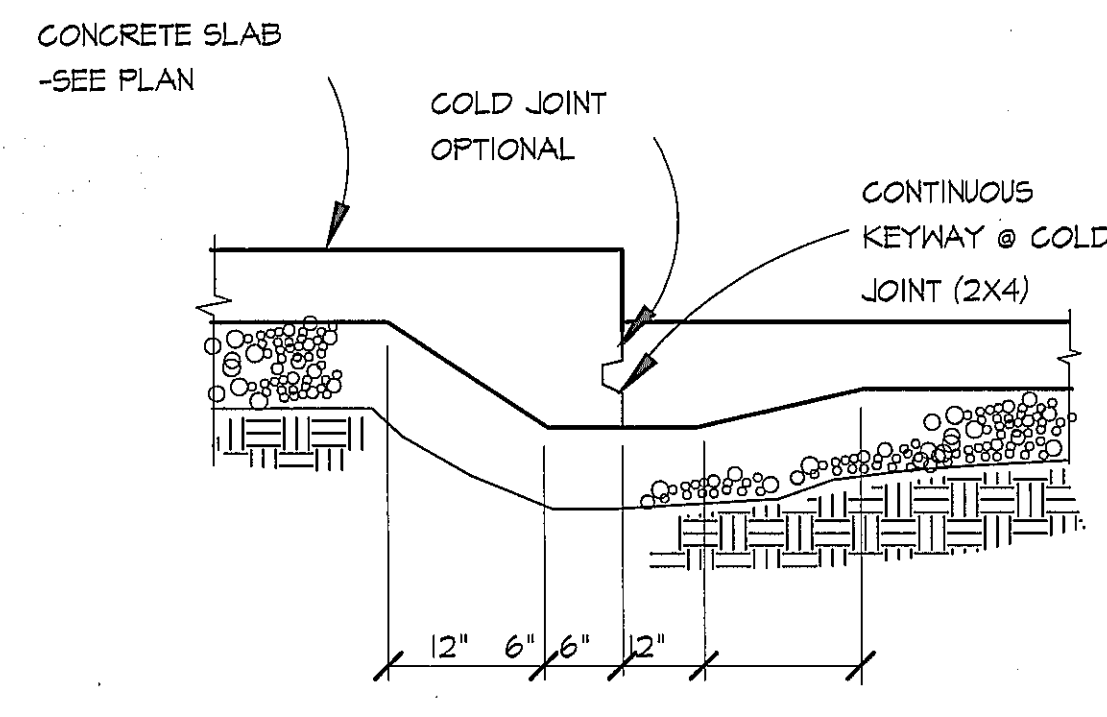
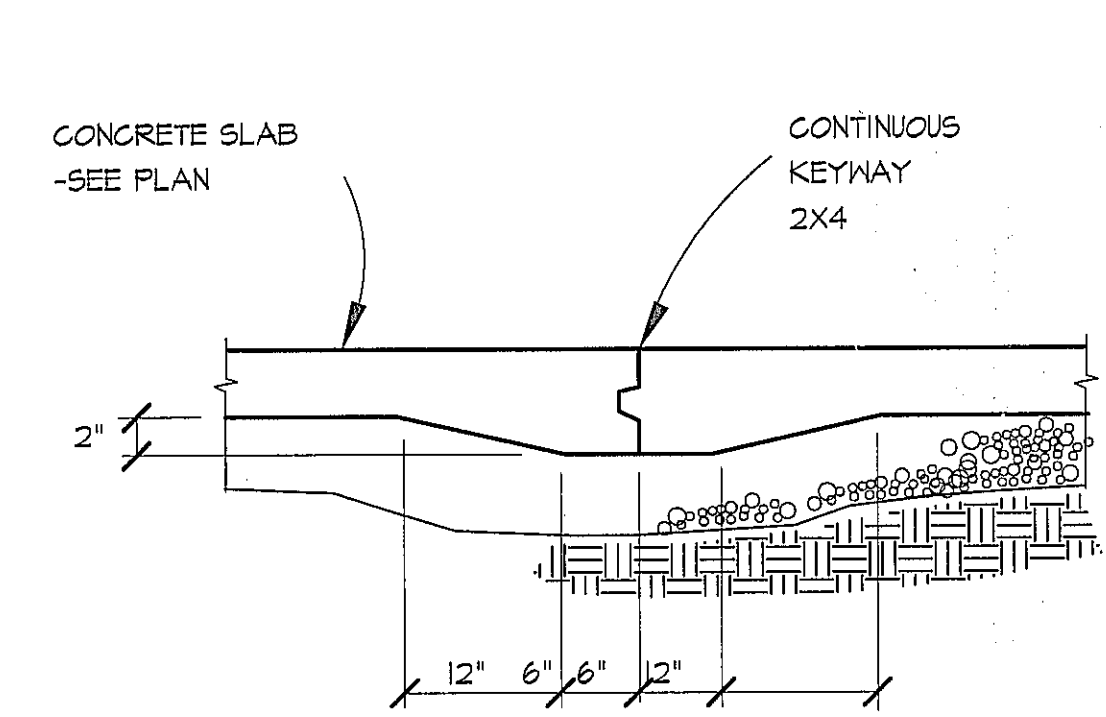
NOTES:
SEE ADDITIONAL INFORMATION IN STRUCTURAL SPECIFICATIONS, GENERAL NOTES AND SOILS REPORT (IF AVAILABLE). SITE PREPARATION PER SOILS REPORT, TAKES PRECEDENCE OVER ALL OTHER WORK (SEE SOILS ENGINEER).

DEPTH BELOW FINISH GRADE 'A'	DEPTH BELOW NATURAL GRADE 'B'
SEE STRUCTURAL SPECIFICATIONS	24" MIN.

- CONCRETE FOOTING NOTES**
- EMBED ANCHOR BOLT 9" MIN. BELOW THE TOP OF THE SLAB WHERE NO COLD JOINT IS USED.
 - EMBED ANCHOR BOLT 7" MIN. BELOW THE COLD JOINT LINE WHERE A COLD JOINT IS USED.
 - WHERE A COLD JOINT (DOUBLE POUR) IS USED PARTICULAR ATTENTION TO THE CLEANLINESS OF THE TOP OF THE FOOTING IS NEEDED, I.E. CLEAN WIRE BRUSH PREFERRED) AND REMOVE LOOSE SAND, DEBRIS, ETC. FROM THE TOP OF THE LOWER POUR TYP. WHERE A DOUBLE POUR SYSTEM IS USED.
 - USE 3X P.T. SILL TYP. AT "M", "N", "O" & "P" TYPE SHEAR WALLS.
 - A STEEL PLATE WASHER IS REQUIRED AT SHEAR WALL ANCHOR BOLTS FOR SIZE SEE DETAIL #12 ON SHEET S-5.1
 - WHERE A COLD JOINT (DOUBLE POUR) IS USED INSTALL #4 DOWELS AT 8'-0" O.C. WITH BAR SIZE / SHAPE AS SHOWN.

EXTERIOR FOUNDATION AT SLAB ON GRADE OPTION 1, 2 OR 3 (SEE PLANS & SPECIFICATIONS FOR REFERENCE)

1



CONSTRUCTION JOINT IN SLAB ON GRADE

11

DEPRESSION IN SLAB ON GRADE

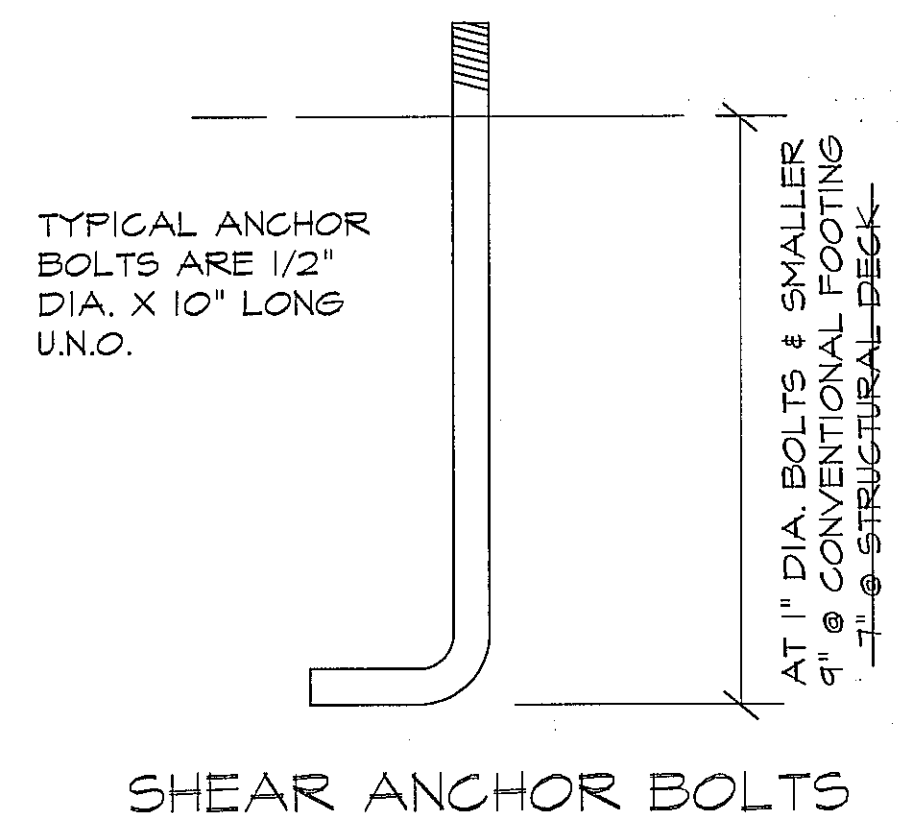
8

OPENING IN MASONRY OR CONC. WALL

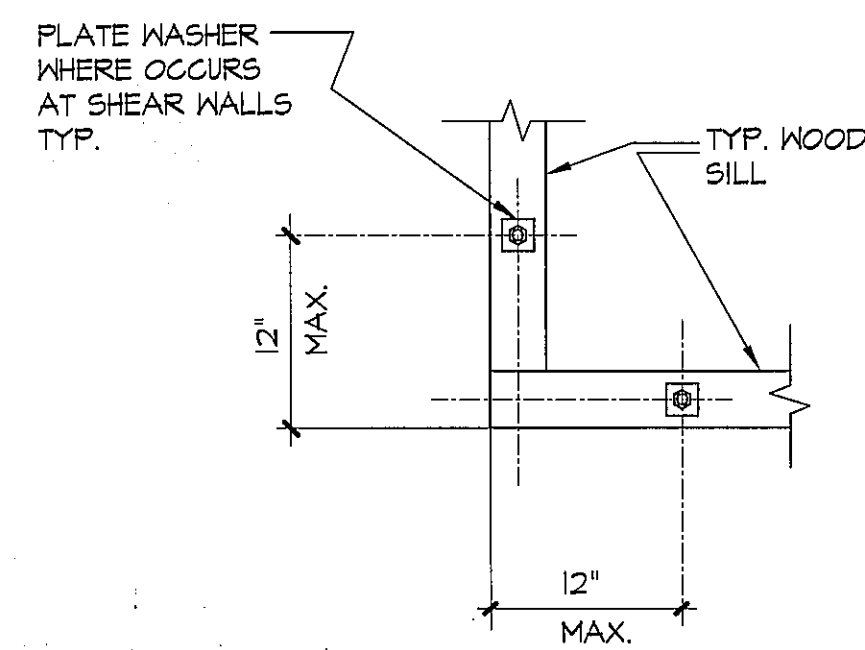
5

PIPE THROUGH FOUNDATIONS

2

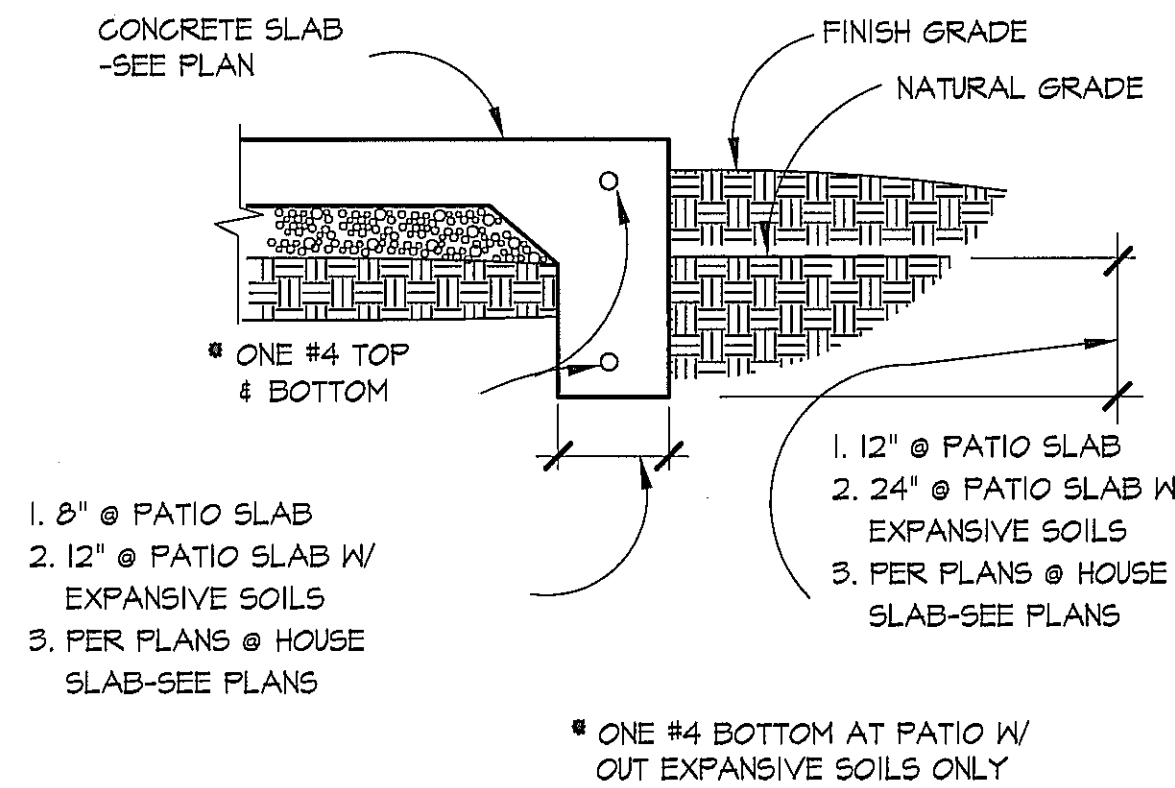


SHEAR ANCHOR BOLTS



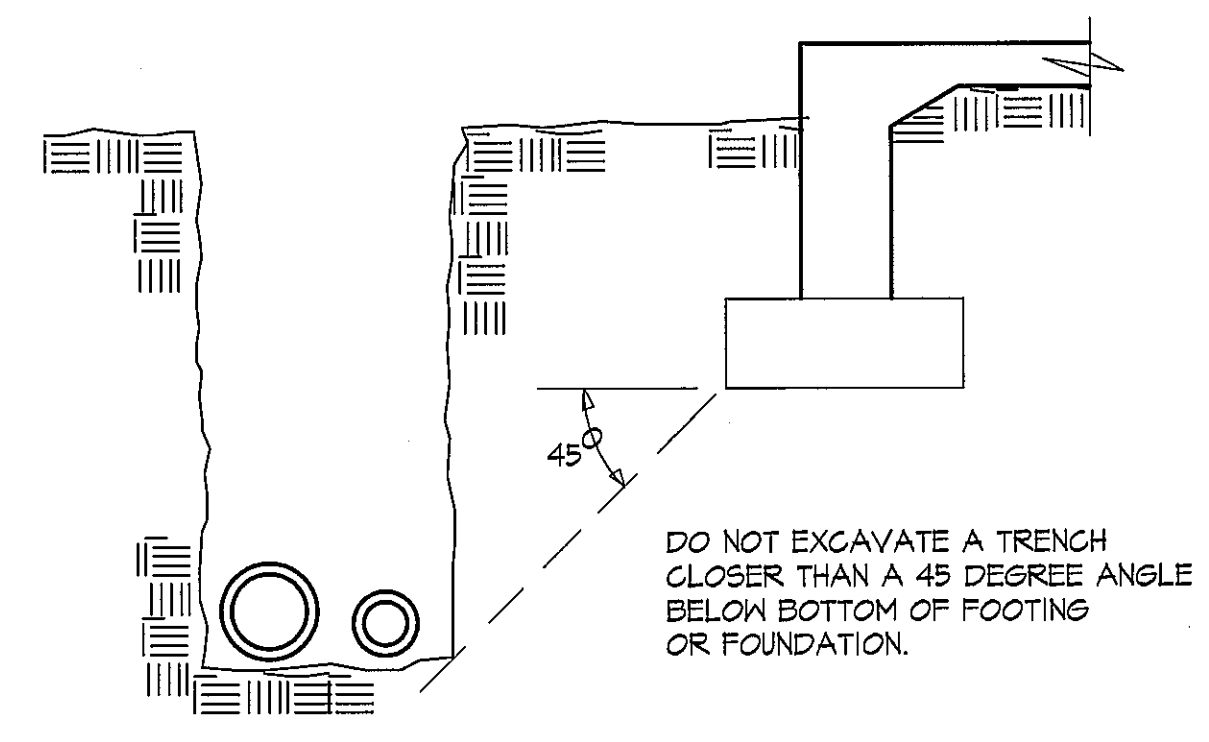
TYP. ANCHOR BOLT PLACEMENT

9



TURNDOWN @ EXTERIOR SLAB ON GRADE

6



TRENCH PARALLEL TO FOUNDATION

3

ANCHOR BOLT DETAIL FOR SHEAR ANCHORS

12

REVISIONS BY

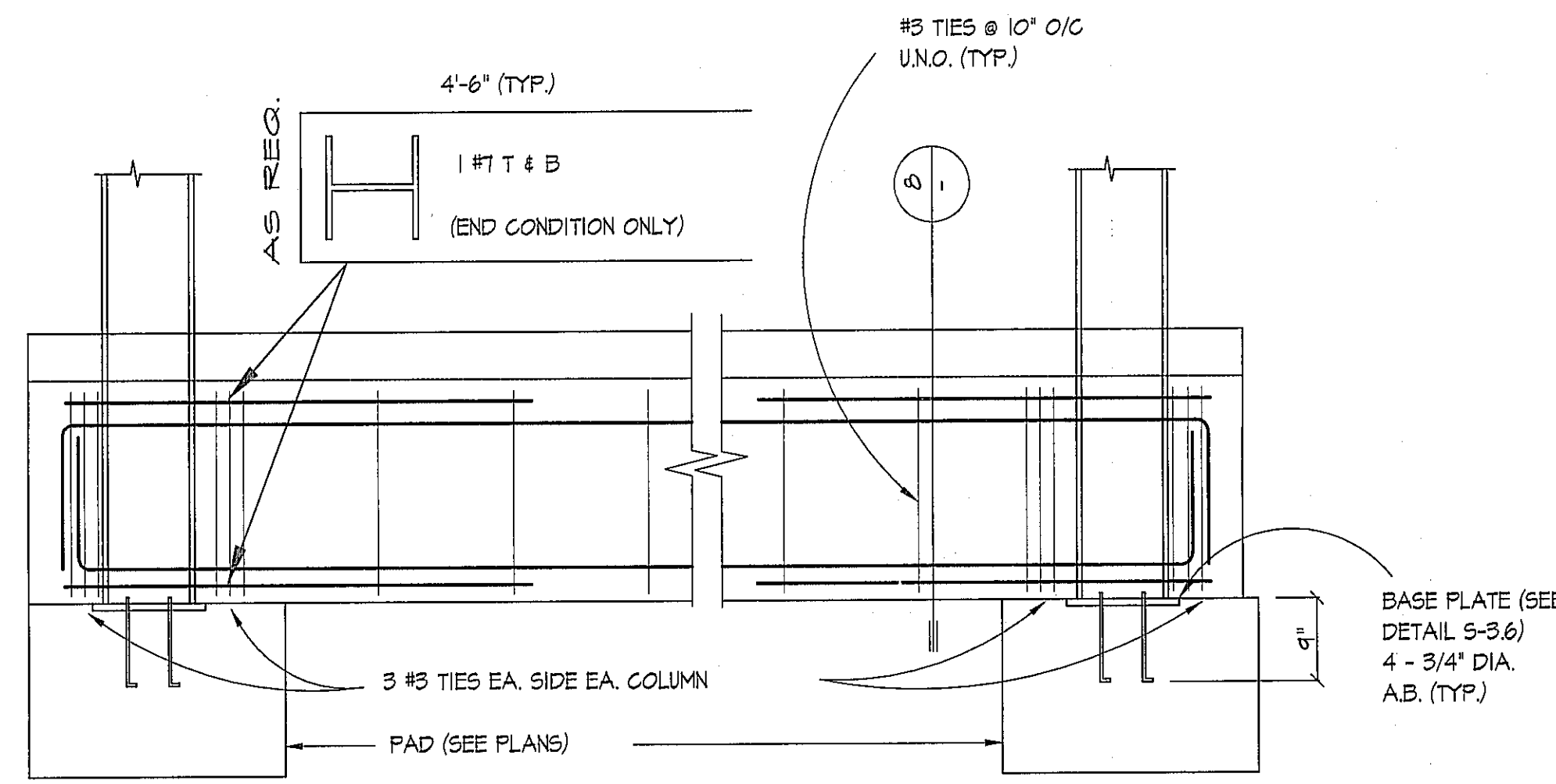
HRD ENGINEERING
STRUCTURAL DESIGN AND DRAFTING
7463 VARNA AVE., SECOND FLOOR
N. HOLLYWOOD, CA 91605
PHONE NO. (805) 431-5415 - FAX NO. (805) 888-4840

FOUNDATION DETAILS

TWO STORY SINGLE FAMILY RESIDENCE
901 LAUREL CYN.
LA, CA 90046 FOR: L.I. INVESTMENTS, LLC

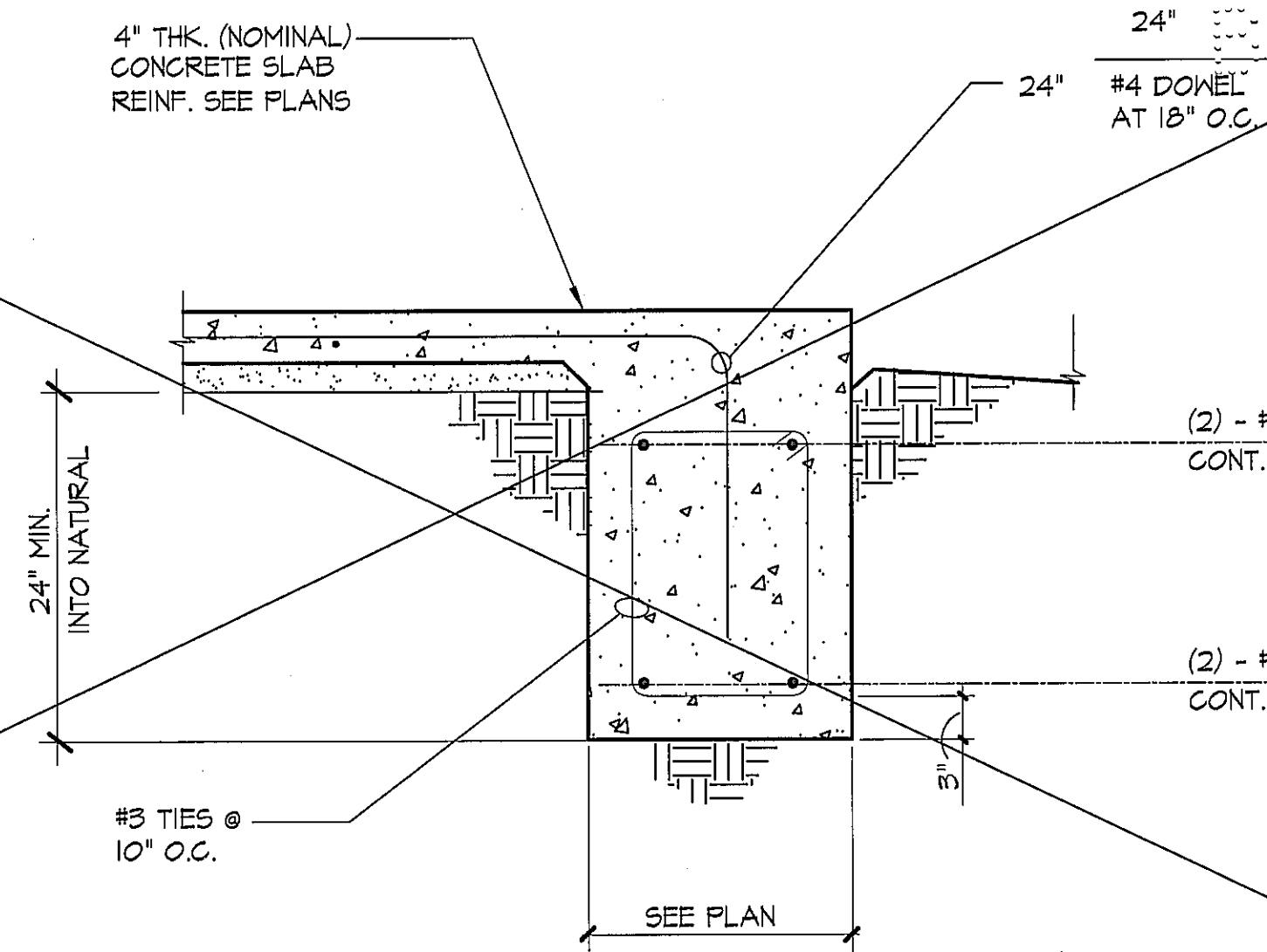


Date 12-18-12
Scale
Drawn hrd
Job
Sheet S-2.1
Of



SEQUENCE OF CONSTRUCTION

1. EXCAVATE AND POUR PADS. NOTE: POUR 1 1/2" LOW FOR LEVELING.
2. PLACE COLUMN LEVEL AND DRYPACK UNDER.
3. PLACE GRADE BEAM REINFORCEMENT. HAVE GRADE BEAM DEPUTY INSPECTED (3000 PSI CONCRETE).
4. POUR GRADE BEAM. NOTE: BY CODE, THERE CAN NOT BE TWO GRADES OF CONCRETE AT SITE AT SAME TIME. STAGGER FOUR SUCH THAT GRADE BEAMS ARE POURED SEPARATELY FROM ALL OTHER FOOTING (2000 PSI).
5. POUR SLAB OVER GRADE BEAM.

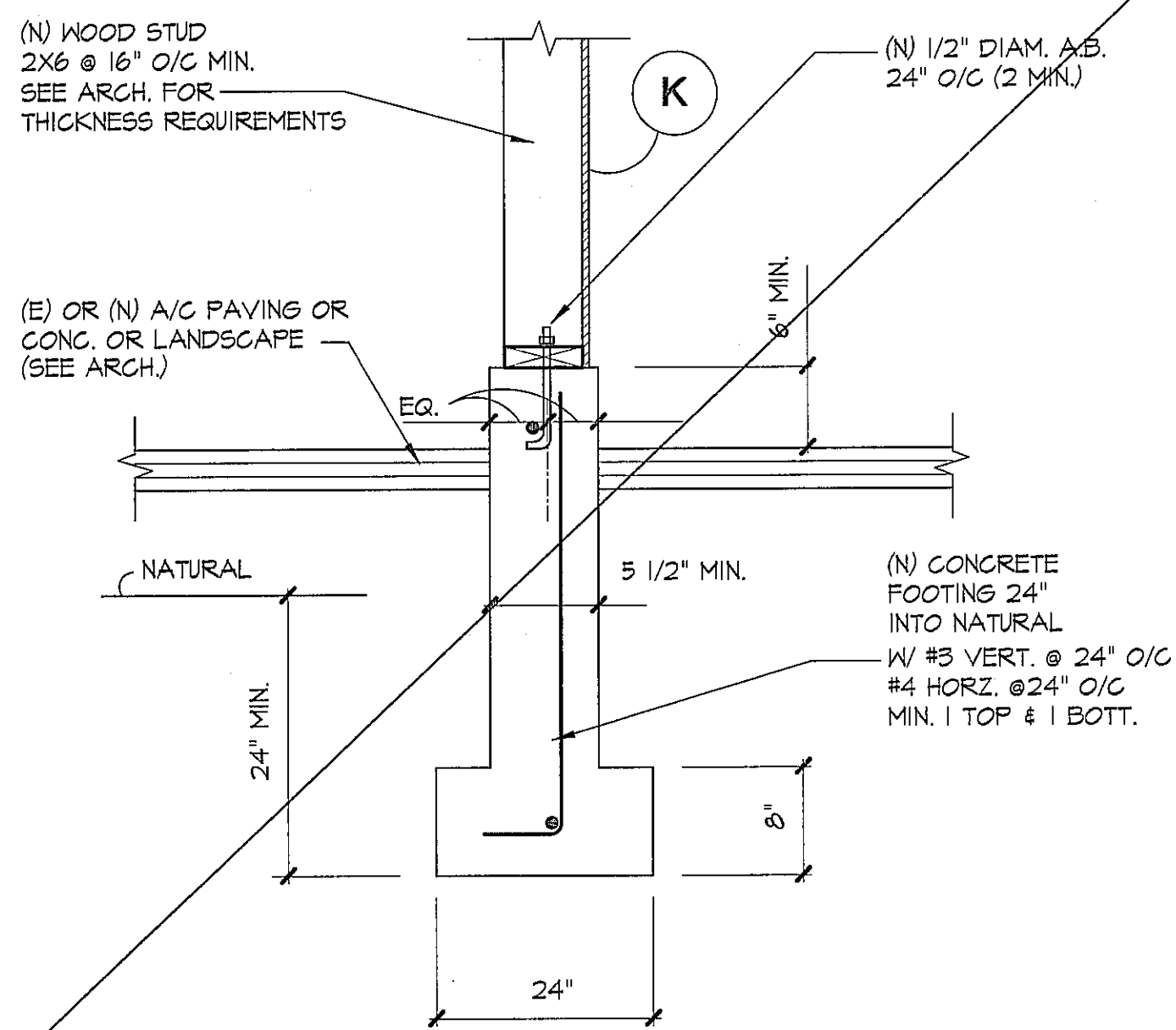


LATERAL (WIND/SEISMIC) SUPPORT COLUMN / GRADE BEAM CONNECTION DETAIL TYPICAL

7

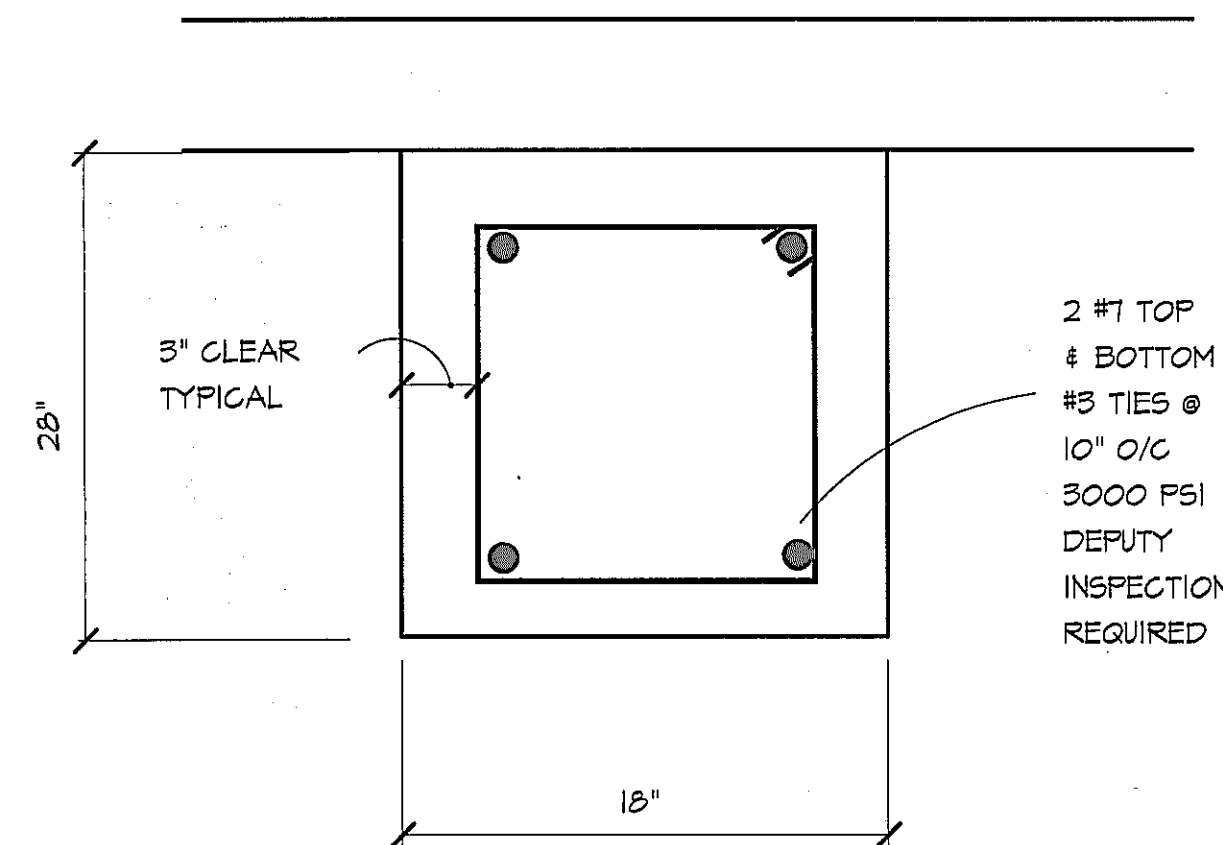
GRADE BEAM AT GARAGE ENTRANCE

1



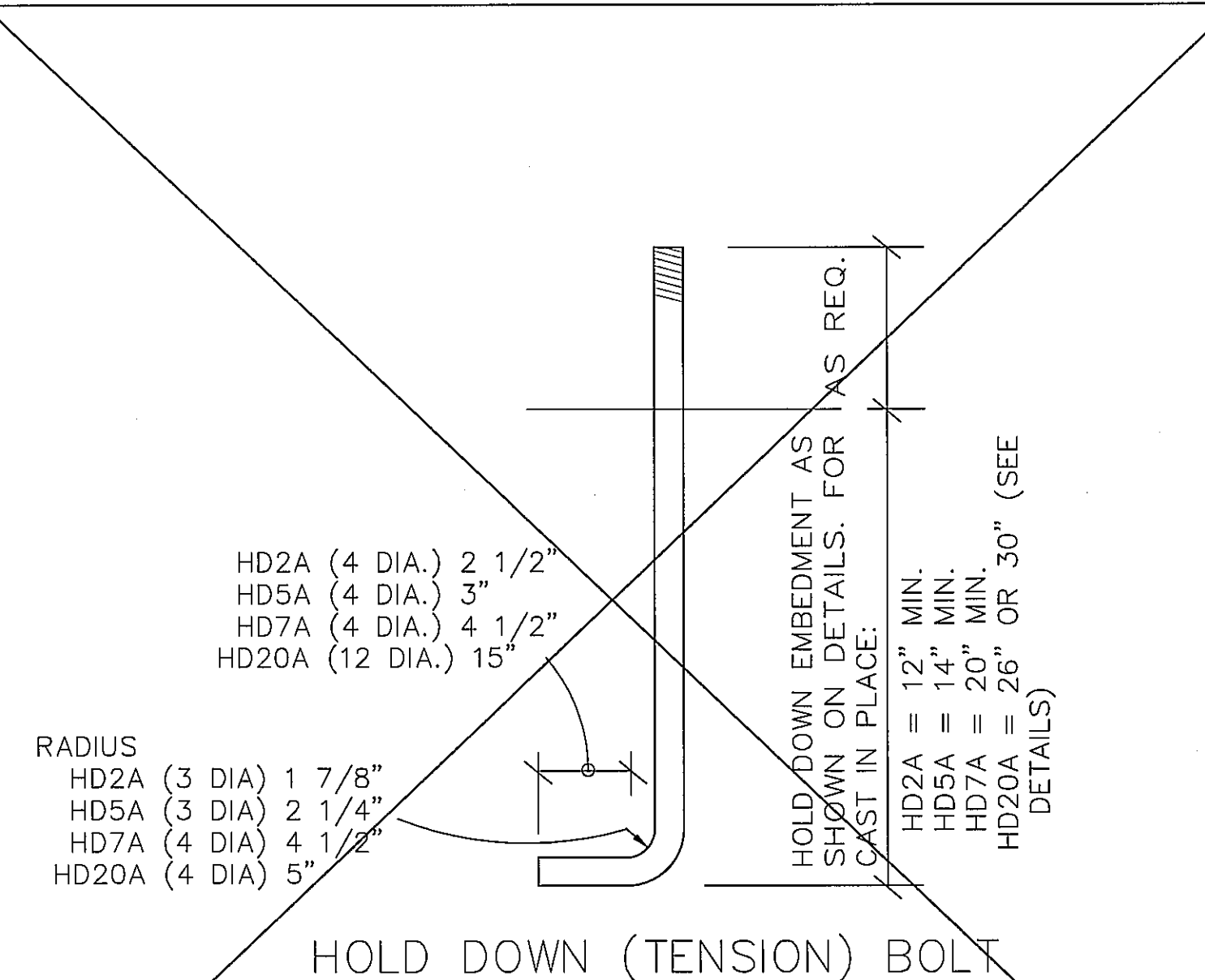
FOOTING AT GARAGE WALL

11



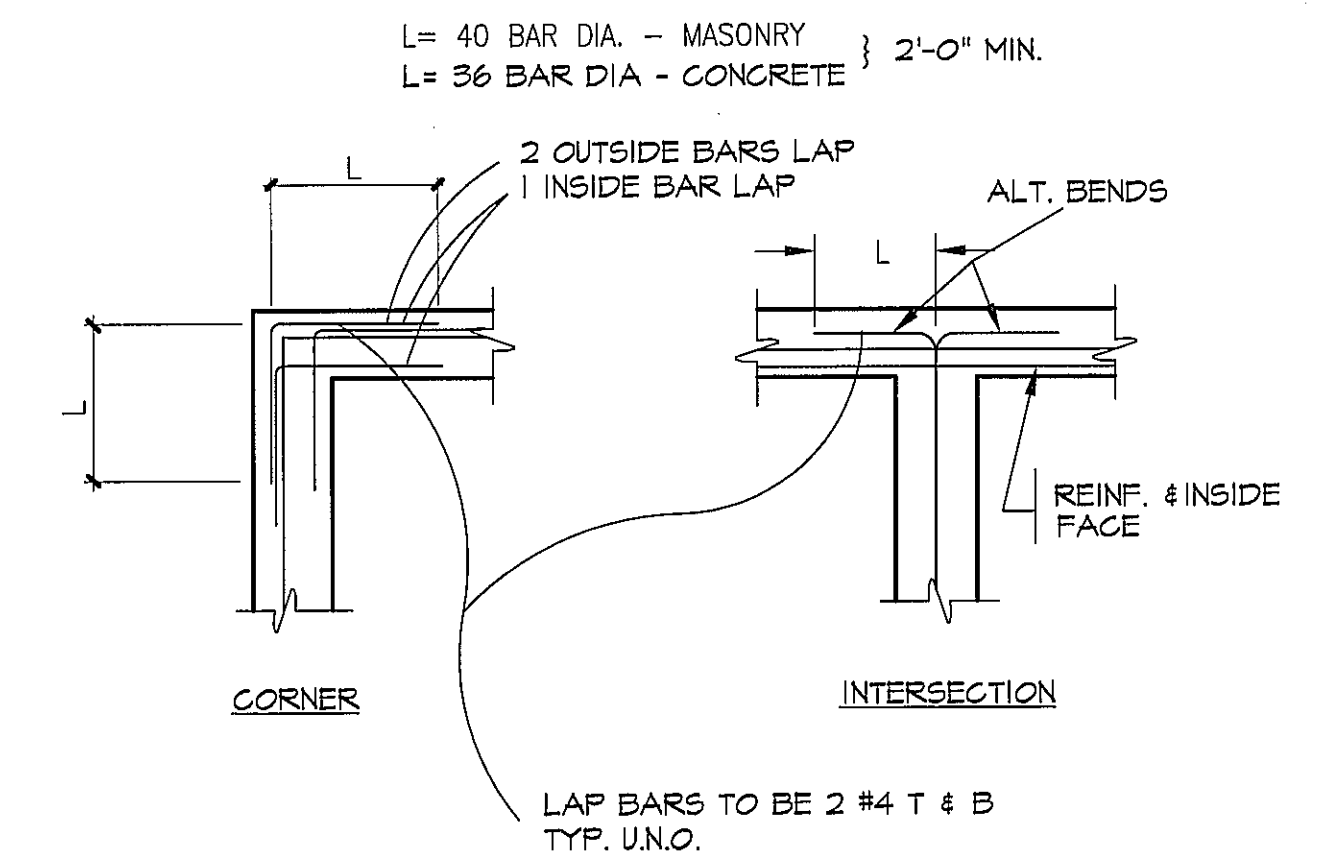
BELOW GRADE TIE FOOTING

8



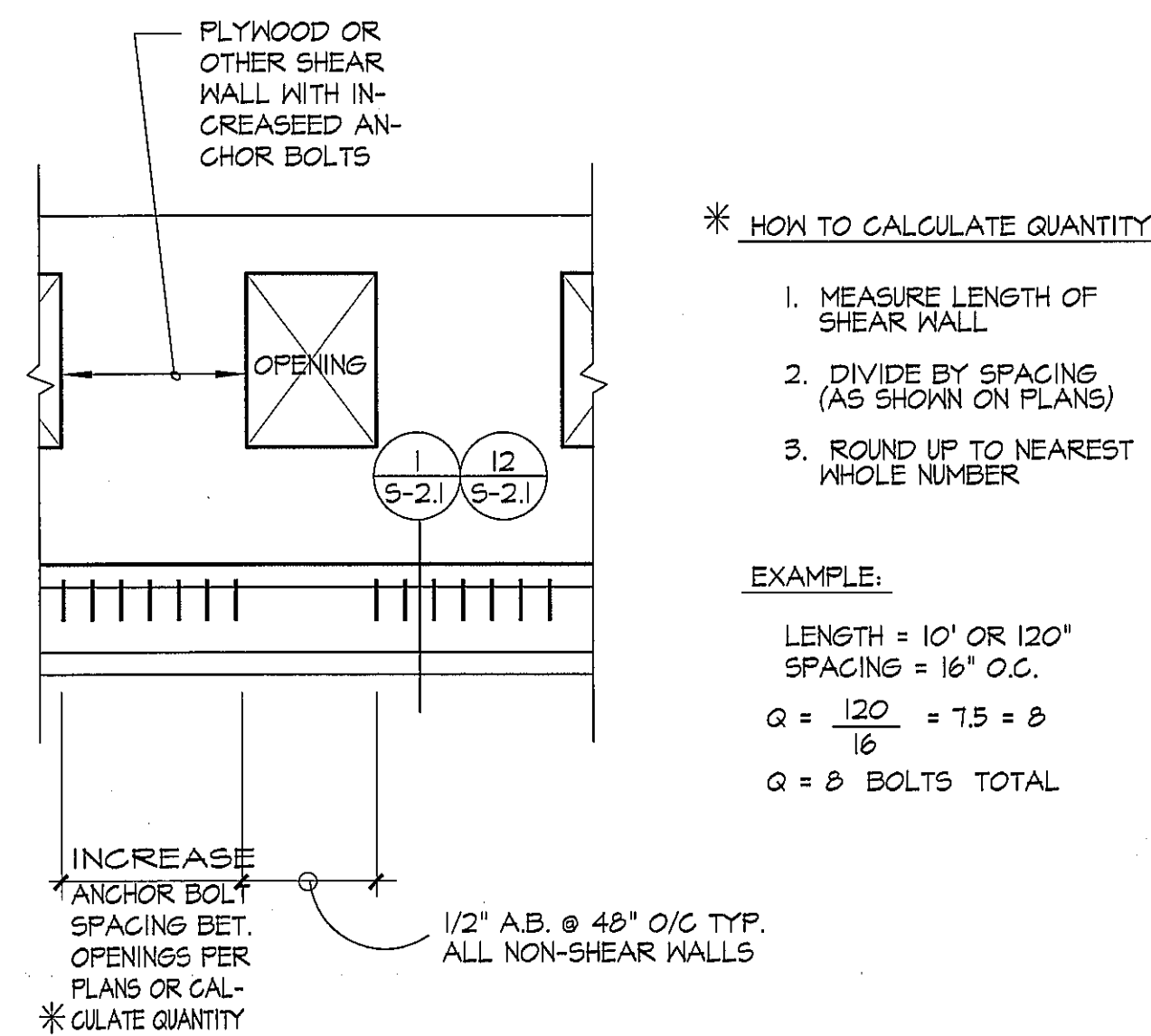
ANCHOR BOLT AT HOLD DOWN

5



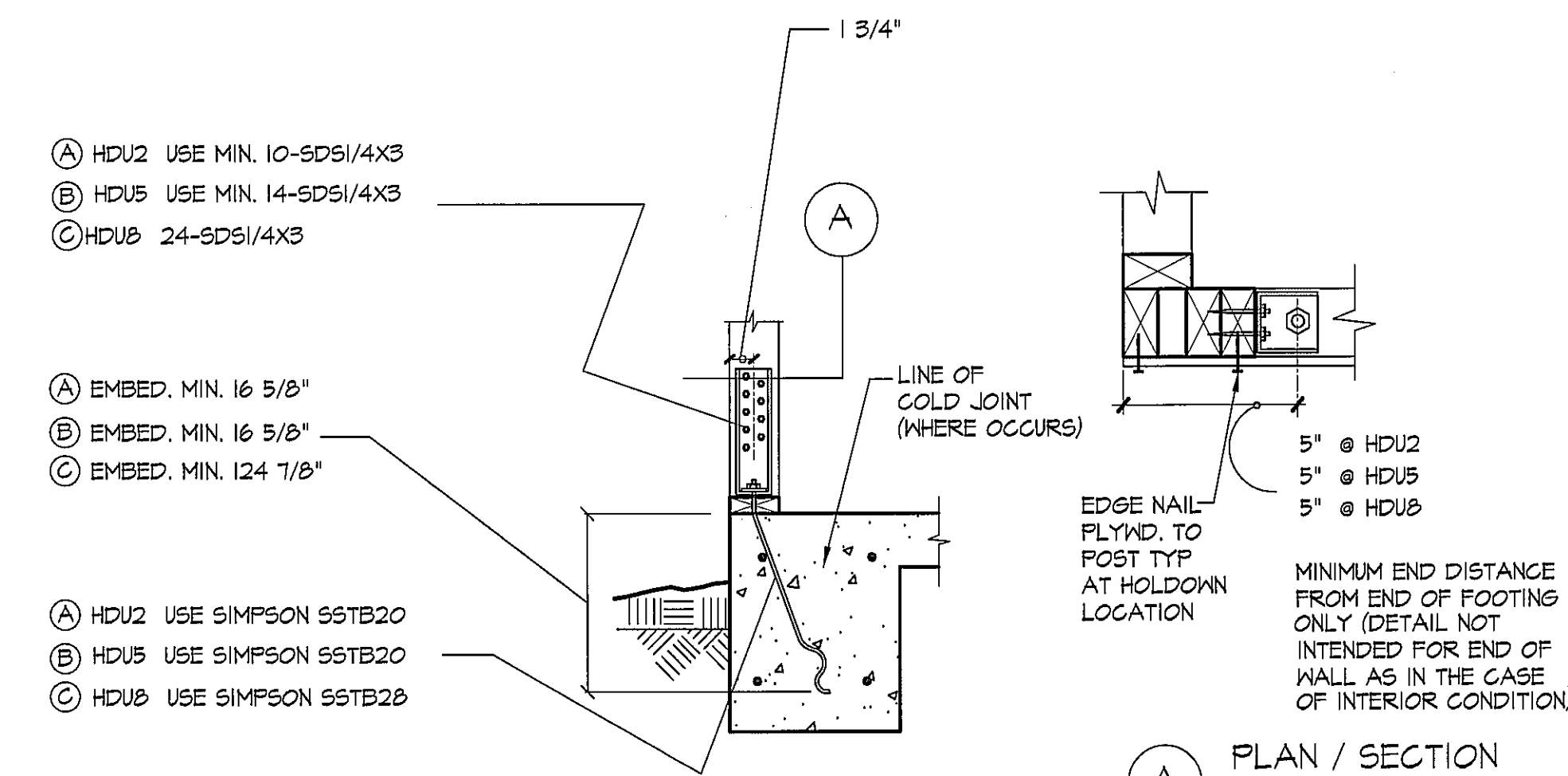
FOOTING CORNER/INTERSECTION REINF. LAP

2



ADDED ANCHOR BOLTS AT SHEAR WALLS

12



HOLDDOWN AT SLAB ON GRADE

3

FOR MORE INFORMATION SEE:
1. THE HOLDDOWN SCHEDULE ON S-5.1
2. THE SIMPSON SPECIFICATIONS ON SHEET S-1.1

REVISIONS	BY

THESE PLANS DRAWINGS, DESIGNS AND SPECIFICATIONS ARE THE SOLE PROPERTY OF HRD ENGINEERING FOR THE EXCLUSIVE USE OF ITS CLIENTS FOR THE PROJECT AND SITE SPECIFICALLY IDENTIFIED HEREON. NO PARTS OF THESE PLANS DRAWINGS, DESIGNS OR SPECIFICATIONS ARE TO BE REPRODUCED, COPIED, TRANSFERRED OR USED IN ANY MANNER WITHOUT WRITTEN APPROVAL. COPYRIGHT © HRD ENGINEERING.

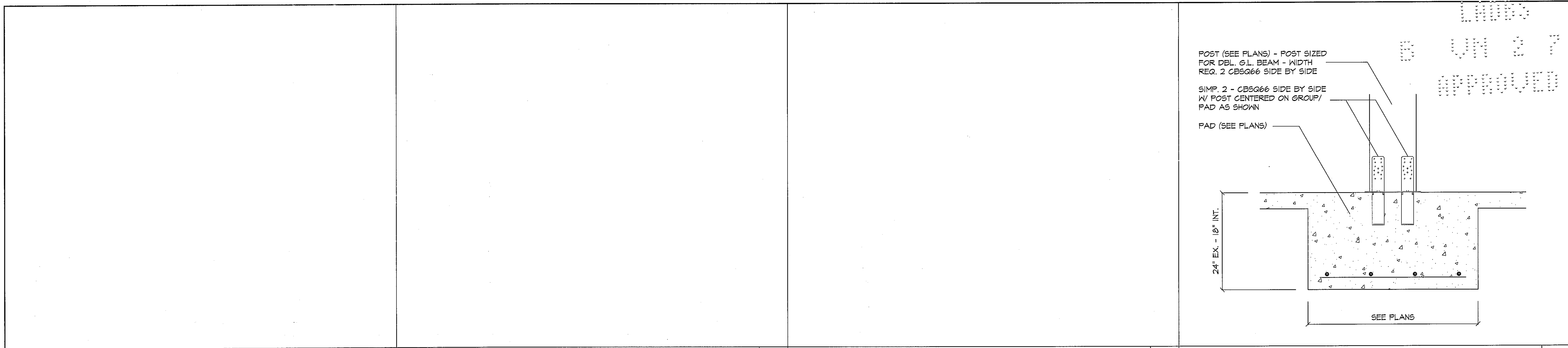
HRD ENGINEERING
STRUCTURAL DESIGN AND DRAFTING
7463 VARNA AVE., SECOND FLOOR
N. HOLLYWOOD, CA 91605
PHONE NO. (805) 431-5415 - FAX NO. (805) 888-4840

FOUNDATION DETAILS

TWO STORY SINGLE FAMILY RESIDENCE
901 LAUREL CYN.
LA, CA 90046 FOR: L.I. INVESTMENTS, LLC

PROFESSIONAL ENGINEER
No. SE2628
3-31-14
STATE OF CALIFORNIA

Date 12-18-12
Scale
Drawn hrd
Job
Sheet
S-2.2
of



POST (SEE PLANS) - POST SIZED FOR DBL. G.L. BEAM - WIDTH REQ. 2 C8S966 SIDE BY SIDE

SIMP. 2 - C8S966 SIDE BY SIDE W/ POST CENTERED ON GROUP/ PAD AS SHOWN

PAD (SEE PLANS)

24" EX. - 18" INT.

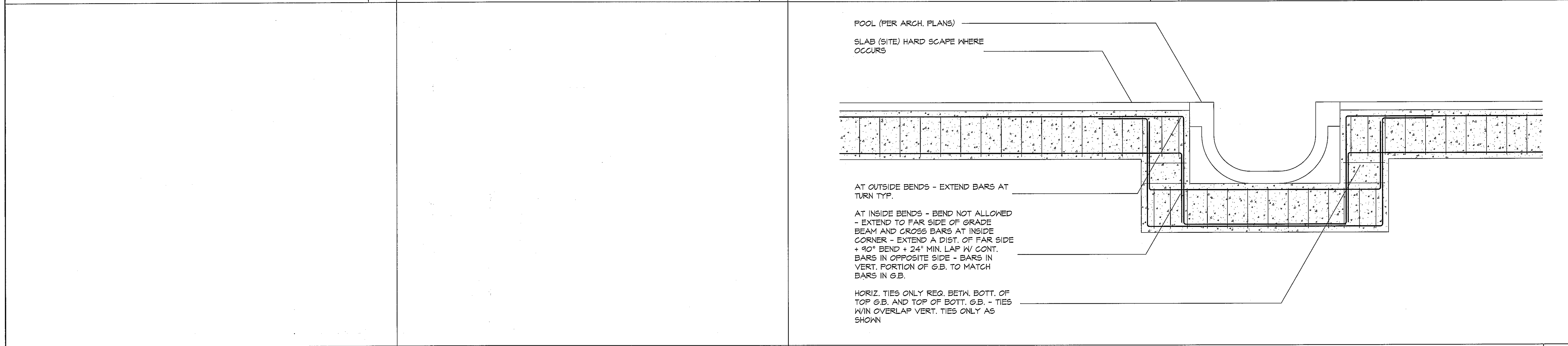
SEE PLANS

REVISIONS	BY

THESE DRAWINGS, DESIGNS AND SPECIFICATIONS ARE THE SOLE PROPERTY OF HRD ENGINEERING FOR THE EXCLUSIVE USE OF ITS CLIENTS FOR THE PROJECT AND SITE SPECIFICALLY IDENTIFIED HEREIN. ANY REUSE, REPRODUCTION, TRANSFER OF USE OR NOT PERMITTED WITHOUT WRITTEN APPROVAL. COPYRIGHT © HRD ENGINEERING.

HRD ENGINEERING
 STRUCTURAL DESIGN AND DRAFTING
 7463 VARNA AVE., SECOND FLOOR
 N. HOLLYWOOD, CA 91605
 PHONE NO. (805) 431-5415 - FAX NO. (805) 888-4840

10	7	4	DET. DBL. CB	1
----	---	---	--------------	---



POOL (PER ARCH. PLANS)

SLAB (SITE) HARD SCAPE WHERE OCCURS

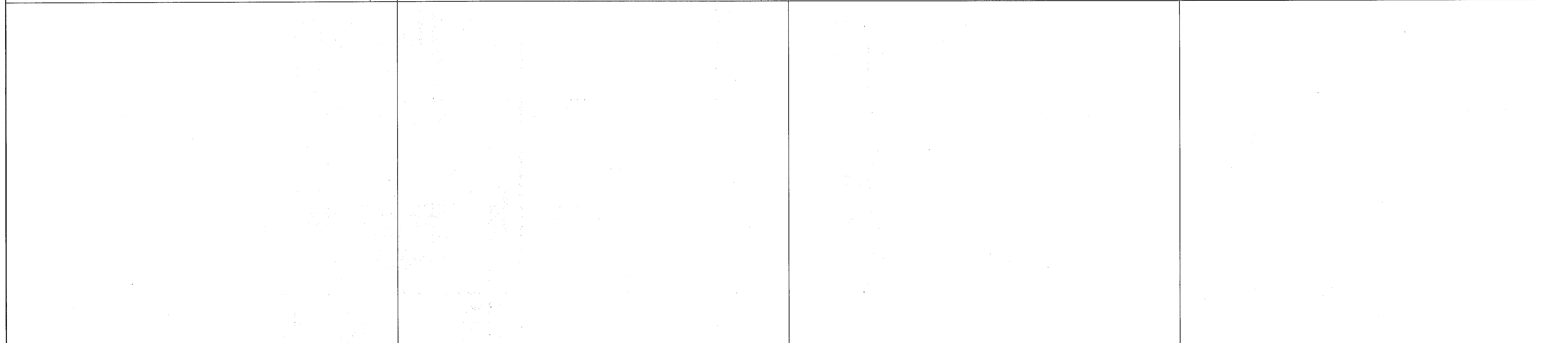
AT OUTSIDE BENDS - EXTEND BARS AT TURN TYP.

AT INSIDE BENDS - BEND NOT ALLOWED - EXTEND TO FAR SIDE OF GRADE BEAM AND CROSS BARS AT INSIDE CORNER - EXTEND A DIST. OF FAR SIDE + 90° BEND + 24" MIN. LAP W/ CONT. BARS IN OPPOSITE SIDE - BARS IN VERT. PORTION OF G.B. TO MATCH BARS IN G.B.

HORIZ. TIES ONLY REQ. BETW. BOTT. OF TOP G.B. AND TOP OF BOTT. G.B. - TIES W/IN OVERLAP VERT. TIES ONLY AS SHOWN

GRADE BEAM JOG AT POOL DET. 2

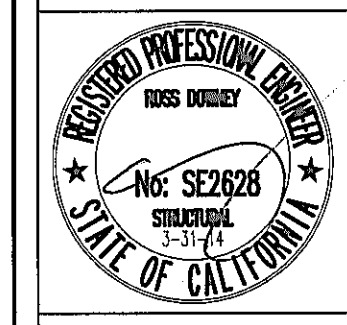
11	8		GRADE BEAM JOG AT POOL DET.	2
----	---	--	-----------------------------	---



12	9	6		3
----	---	---	--	---

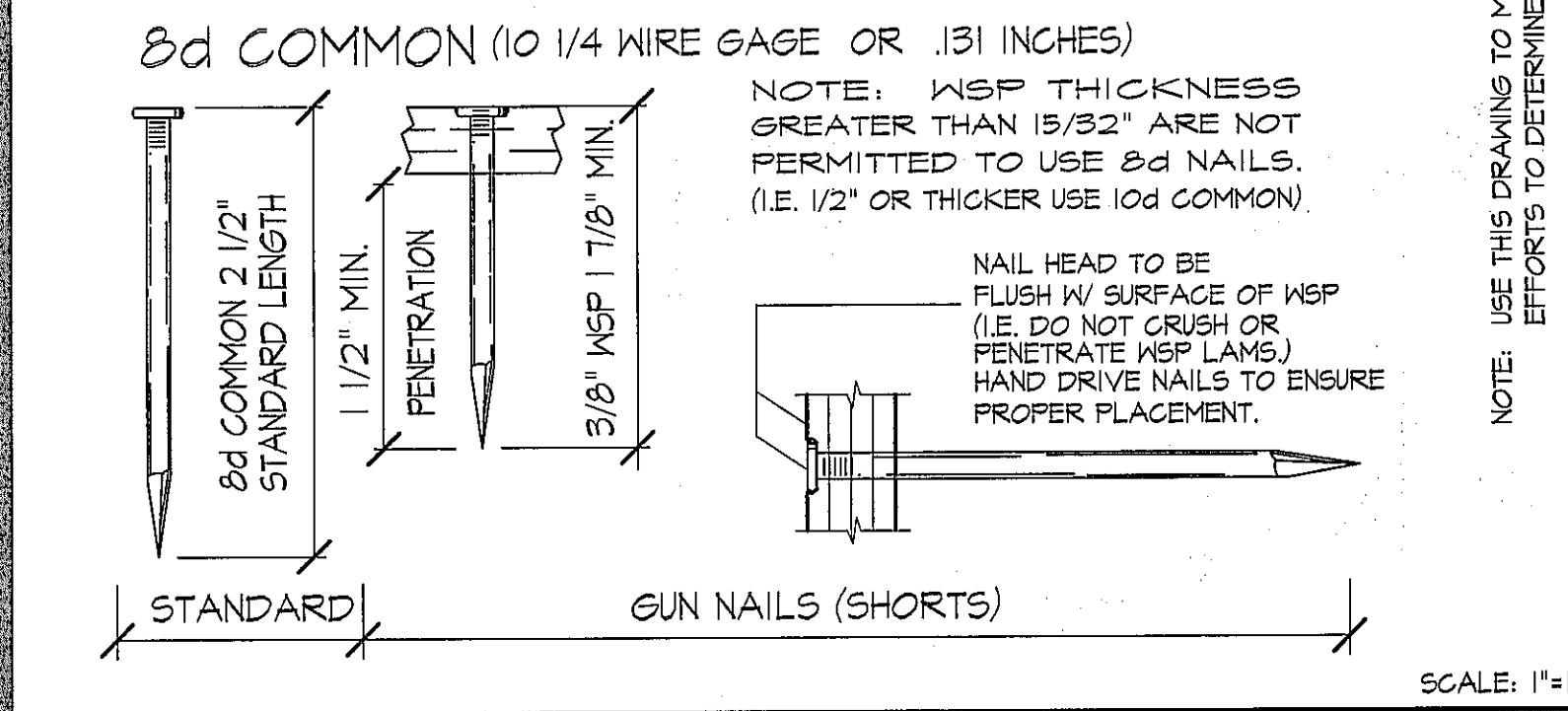
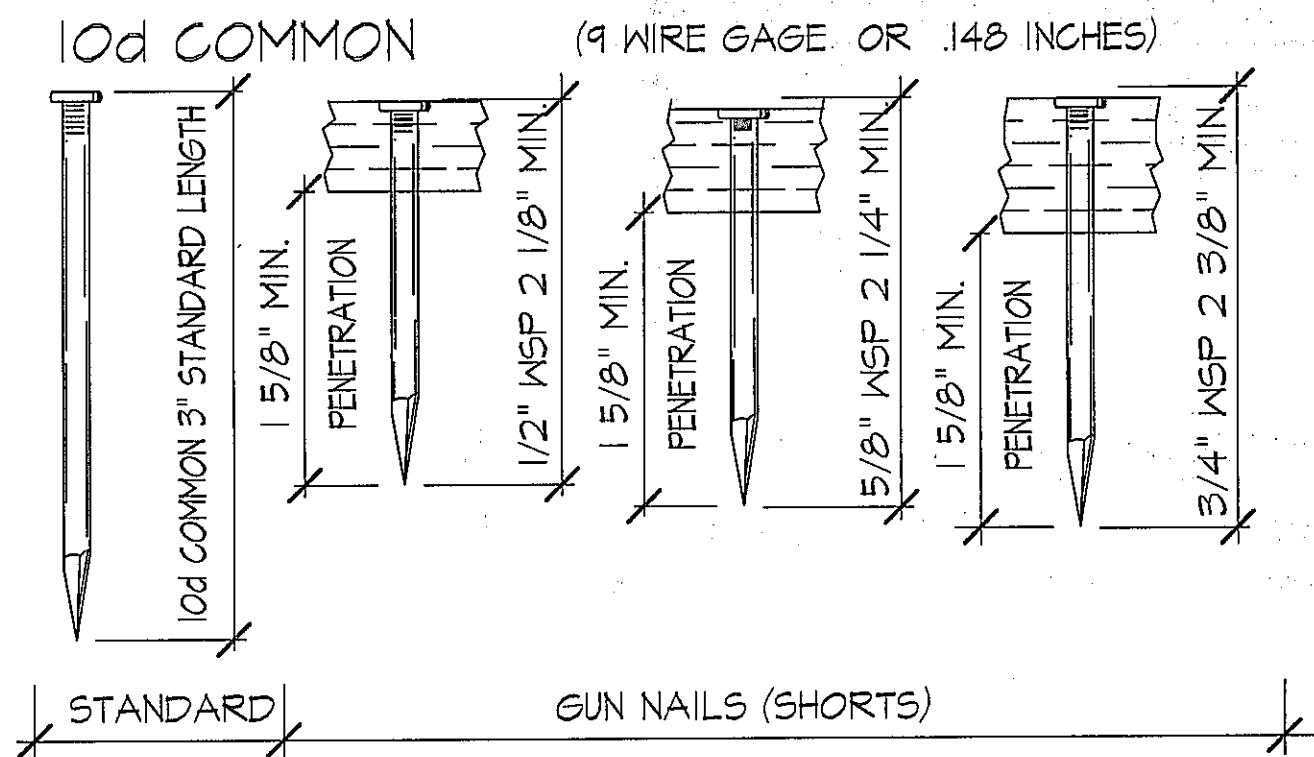
FOUNDATION DETAILS

TWO STORY SINGLE FAMILY RESIDENCE
 901 LAUREL CYN.
 LA, CA 90046 FOR L.I. INVESTMENTS, LLC



Date	12-18-12
Scale	
Drawn	hrd
Job	
Sheet	6-2.3
of	

NOTE: COMMON NAILS HAVE ALWAYS BEEN REQUIRED FOR WSP SHEAR WALLS AND THEIR USE IS NOW BEING ENFORCED. THE FOLLOWING NAILS ARE DRAWN AT FULL SCALE WITH SPECIFICATIONS TO ASSIST THE CONTRACTOR IN DETERMINING WHETHER OR NOT THE NAILS BEING USED ARE THE CORRECT SIZE.



A REQUIRED SHEAR WALL NAIL LENGTHS AND PENETRATION INTO FRAMING			
MIN. 25"	MAX. 31"	MIN. 28"	MAX. 34"
8d COMMON NAIL HEAD DIAMETER LIMITS		10d COMMON NAIL HEAD DIAMETER LIMITS	
28" CODE REQ.		32" CODE REQ.	

B APPROVED NAIL HEAD DIAMETER(S) / SHAPE(S)	C NOT APPROVED NAIL HEAD DIAMETER / SHAPE

SHEAR WALL NAIL(S) INFORMATION 10

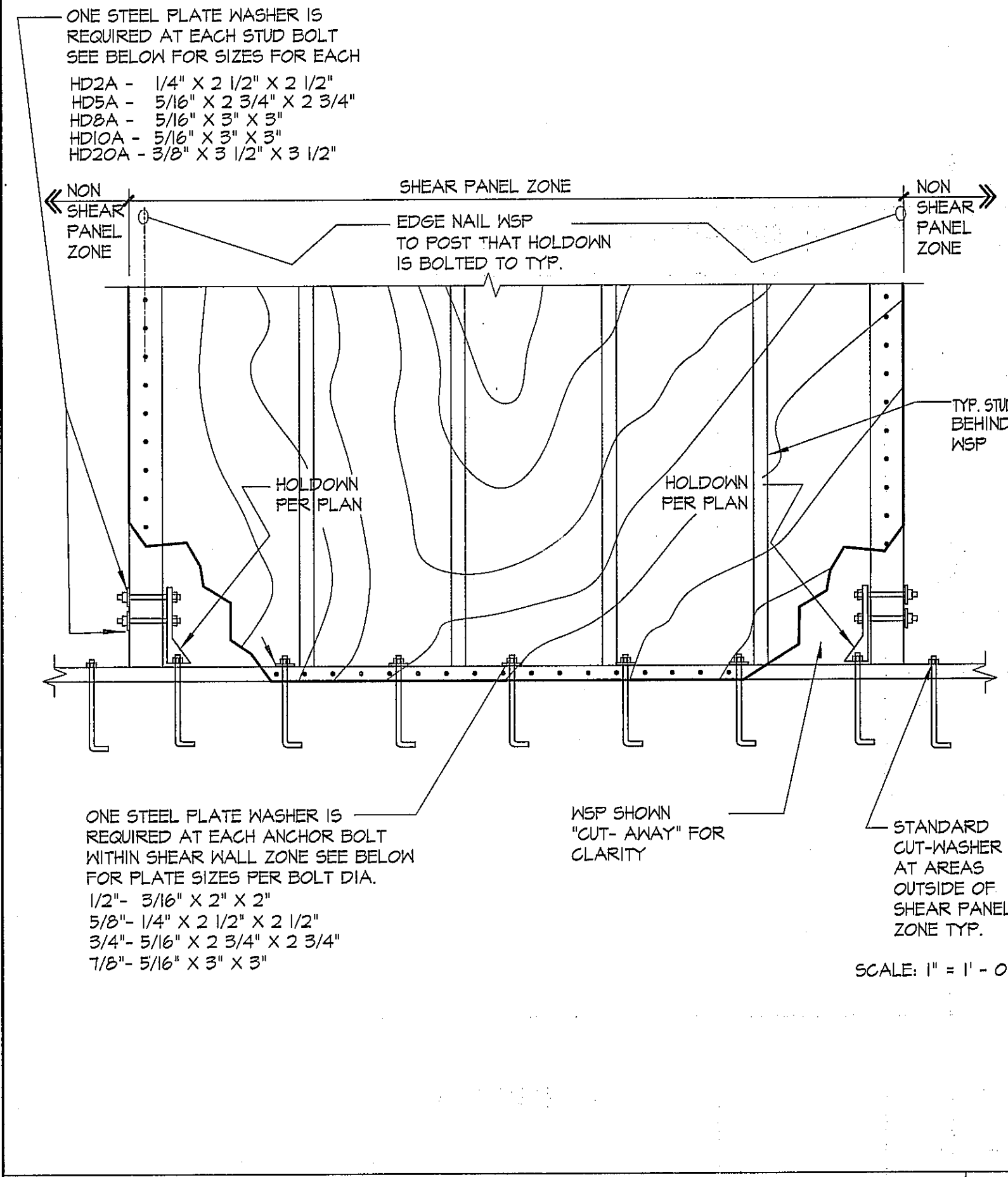
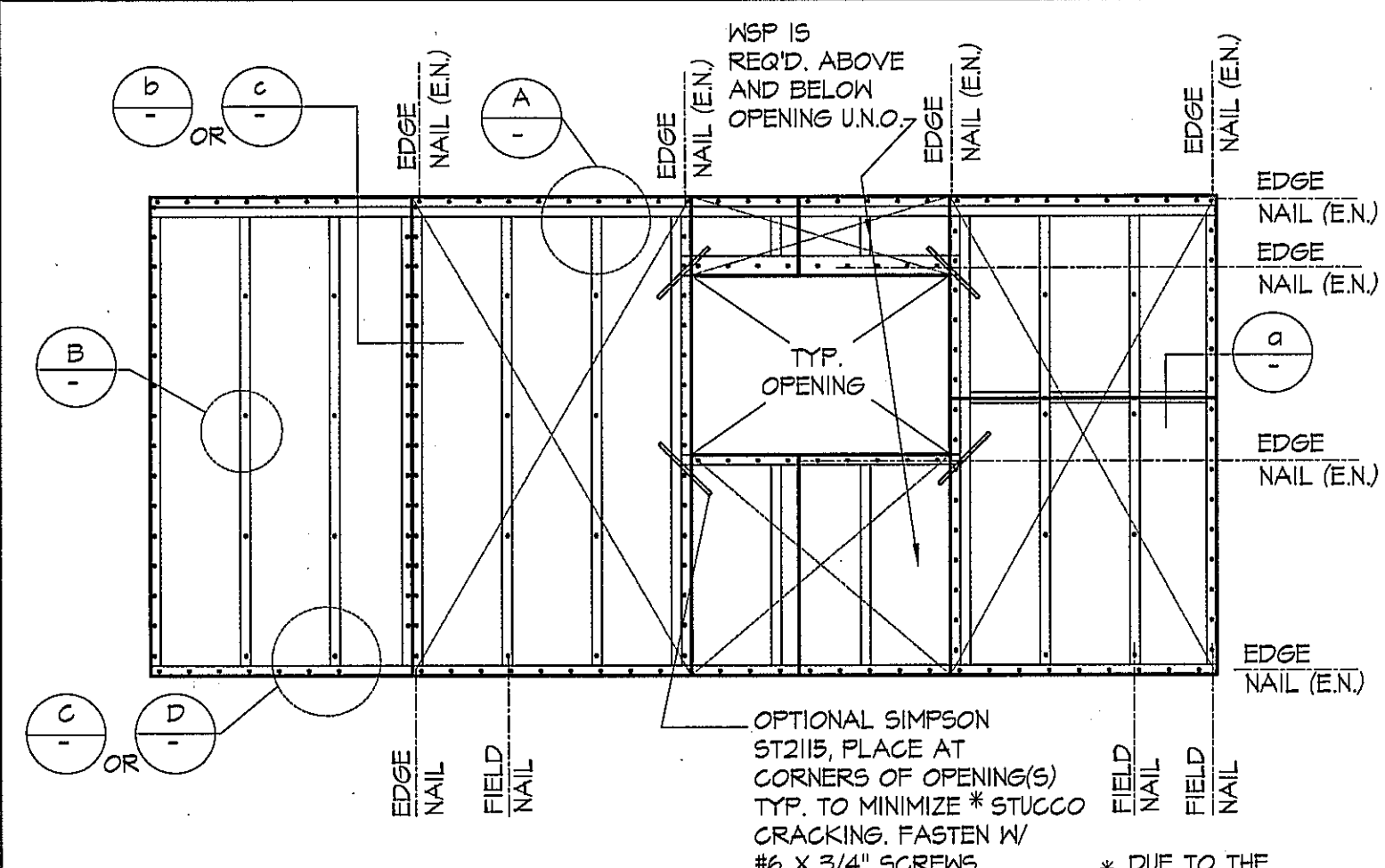
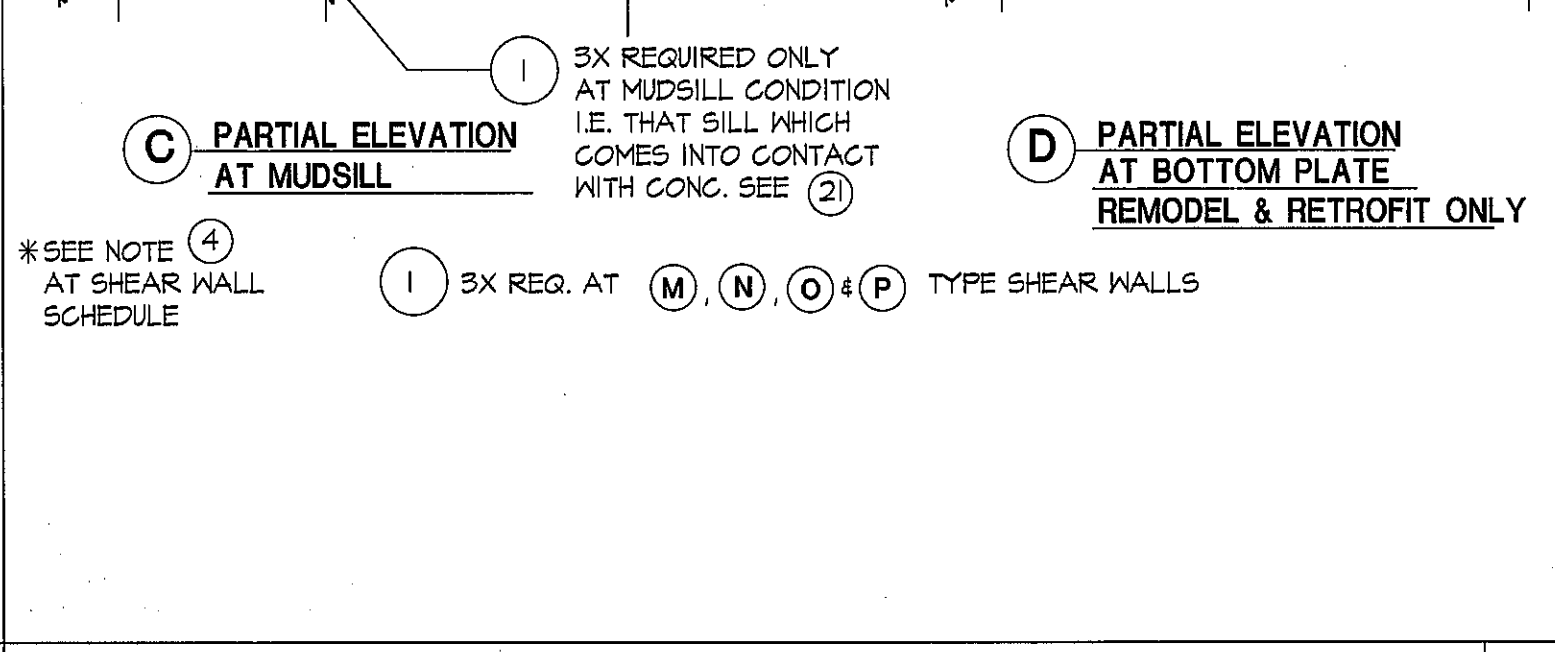
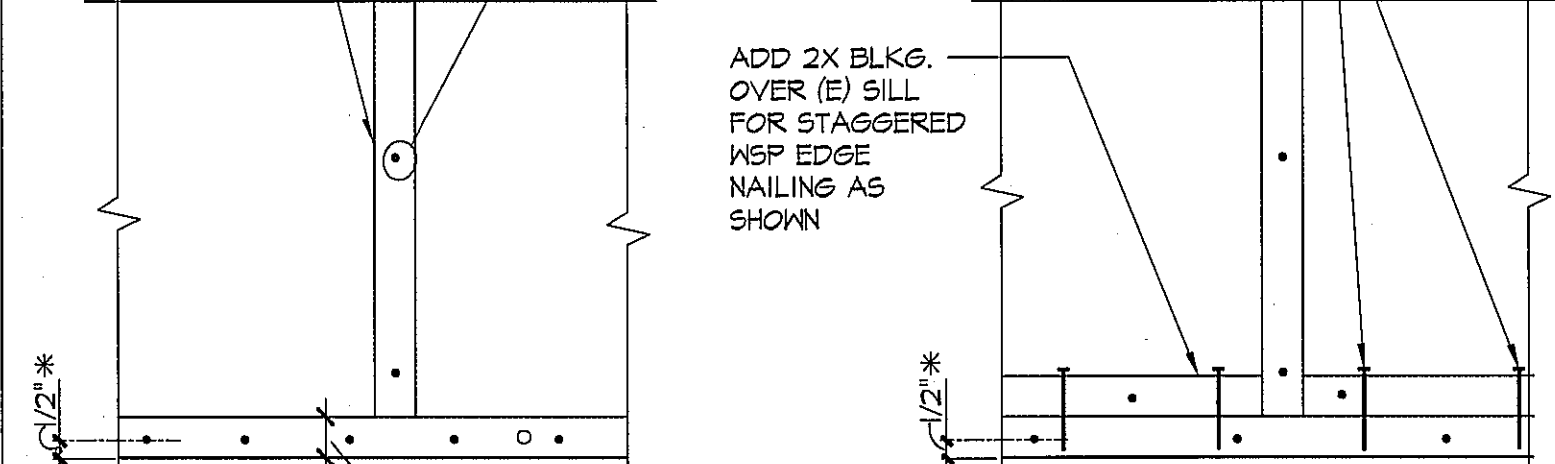
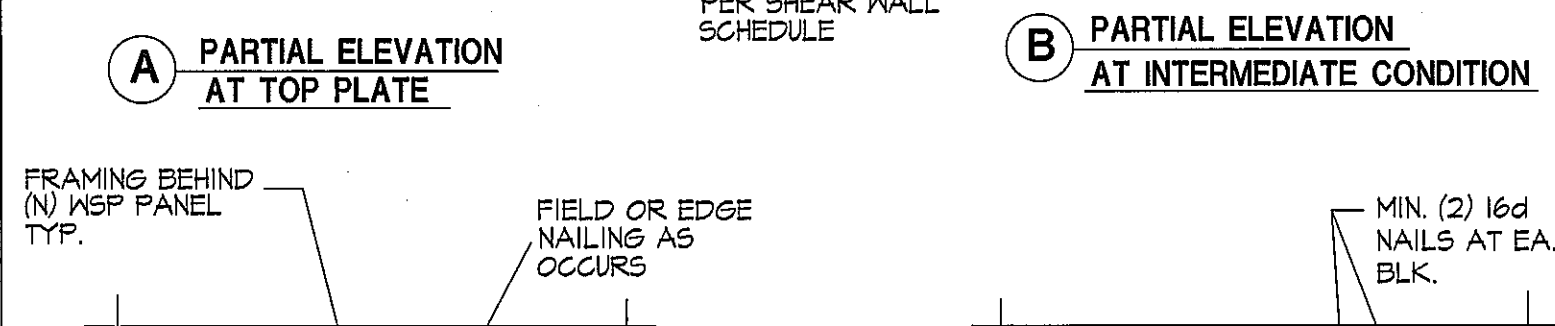
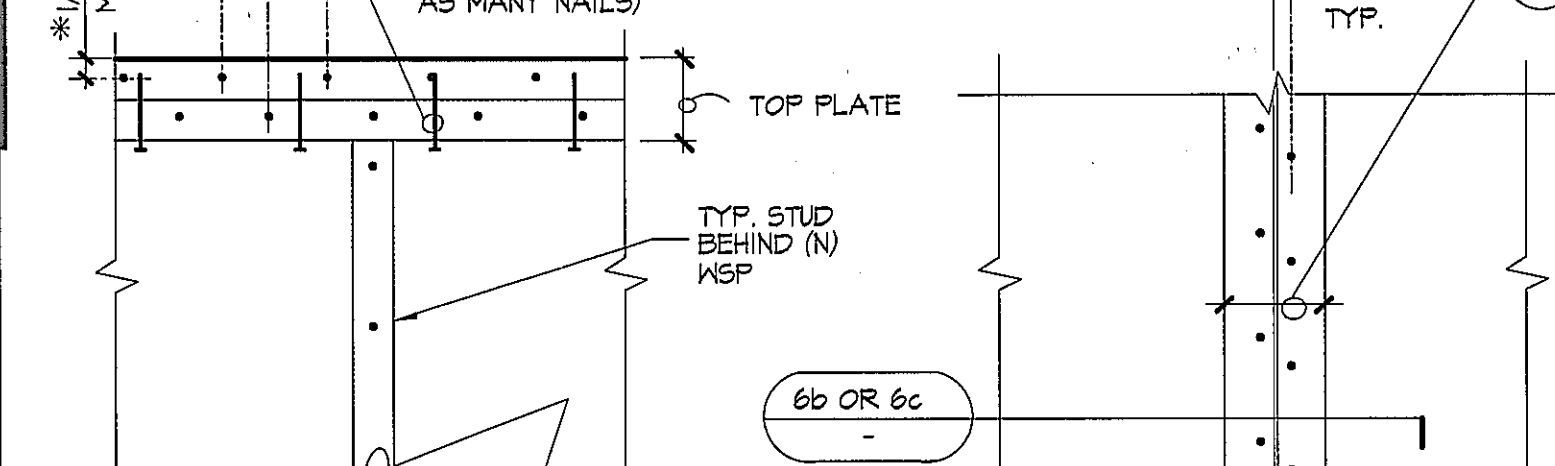
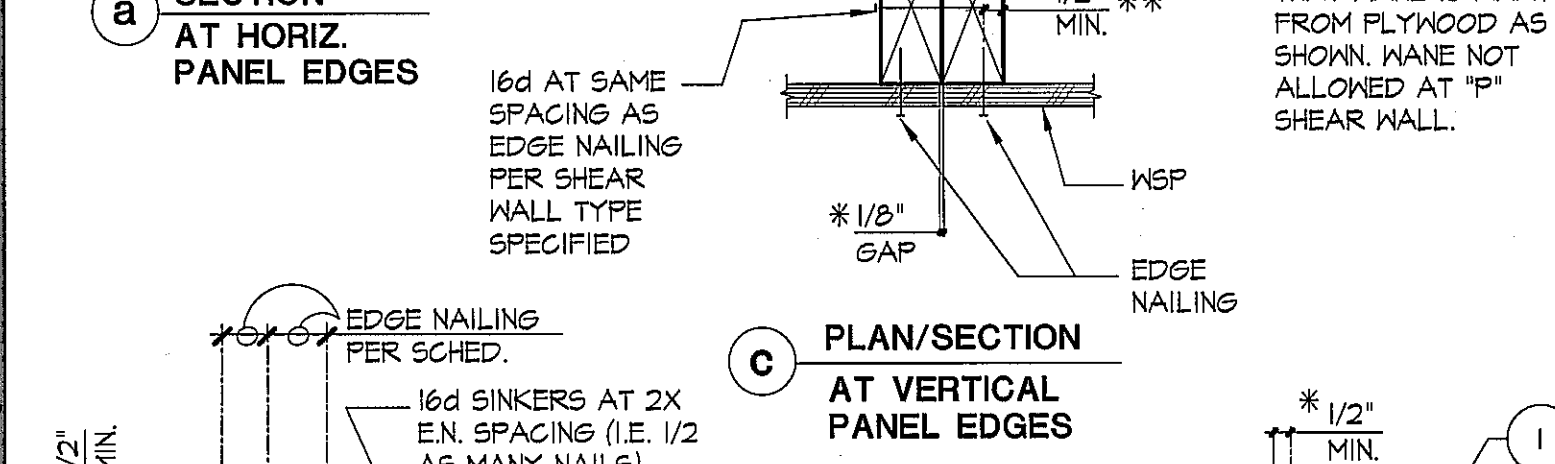
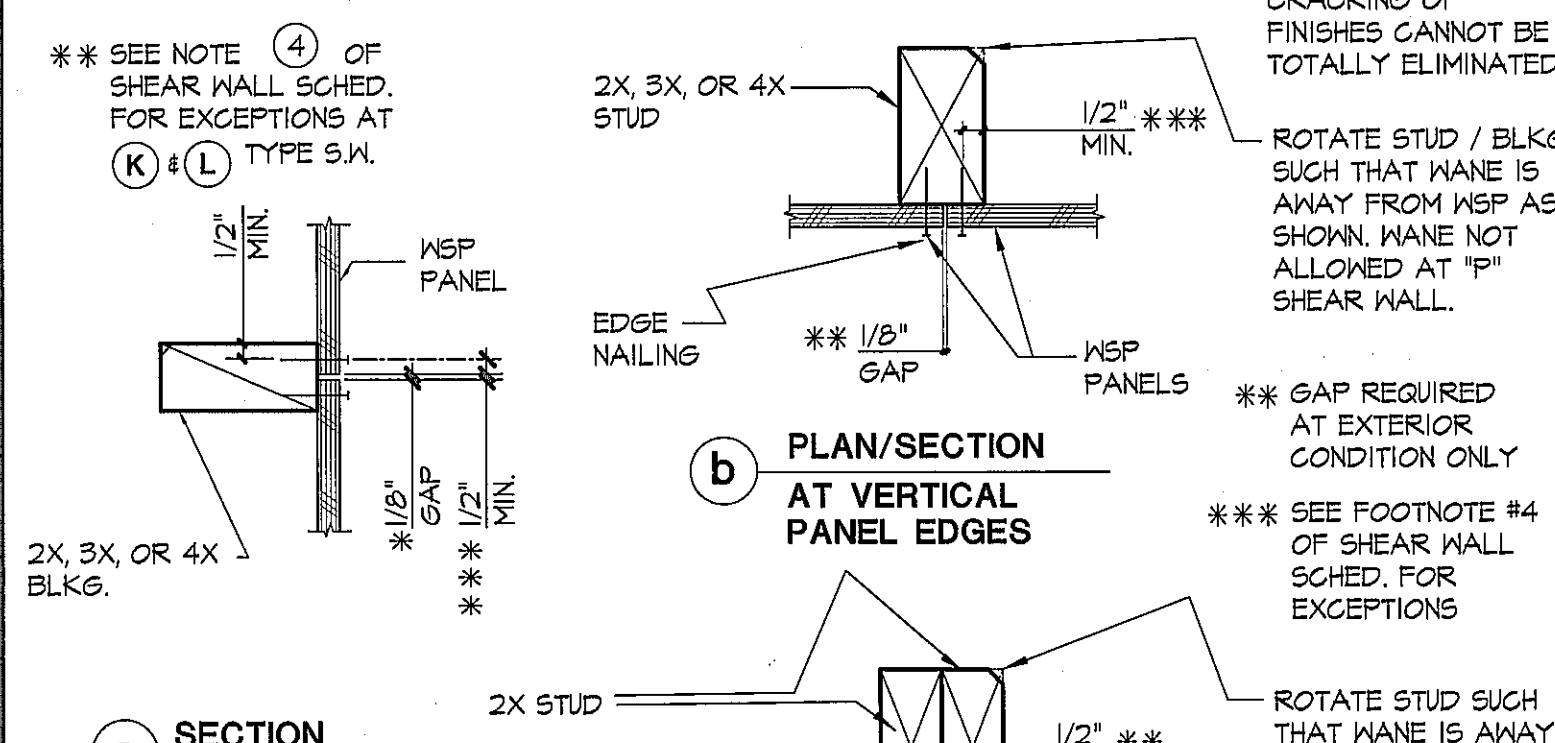


PLATE WASHERS AT HD AND ANCHOR BOLTS 12



SHEAR PANEL NAILING DIAGRAM



SHEAR PANEL NAILING CONDITIONS 6

SHEAR WALL SCHEDULE

MARK	MATERIAL	LAYERS	SIDES	NAILING	BLOCKING REQ.	SILL CONNECTION ①②			DESIGN LOAD CAPACITY	REMARKS (NOTES)		
						WOOD TO WOOD	WOOD TO CONG.	WOOD TO WOOD				
1	3/8" CDX WSP	1	1	8d COMMON @ 6" O/C EDGES 12" O/C FIELD	YES	2 1/2" alc	6" O/C	24" O/C	32" O/C	14C PLF	(4) (23)	
2	1/2" STRUCT. 1 WSP	1	1	8d COMMON @ 4" O/C EDGES 12" O/C FIELD	YES	3 1/4" alc	12" O/C	12" O/C	16" O/C	24C PLF	(4) (22)	
3	1/2" STRUCT. 1 WSP	1	1	10d COMMON @ 4" O/C EDGES 12" O/C FIELD	YES	3X	8" O/C	12" O/C	12" O/C	32B PLF	(3) (10) (12) (16)	
4	1/2" STRUCT. 1 WSP	1	1	10d COMMON @ 3" O/C EDGES 12" O/C FIELD	YES	3X	8" O/C	8" O/C	8" O/C	16" O/C	42B PLF	(3) (10) (12) (16)
5	1/2" STRUCT. 1 WSP	1	1	10d COMMON @ 2" O/C EDGES 12" O/C FIELD	YES	3X	6" O/C	6" O/C	6" O/C	12" O/C	55B PLF	(3) (10) (12) (16)
6	1/2" STRUCT. 1 WSP	1	2	10d COMMON @ 3" O/C EDGES 12" O/C FIELD	YES	4X	4" O/C	5" O/C	4" O/C	6" O/C	72B PLF	(10) (2) (15) (16)
7	1/2" STRUCT. 1 WSP	1	2	10d COMMON @ 3" O/C EDGES 12" O/C FIELD	YES	4X	4" O/C	5" O/C	4" O/C	6" O/C	76B PLF	(24) (10) (2) (15) (16)
8	1/2" STRUCT. 1 WSP	1	2	10d COMMON @ 3" O/C EDGES 12" O/C FIELD	YES	4X	4" O/C	5" O/C	4" O/C	6" O/C	85B PLF	(25) (10) (2) (15) (16)

NOTES:

- ANCHOR BOLTS ARE REQUIRED AT ALL EXTERIOR WALLS AND INTERIOR SHEAR WALLS, SHOT PINS ARE NOT ALLOWED AT SHEAR WALLS AND EXTERIOR WALLS TYPICAL.
- RED HEAD OR OTHER TYPE WEDGE ANCHOR MAY BE SUBSTITUTED FOR A.B. EXCEPT THAT WHERE EDGE DISTANCE IS LESS THAN 4", DRILL & EPOXY (SEE SPECS. ELSEWHERE) IS REQUIRED (CARE SHALL BE TAKEN NOT TO DAMAGE CONCRETE DURING DRILLING APPLICATION) AT EDGES < 4". NOT ALLOWED.
- 3X MEMBERS ARE REQUIRED FOR NEW CONSTRUCTION AT:
 - MUDSILL, SEE NOTE #21 BELOW (3X NOT REQUIRED AT NON-SHEAR WALLS)
 - COMMON EDGE(S), STUDS, POSTS
 - ALL REQUIRED BLOCKING
- AT (K) (L) NAILS SHALL BE PLACED AT LEAST 3/8" FROM PANEL EDGES AND AT LEAST 1/4" FROM THE EDGE OF THE FRAMING MEMBER(S) (STUD, PLATE SILL, BLKG, ETC.).
- NOT USED
- NOT USED
- SEE SHEAR WALL LAYOUT PLAN(S), & DETAILS AND SPECIFICATIONS FOR ADDITIONAL INFORMATION.
- WHERE SEISMIC LOADS CONTROL DESIGN FOR ZONES 3 & 4, DRYWALL DESIGN LOAD CAPACITIES HAVE BEEN REDUCED TO 30 PLF
- SEE DETAILS OTHER SHEETS FOR SHEAR TRANSFER, HOLD DOWN AND DRAG STRUT INSTALLATIONS, ETC.
- (AT EARTHQUAKE REPAIRS ONLY) STAGGERED NAILING AT ADDED 2X BLKG OVER EXISTING BOTTOM PLATE BETWEEN (E) STUDS O.K. IN-LIEU OF REQ'D. 3X, SEE DETAILS
- THE ABOVE ALLOWABLE WSP VALUES ARE 75% OF TABLE 25K-1 BASED UPON CURRENT INDUSTRY STANDARDS AND THE CALIF. STATE CODE.
- 4 PLY MIN. WSP IS REQUIRED FOR SHEAR WALLS OVER 200 PLF.
- STRUCTURAL OBSERVATION IS REQUIRED (SEE NOTE ELSEWHERE ON THIS SHEET AND OTHER SHEETS)
- HOLD DOWN POST BOLT AND SHEAR WALL ANCHOR BOLTS SHALL BE PROVIDED WITH "PLATE" WASHER NOT STANDARD "CUT" WASHER AND SHALL BE AS SHOWN IN DETAIL #12 OF THIS SHEET.
- 3X WALL REQ. I.E. 3 X 4 STUDS, 3 X 4 SILL, 3 X 4 TOP/TOP PLATE END POST, ETC.
- PRE-DRILL OR SCREW ALL A35s W/ SPACING LESS THAN 8" O.C.
- DOUGLAS-FIR OR SOUTHERN PINE FRAMING (S.G. 0.41 MINIMUM). ALL PANEL EDGES FASTENED TO FRAMING.
- ALL PANEL EDGES BACKED WITH 2-INCH NOMINAL OR WIDER FRAMING. WSP INSTALLED EITHER HORIZONTALLY OR VERTICALLY
- WHERE WSP IS INSTALLED ON BOTH FACES OF A WALL AND NAIL SPACING IS LESS THAN 6 INCHES ON CENTER ON EITHER SIDE PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS OR FRAMING SHALL BE 3-INCH OR THICKER AND NAILS ON EACH SIDE SHALL BE STAGGERED.
- WSP IS AN ABBREVIATION FOR WOOD STRUCTURAL PANEL WHICH IS DEFINED AS "WOOD STRUCTURAL PANEL" WHICH IS TO INCLUDE PROPERLY GRADE STAMPED, (OSB) ORIENTED STRAND BOARD, WAFERBOARD, COMPOSITE PANELS, AND CONVENTIONAL VENEER PLYWOOD. NOTE THAT PANEL(S) SHOULD BE INSTALLED SUCH THAT THE GRADE STAMP IS EASILY VISIBLE FROM BLDG. EXTERIOR PANEL(S) INSTALLED WITHOUT GRADE STAMP ARE SUBJECT TO REMOVAL BY ENGINEER DURING STRUCTURAL OBSERVATION VISIT(S)
- MUDSILL IS DEFINED AS THE SILL WHICH RESTS ON CONCRETE, (FTG, DECK, SLAB ETC.) AND EN. WSP AND HAS WSP EN. TO SAME (I.E. A.B. & EN. TO SAME MEMBER WHETHER OR NOT IT IS PRESSURE TREATED).
- 15/32" OR 5/8" WSP CAN BE SUBSTITUTED WITHOUT WRITTEN APPROVAL PROVIDED REQUIRED NAIL PENETRATION IS ACHIEVED AND ALL OTHER SPECIFICATIONS ARE FULFILLED.
- SEE ALSO DETAIL 9/5-3.2 FOR ALTERNATE LAG SCREWS AT SILL CONNECTIONS
- REDUCE OPENING FROM 15% TO 1 1/2" (1/2 VALUES SHOWN IN TABLE 2/5-3.2)
- FULL VALUE USED IN CALCULATIONS, NO REDUCTION ALLOWED (STRUCT. OBSERV. ENGR. NOTE % ON PLANS)

SHEAR WALL SCHEDULE, FOOTNOTES & A35 ORIENTATION OPTIONS 2

CALLOUT (MODEL NO. & RATING)	ANCHOR DIAM.	NO SPECIAL INSPECTION		WITH SPECIAL INSPECTION		ANCHOR BOLT EMBEDMENT			MIN. POST	POST SCREWS	LOAD GAP (LBS)	
		SET INTO WET CONCRETE (5)	DRILLED AND EPOXY SET	SET INTO WET CONCRETE	DRILLED AND EPOXY SET (7)	SET INTO WET CONCRETE	DRILLED AND EPOXY 6" STEM	DRILLED AND EPOXY 8" STEM & 12" ON GRADE FTG				
PHD2	5/8"	2,464	NOT ALLOWED	2,464	2,464	2,464	12"	10"	8"	(2) 2 X 4	10-SDS/4X3	2,464
PHD5	3/4"	3,375	NOT ALLOWED	3,375	3,375	3,375	15"	15"	11"	(2) 2 X 4	14-SDS/4X3	3,375
PHD8	7/8"	5,340	NOT ALLOWED	5,340	5,340	5,340	20"	18"	18"	4 X 4	20-SDS/4X3	5,340
DBL. PHD8 (LOW)	7/8"	8,010	NOT ALLOWED	8,010	8,010	8,010	24"	24"	24"	4 X 6	48-SDS/4X3 (24-SDS/6 EA)	8,010
DBL. PHD8 (HIGH)	7/8"	10,680	NOT ALLOWED	10,680	10,680	10,680	30"	30"	30"	4 X 6	48-SDS/4X3 (24-SDS/6 EA)	10,680

(ALLOWABLE LOADS FOR SIMPSON STRONG-TIE® HOLD-DOWN HARDWARE IN POUNDS) (2)

FOOTNOTES:

- INFORMATION SHOWN HERE IS FROM L.A. BLDG. & SAFETY "HOLD-DOWN QUICK REFERENCE CHART" WHICH REFERENCES TABLE 26-E (TENSION ONLY) OF THE LOS ANGELES CITY BUILDING CODE.
- VALUES SHOWN ARE IN COMPLIANCE WITH BLDG. DEPT. MEMO DATED MAY 20, 1994 WHICH REQUIRES:
 - BOLTS HOLES IN WOOD POST MIN. 1/32" TO MAX 1/16" LARGER THAN POST BOLT DIA.
 - HOLD-DOWN CONNECTORS TO BE TIGHTENED JUST PRIOR TO COVERING WALL FRAMING
- BLDG. DEPT. MEMO DATED NOV. 30, 1994 REQUIRES "APPROVED PLATE WASHERS" IN-LIEU OF CUT WASHERS FOR THE HOLD-DOWN CONNECTOR BOLTS AT THE POST (SEE ALSO 3/5-3.1)
- SEE SPECIFICATION SHEET(S) 5-1.1 OR 5-1.2 FOR ADDITIONAL SIMPSON STRONG-TIE HARDWARE INFORMATION.
- VALUES FOR WET CONCRETE ARE BASED UPON ALTHREAD W/ PL. WASHER ANCHOR AND TRIDITIONAL A.B. OR SSTB A.B. NOT ALLOWED.
- SEE ADDITIONAL SIMPSON STRONG-TIE SPECIFICATIONS ON SHEET 5-1.1 OR 5-1.2 VALUES ASSUME A.B. PLACED AT CENTER OF SLAB ON GRADE FOOTING. IF NOT CENTER OR IF RAISED WOOD FLOOR USE VALUES TWO COLUMNS TO THE LEFT (2 X 4 AT FTG. EDGE).
- DRILL EPOXY INTO STEM WALLS TO BE AT THE CENTER OF THE WALL AND 1 3/4" FOR SLAB ON GRADE DRILLED 1/6 DIAGONALLY FOR 3" AVERAGE (SEE FOOTING DETAILS).

HOLD-DOWN AND POST SCHEDULE 3

REVISIONS BY

HRD ENGINEERING
STRUCTURAL DESIGN AND DRAFTING
7463 VARNA AVE., SECOND FLOOR
N. HOLLYWOOD, CA 91605
PHONE NO. (805) 431-5415 - FAX NO. (805) 888-4840

SHEAR WALL SCHEDULE

TWO STORY SINGLE FAMILY RESIDENCE
901 LAUREL CYN
LA, CA 90046 FOR: L.I. INVESTMENTS, LLC

DATE 12-18-12
SCALE
DRAWN hrd
JOB
SHEET S-3.1

STRUCTURAL OBSERVATION CHECKLIST

THE FOLLOWING CHECKLIST IS PROVIDED TO SHOW THE CONTRACTOR(S)/SUBCONTRACTOR(S) WHAT THIS OFFICE WILL BE LOOKING FOR DURING STRUCTURAL OBSERVATIONS.

- I. PLYWOOD
 1. PLYWOOD THICKNESS
 2. PLYWOOD SIZE (EACH PIECE)
 3. PLYWOOD GRADE
- II. NAILS
 1. NAIL SIZE
 2. NAIL SHANK DIAMETER
 3. NAIL HEAD DIAMETER
- III. FRAMING
 1. PLYWOOD BOUNDARY FRAMING 3X WHERE REQUIRED
 2. PLYWOOD EDGE BLOCKING/FRAMING 3X WHERE REQUIRED
- IV. APPLICATION (NOTE: 15% MARGIN OF ERROR ALLOWED)
 1. NAIL PENETRATION INTO FRAMING
 2. NAIL HEAD PENETRATION INTO PLYWOOD LAMS.
 3. NAIL SPACING
 4. EDGE DISTANCE
 5. SHEAR WALL LENGTH
 6. EXTERIOR PLYWOOD GAPPED 1/8" BETWEEN SHEETS
- V. SHEAR TRANSFER TOP AND DRAG CONNECTIONS
 1. ANGLE CLIPS SPACING
 2. ANGLE CLIPS NAILED CORRECTLY
 3. DRAG CONNECTION TO SHEAR WALL
- VI. SHEAR TRANSFER BOTTOM
 1. SILL NAILING OR ANCHOR BOLTING
 2. ANCHOR BOLT PLATE WASHERS
 3. ANCHOR BOLT NUT TIGHT AND ALL THREADS ENGAGED
- VII. HOLD DOWN
 1. PLYWOOD EDGE NAILED TO POST WITH HOLD DOWN
 2. ANCHOR BOLT SIZE / EMBEDMENT
 3. HOLD DOWN SIZE
 4. HOLES DRILLED IN POST 1/8" MAX. LARGER THAN BOLT DIAMETER
 5. HOLD DOWN POST BOLTS WITH CORRECT PLATE WASHERS
 6. HOLD DOWN NUTS TIGHT AND ALL THREADS ENGAGED
- VIII. OPENINGS IN PLYWOOD PANEL
 1. HOLES / NOTCHING OF PLYWOOD
- IX. MISC.
 1. WANE NOT A PROBLEM (SEE FOOT NOTE 1)
 2. ALL PLYWOOD EDGES BLOCKED CORRECTLY
 3. ADJOINING PANEL EDGES A SINGLE PIECE (3X WHERE REQUIRED)
 4. 2-2X NAILED TOGETHER AT ADJOINING PANEL EDGES (SEE FOOT NOTE 4)
 5. PLYWOOD CONTINUOUS AT INTERSECTING WALL(S)
 6. SHEAR TRANSFER CLIP ANGLES WITH CORRECT ORIENTATION
 7. CUTTING / NOTCHING FRAMING MEMBER(S)
 8. SHEAR TRANSFER CLIP ANGLES ORIENTED CORRECTLY
 9. PLYWOOD EDGE NAILED TO BOTTOM OF DOUBLE SILL PLATE (IF DOUBLED)
 10. PLYWOOD EDGE NAILED TO TOP OF DOUBLE TOP PLATE?
 11. SHEAR WALL LENGTH AS SHOWN ON PLANS
 12. PLYWOOD TWO SIDES WITH STAGGERED ADJOINING PANEL EDGES
 13. EDGE NAILS STAGGERED
 14. PLATES PROPER GRADE (D.F.) NOTE THAT SOMETIMES P.T. SILLS ARE H.F.
 15. HOLD DOWN ORIENTATION
 16. METAL STRAPS/CLIPS OVER/UNDER PLYWOOD PER PLANS

- NOTE:
1. WANE IS THE CORNER OF THE STUD/PLATE THAT IS MISSING BECAUSE THIS IS THE FACE OF THE TREE AT THE BARK. PIECE OF WOOD WAS CUT AT THE EDGE OF THE LOG (IE. THE WANE SURFACE)
 2. D.F. IS DOUGLAS FIR
 3. H.F. IS HEM FIR
 4. DUE TO SEISMIC DAMAGE ONLY
2 - 2X NOT ALLOWED FOR NEW CONSTRUCTION, ONLY RETROFIT OR UPGRADES

NOTE: ANY "NOT TO CODE / PLANS" RESPONSE TO ANY ITEM CONSTITUTES A FAILED WALL. SEE PLANS AND SPEC'S FOR SPECIFIC INFO. A FAILED WALL REQUIRES EVALUATION BY ENGINEER FOR CORRECTIVE ACTION

ADDITIONAL STRUCTURAL OBSERVATION NOTES

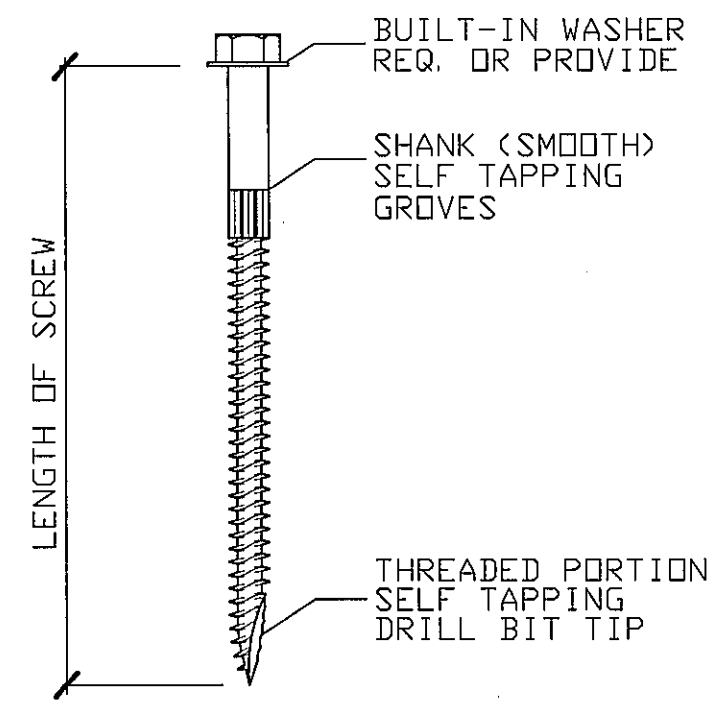
THE CONTRACTOR IS RESPONSIBLE FOR NOTIFYING THIS OFFICE 12 HOURS IN ADVANCE OF REQUIRED OBSERVATIONS FOR SCHEDULING PURPOSES. FAILURE TO MEET OBSERVATION SCHEDULES MAY REQUIRE REMOVAL FOR OBSERVATION PURPOSES ANY FINISHES THAT HAVE BEEN SUBSEQUENTLY INSTALLED. APPROVAL BY THE CITY INSPECTOR DOES NOT PRECLUDE OBSERVATION BY THE STRUCTURAL ENGINEER. REMOVAL AND REPLACEMENT OF ANY FINISHES AND/OR FRAMING DAMAGED BY THE FINISH REMOVAL PROCESS OR AS REQUIRED FOR CORRECTIVE ACTION SHALL BE AT THE CONTRACTOR'S EXPENSE (NOT THE OWNER, STRUCTURAL ENGINEER OR THIS OFFICE). FOR FAILURE TO COMPLY WITH THESE PLANS AND SPECIFICATIONS, NAMELY THAT THE STRUCTURAL ENGINEER SHALL PROVIDE STRUCTURAL OBSERVATION FOR PLYWOOD SHEAR WALLS OVER 300 PLF. THE ENGINEER OR THIS OFFICE DOES NOT GUARANTEE THE PERFORMANCE OF AND SHALL HAVE NO RESPONSIBILITY FOR THE ACTS, ERRORS AND/OR OMISSIONS OF ANY CONTRACTOR, SUBCONTRACTOR, SUPPLIER AND/OR ANY OTHER ENTITY FURNISHING MATERIALS OR PERFORMING ANY WORK ON THIS PROJECT.

15% TABLE				
LENGTH	0"	3"	6"	9"
2'-	3 5/8"	4"	4 1/2"	5"
3'-	5 1/2"	5 7/8"	6 3/8"	6 3/4"
4'-	7 1/4"	7 5/8"	8 1/8"	8 1/2"
5'-	9"	9 1/2"	9 7/8"	10 3/8"
6'-	10 7/8"	11 1/4"	11 3/4"	12 1/8"
7'-	12 5/8"	13"	13 1/2"	14"
8'-	14 3/8"	14 7/8"	15 3/8"	15 3/4"
9'-	16 1/4"	16 5/8"	17 1/8"	17 1/2"
10'-	18"	18 3/8"	18 7/8"	19 3/8"

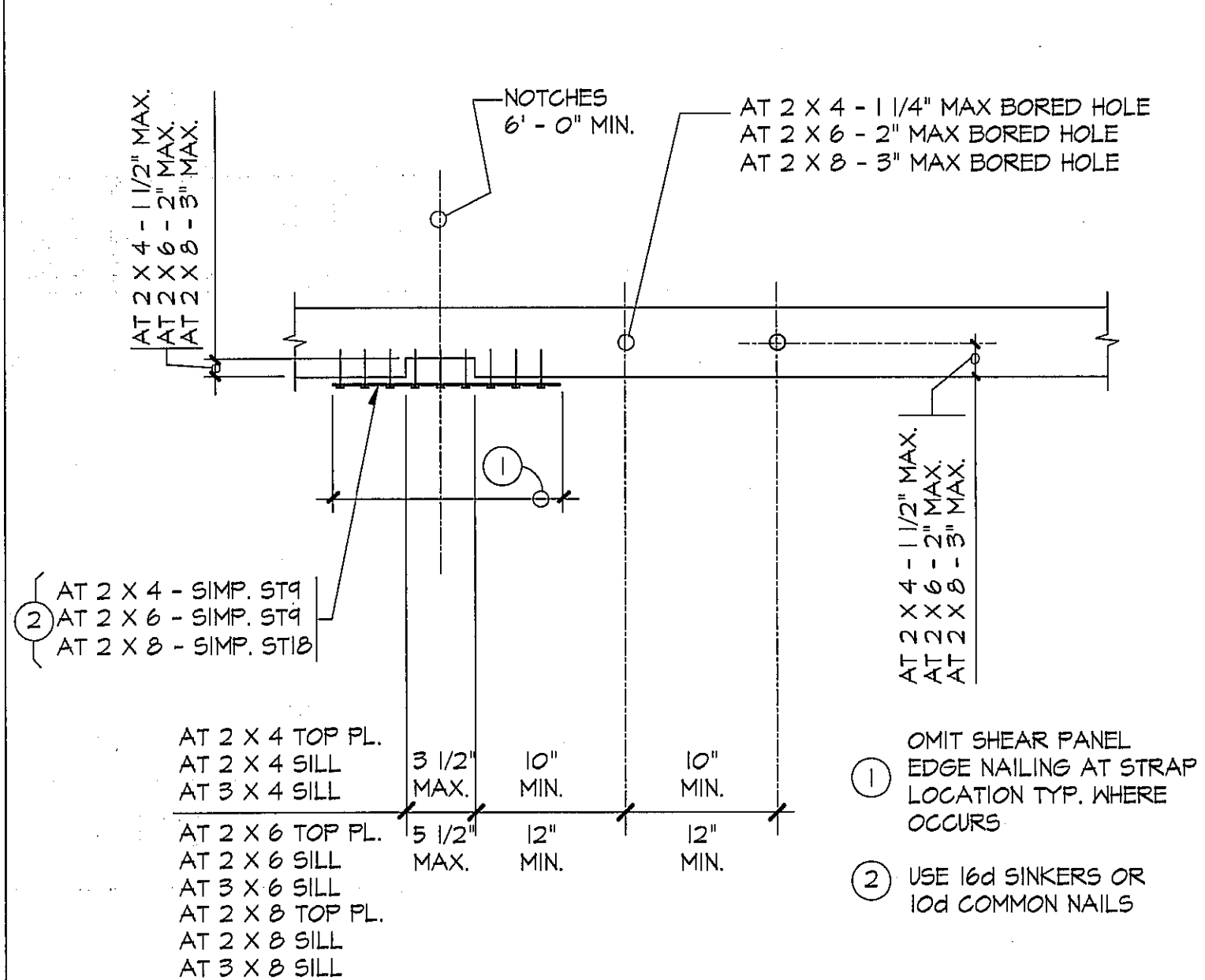
EXAMPLE #1: IF SHEAR WALL LENGTH IS 4' - 9" MAXIMUM OPENING ALLOWED IS 8 1/2"

EXAMPLE #2: IF SHEAR WALL LENGTH IS 12' - 3", MAXIMUM OPENING ALLOWED IS 22" (BECAUSE 12' - 3" = 10' - 0" + 2' - 3", THEREFORE 18' + 4" = 22")

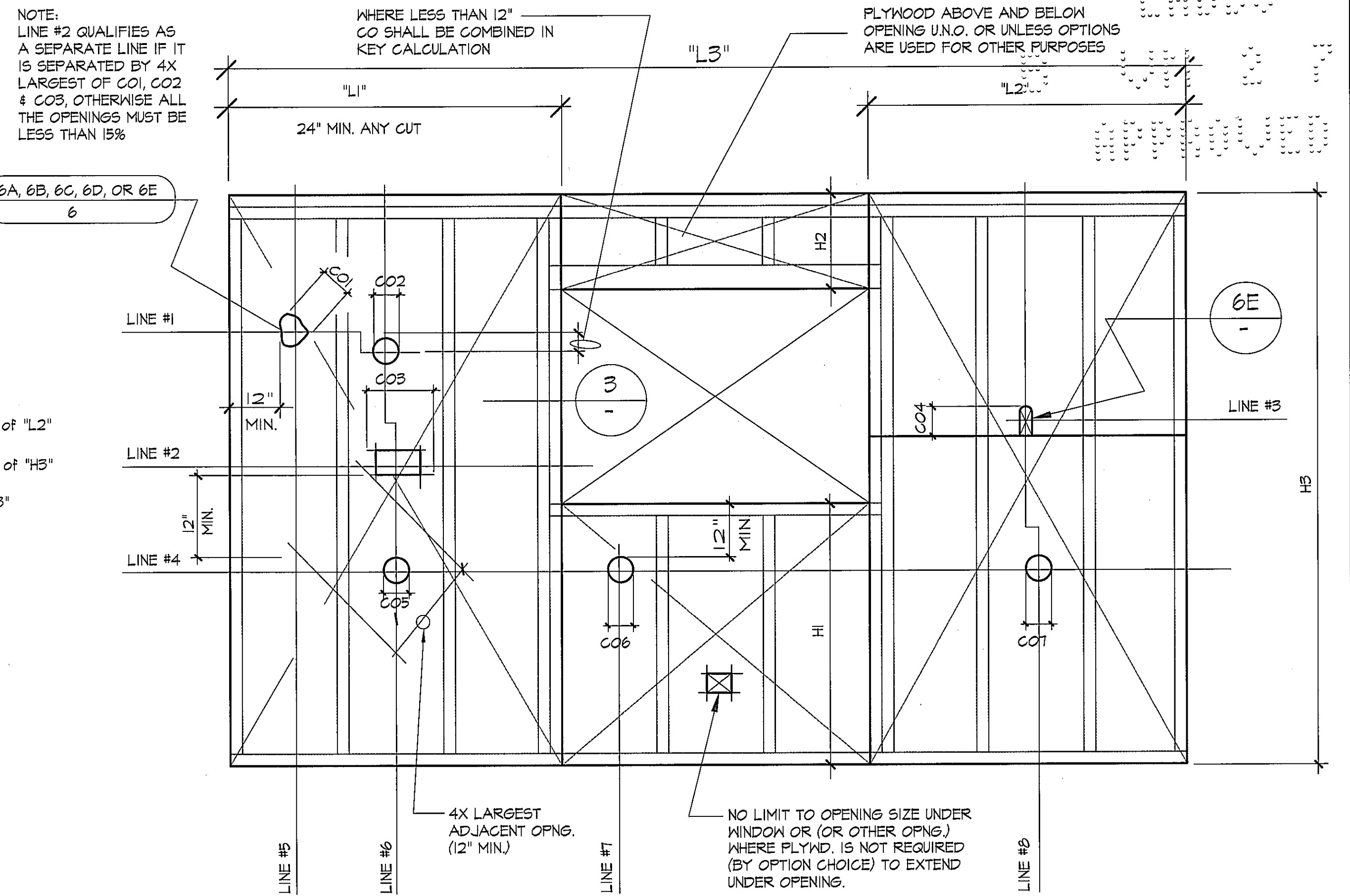
SHEAR WALL	LAG SCREW	
	1/4" X 5"	1/4" X 6"
K	7"	14"
L	4"	8"
M	3 1/2"	7"
N	3"	6"
O	2"	4"
P	N/A	N/A
Q	N/A	N/A



OPTIONAL SELF-DRILL (SELF-TAPPING) SILL CONN. SCREWS



12 TOP / SILL PLATE NOTCHING AT SHEAR WALLS

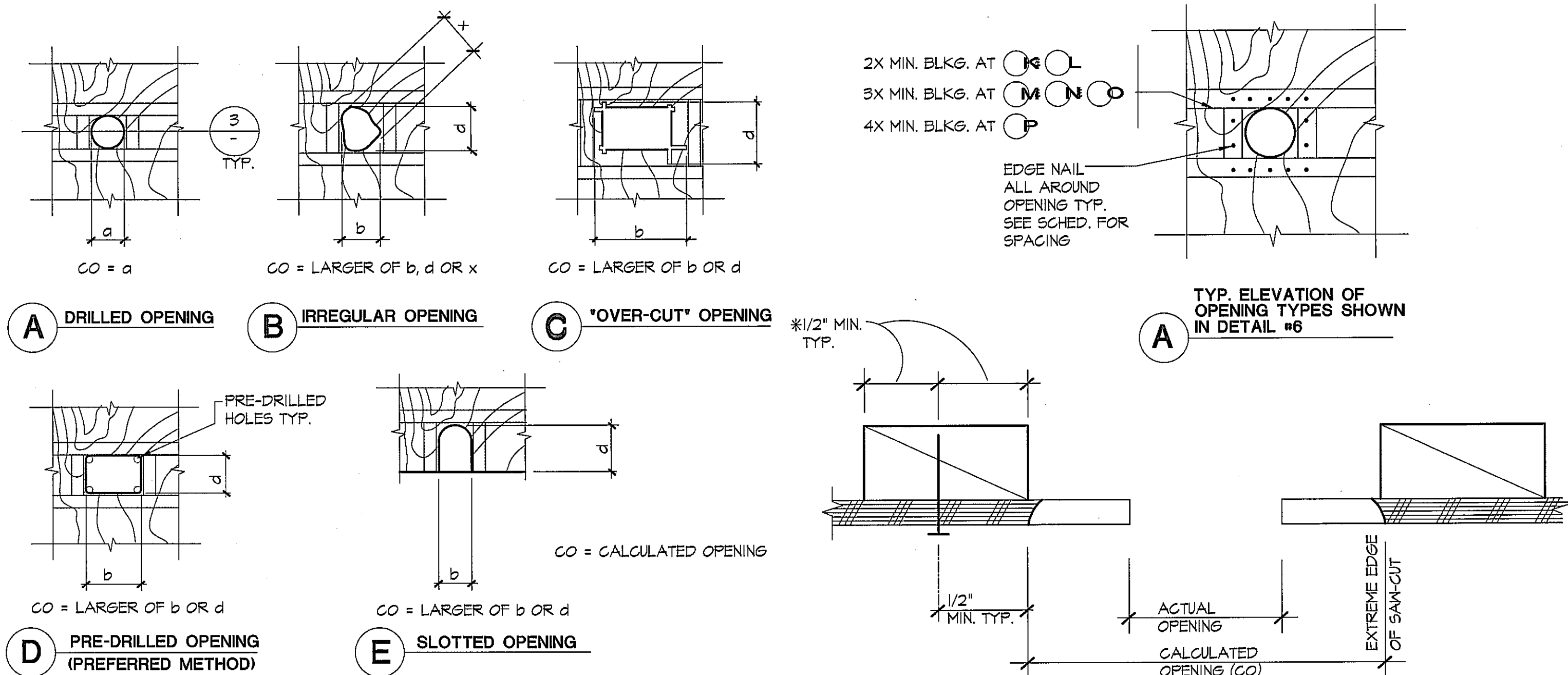


KEY

LINE #1 = CO1 + CO2 < 15% of "L1"
 LINE #2 = CO3 < 15% of "L1"
 LINE #3 = CO4 < 15% of "L2"
 LINE #4 = CO5 + CO6 + CO7 < 15% of "L2"
 LINE #5 = CO1 < 15% of "H3"
 LINE #6 = CO2 + CO3 + CO5 < 15% of "H3"
 LINE #7 = CO6 < 15% of "H1"
 LINE #8 = CO4 + CO7 < 15% of "H3"

ABBREVIATIONS

- (E) = EXISTING
 GA = GAUGE
 PLF REQ. = POUNDS PER LINEAL FOOT
 O.C. = ON CENTER
 φ = DIAMETER
 CO = CALCULATED OPENING



3 TOP / SILL PLATE NOTCHING AT SHEAR WALLS

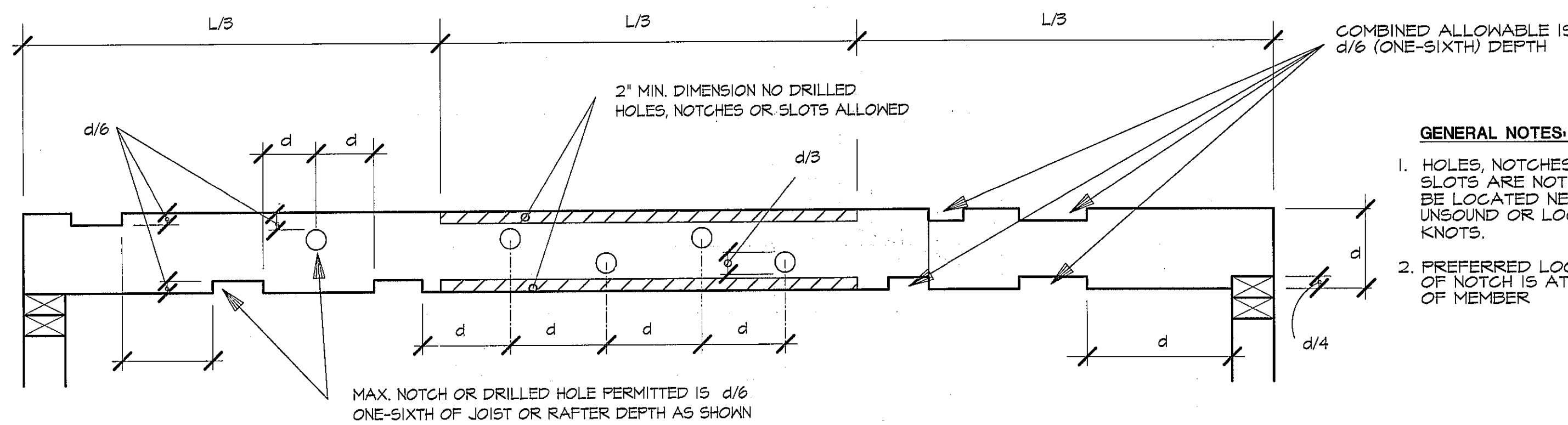
REVISIONS BY

HRD ENGINEERING
 STRUCTURAL DESIGN AND DRAFTING
 7463 VARNA AVE., SECOND FLOOR
 N. HOLLYWOOD, CA 91605
 PHONE NO. (805) 431-5415 - FAX NO. (805) 888-4840

SHEAR WALL DETAILS

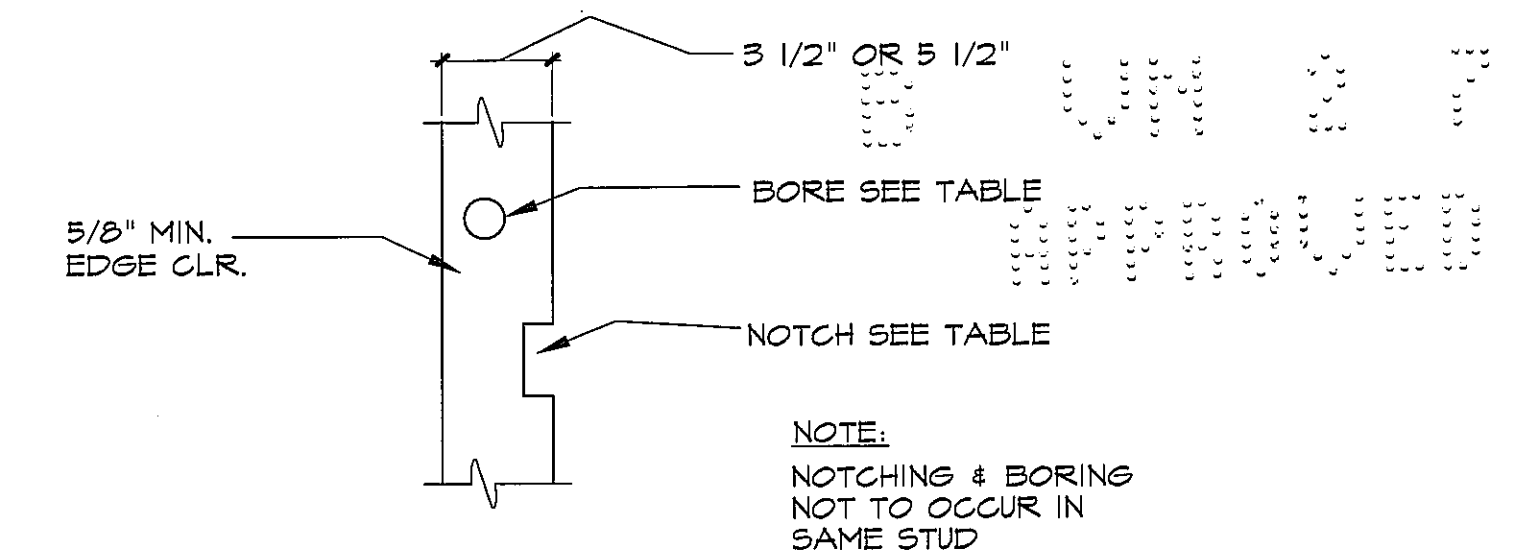
TWO STORY SINGLE FAMILY RESIDENCE
 901 LAUREL CYN.
 LA, CA 90046 FOR: L.I. INVESTMENTS, LLC

Date 12-18-12
 Scale
 Drawn hrd
 Job
 Sheet 6-3.2



NOMINAL	ACTUAL	d/6	d/4	d/3
4"	3 1/2"	9/16"	7/8"	1 1/8"
6"	5 1/2"	7/8"	1 3/8"	1 13/16"
8"	7 1/4"	1 3/16"	1 13/16"	2 7/16"
10"	9 1/4"	1 1/2"	2 5/16"	3 1/16"
12"	11 1/4"	1 7/8"	2 13/16"	3 3/4"
14"	13 1/4"	2 3/16"	3 5/16"	4 7/16"

- GENERAL NOTES:**
- HOLES, NOTCHES & SLOTS ARE NOT TO BE LOCATED NEAR UNSOUND OR LOOSE KNOTS.
 - PREFERRED LOCATION OF NOTCH IS AT TOP OF MEMBER



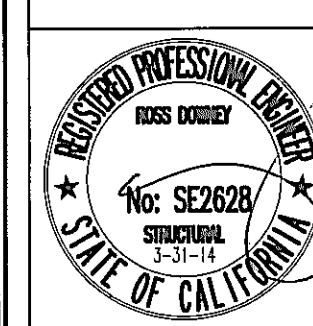
	MULTIPLE STUDS	SINGLE STUDS	
		BEARING	NON-BEARING
BORE	60% MAX	40% MAX	60% MAX
2 X 4	2 1/8"	1 3/8"	2 1/8"
2 X 6	3 3/8"	2 1/4"	3 3/8"
NOTCH	NA	25% MAX	40% MAX
2 X 4	NA	7/8"	1 3/8"
2 X 6	NA	1 3/8"	2 1/4"

REVISIONS BY

HRD ENGINEERING
STRUCTURAL DESIGN AND DRAFTING
7463 VARNA AVE., SECOND FLOOR
N. HOLLYWOOD, CA 91605
PHONE NO. (805) 431-5415 - FAX NO. (805) 888-4840

FRAMING DETAILS

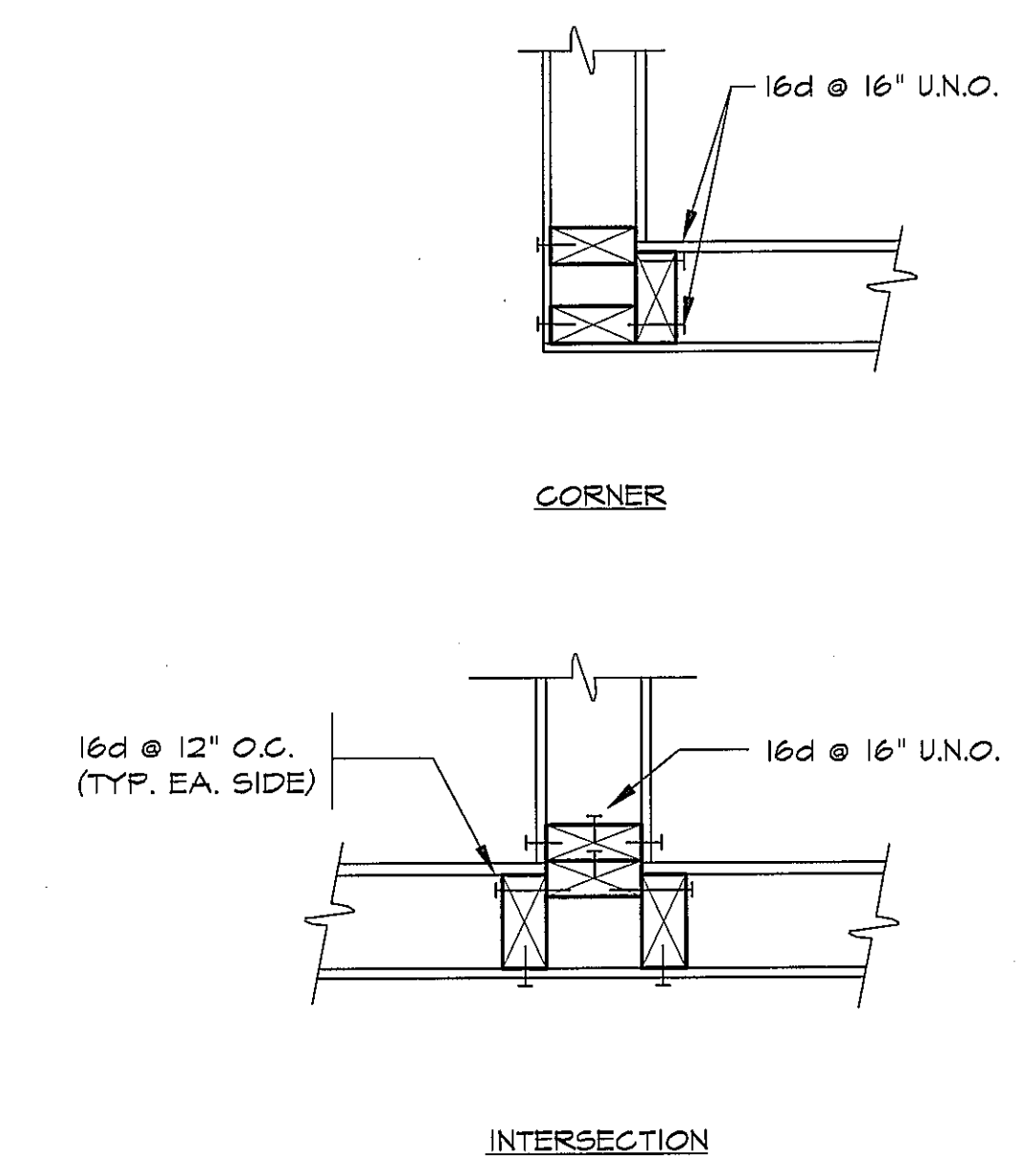
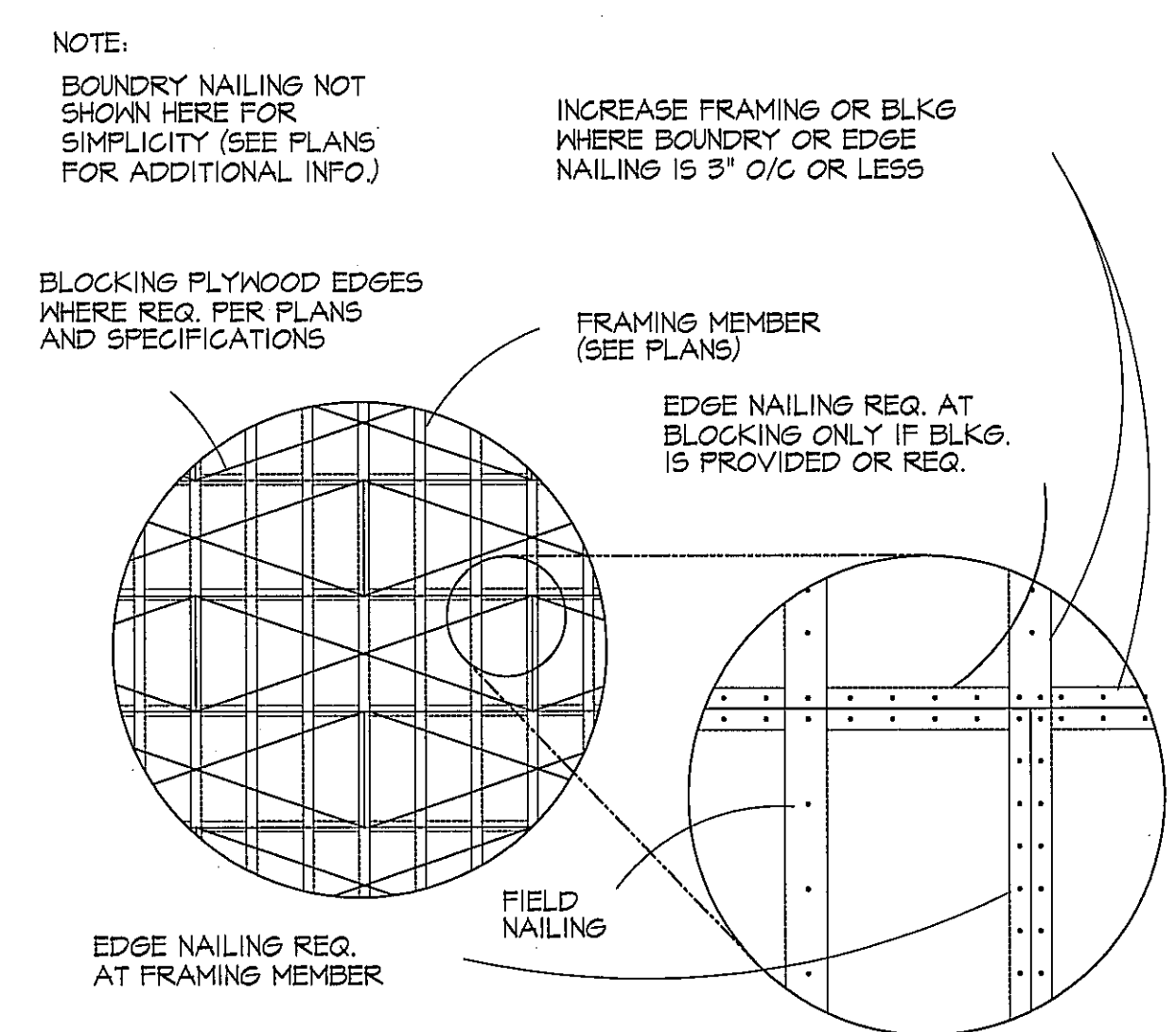
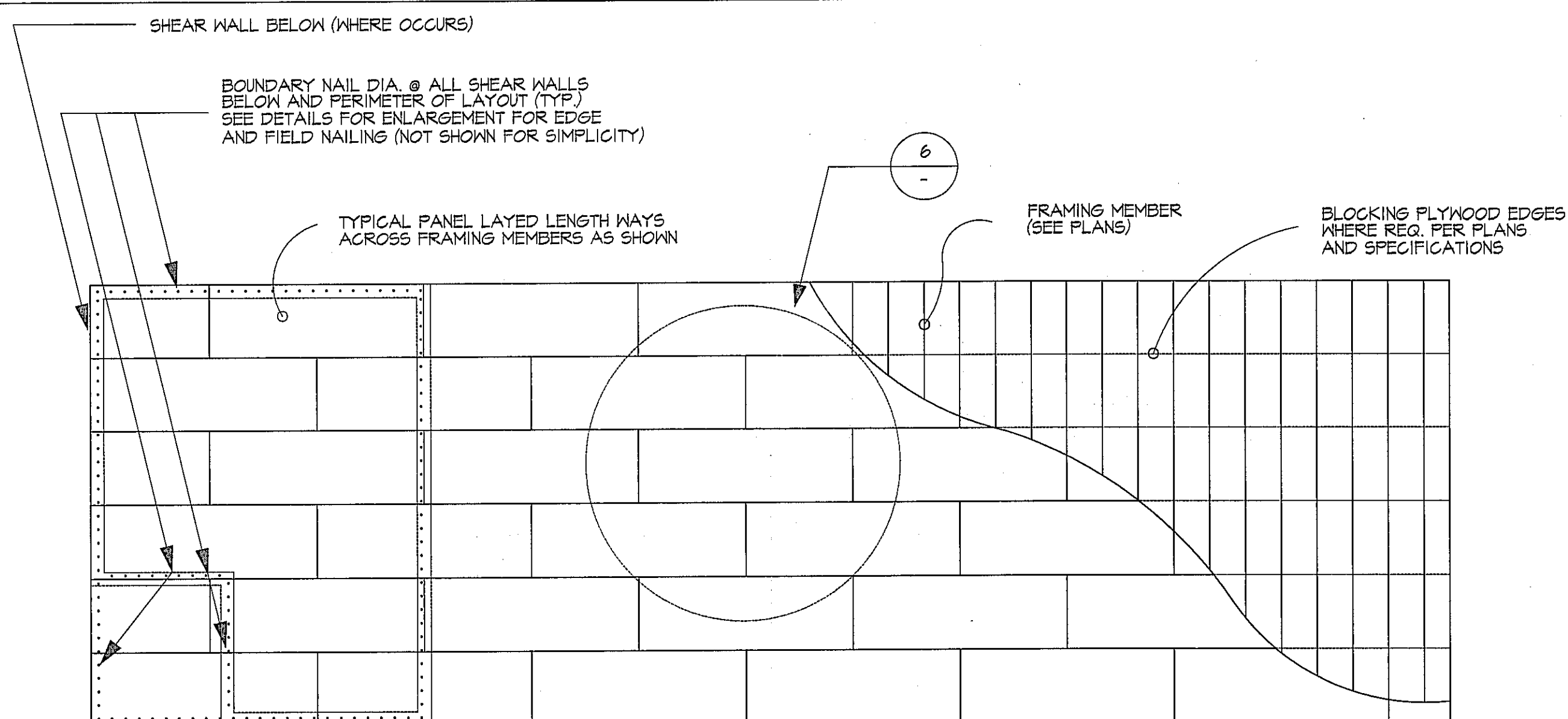
TWO STORY SINGLE FAMILY RESIDENCE
901 LAUREL CYN.
LA, CA 90046 FOR: L.I. INVESTMENTS, LLC



Date 12-18-12
Scale
Drawn hrd
Job
Sheet
6-3.3

FLOOR JOIST (RESIDENTIAL ONLY), ROOF JOIST AND ROOF RAFTERS - NOTCHING AND BORING TYPICAL 4

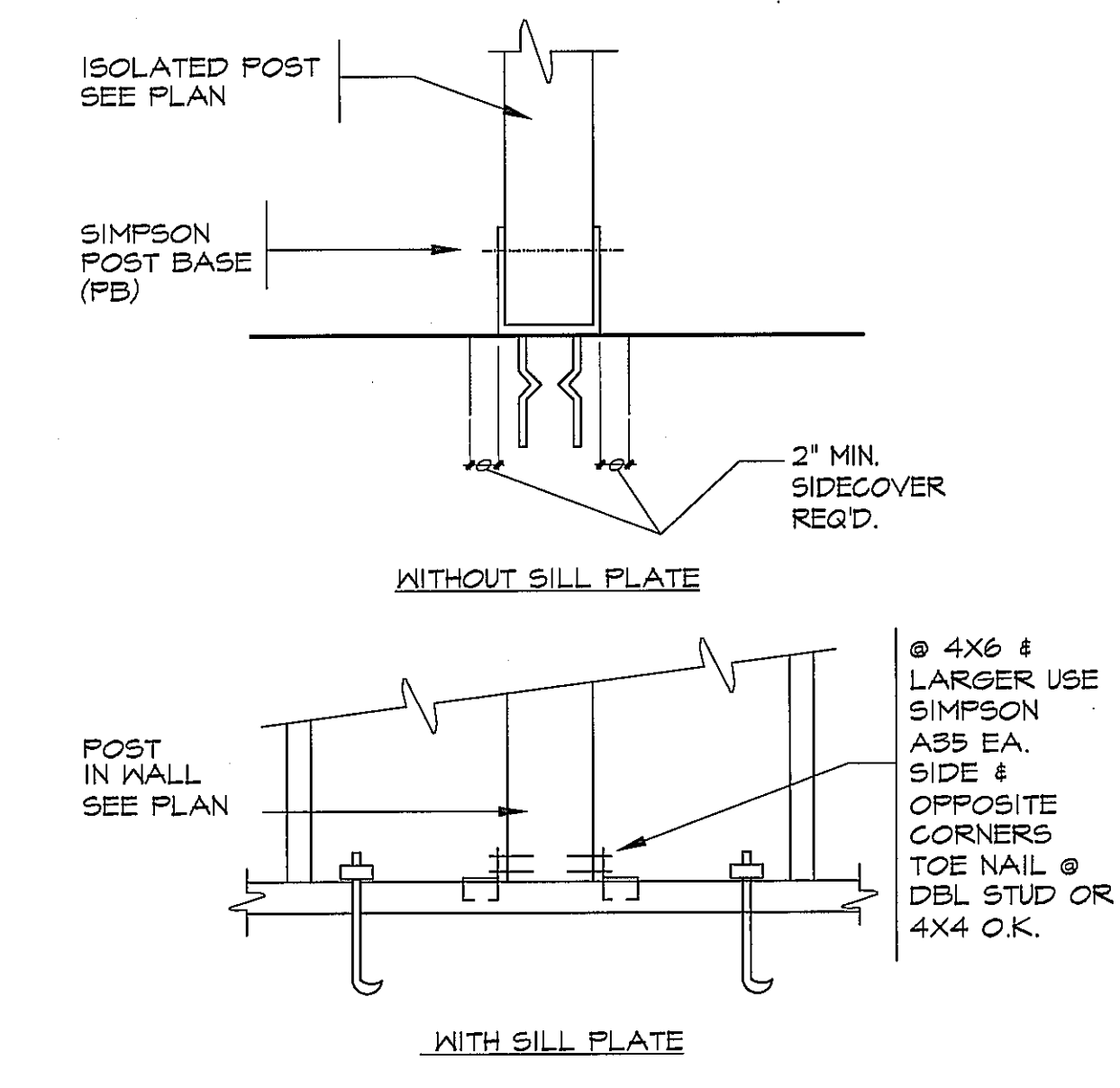
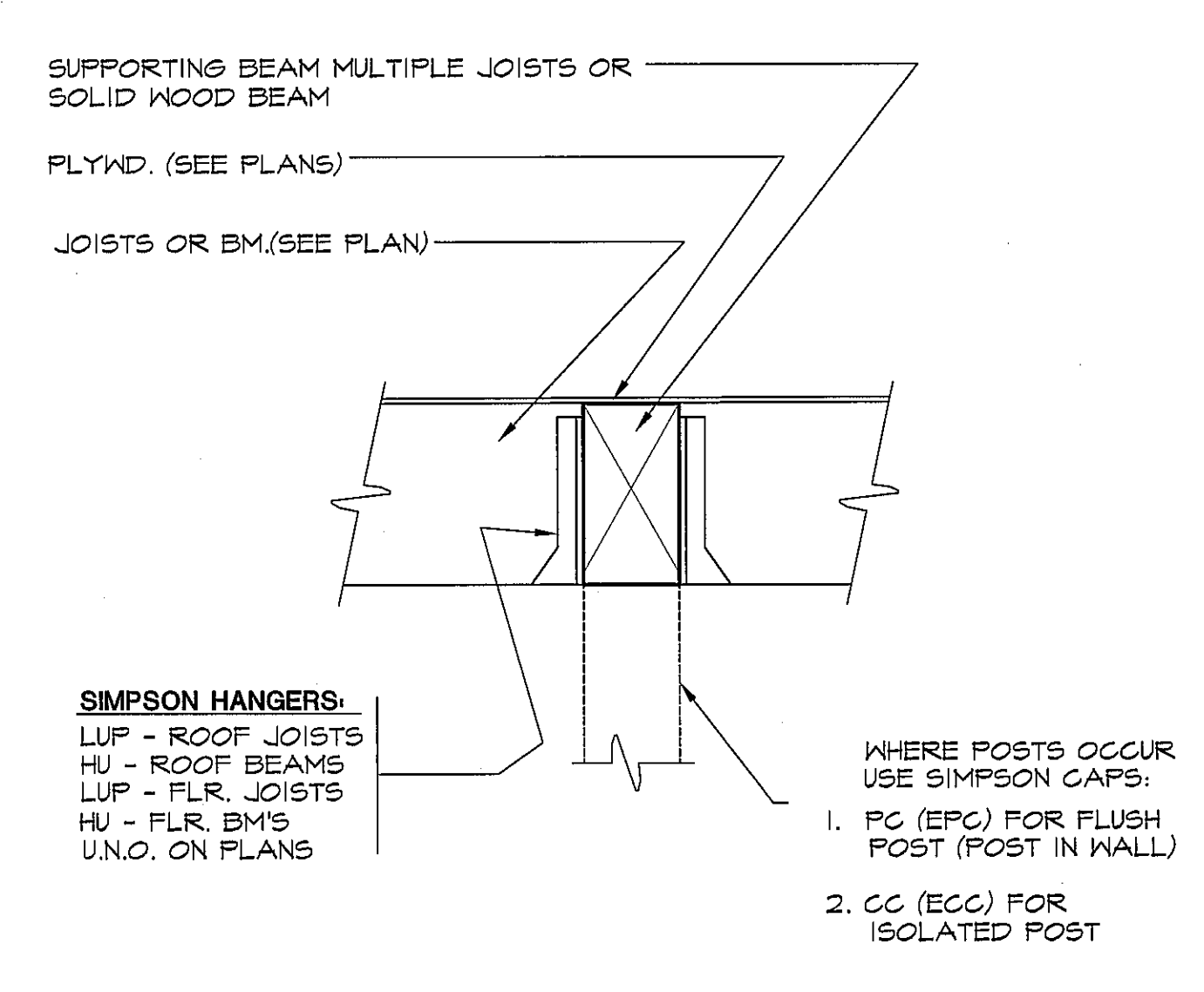
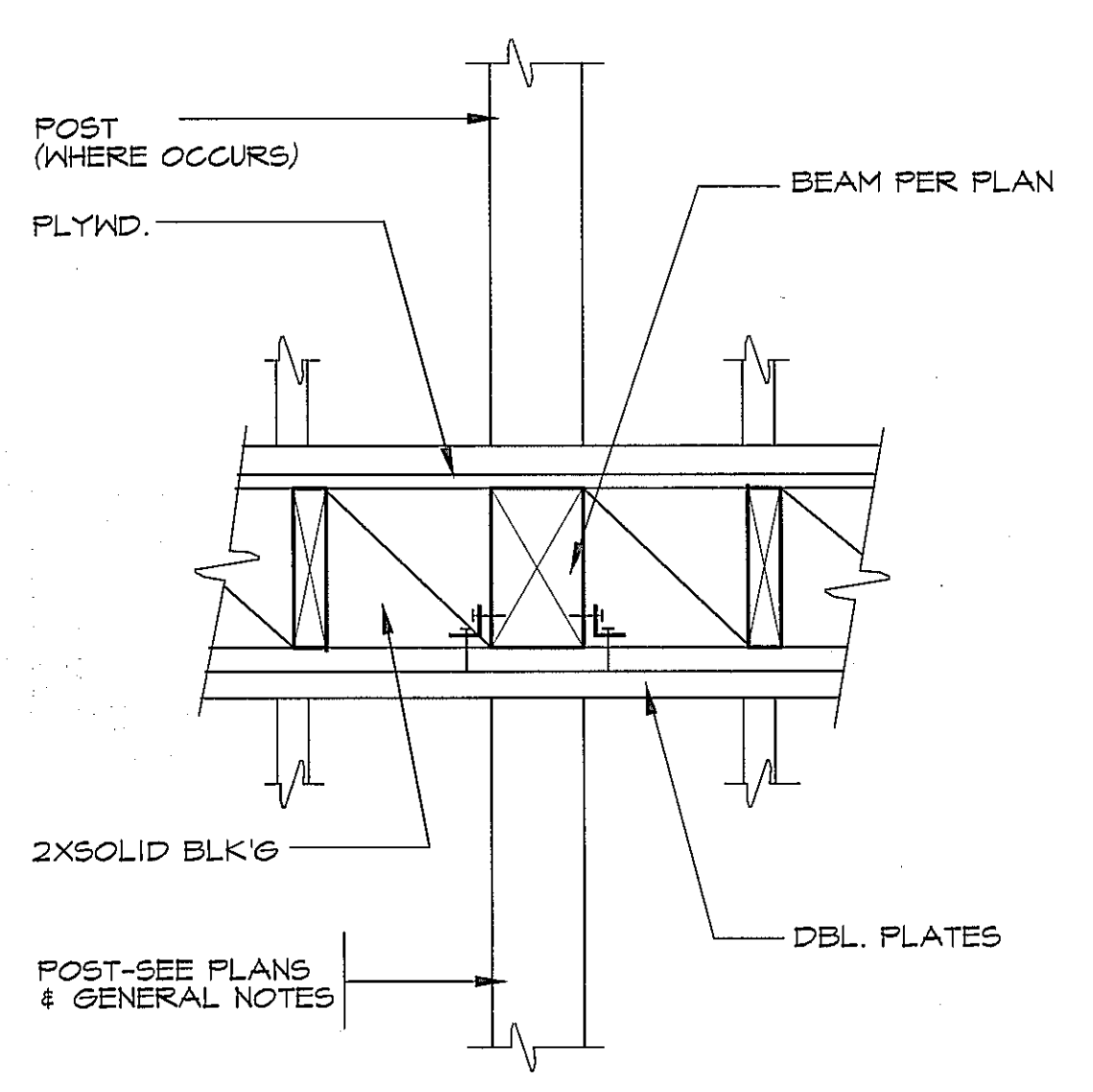
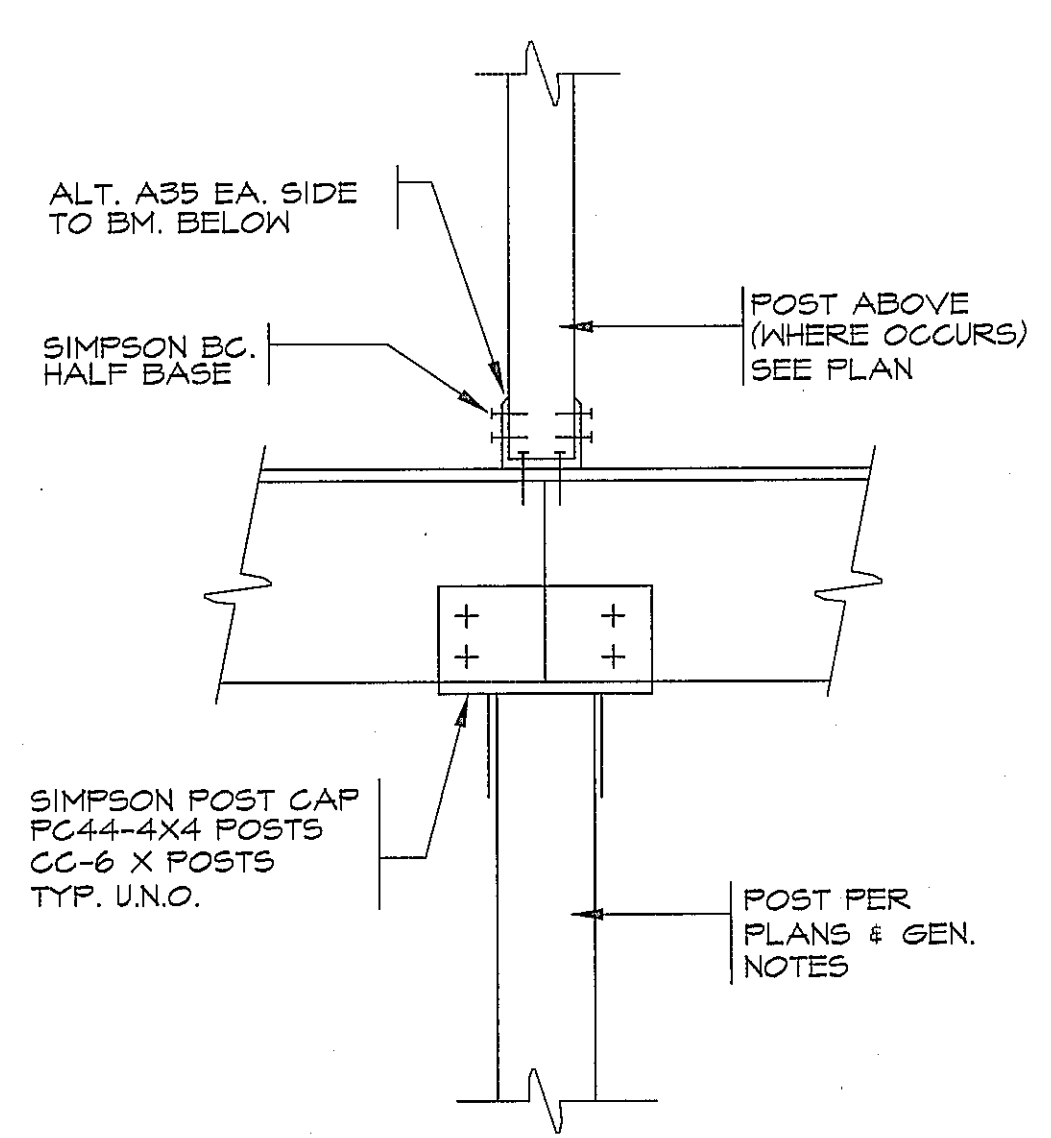
TYP. CUTTING & BORING STUDS 1



HORIZONTAL PLYWOOD SUBFLOOR/ROOF DIAPHRAGM LAYOUT PLAN (TYP.) 8

HORZ. PLYWD. LAYOUT NAILING PATTERN (TYP.) 5

TYP. WALL CORNER/INTERSECTION 2

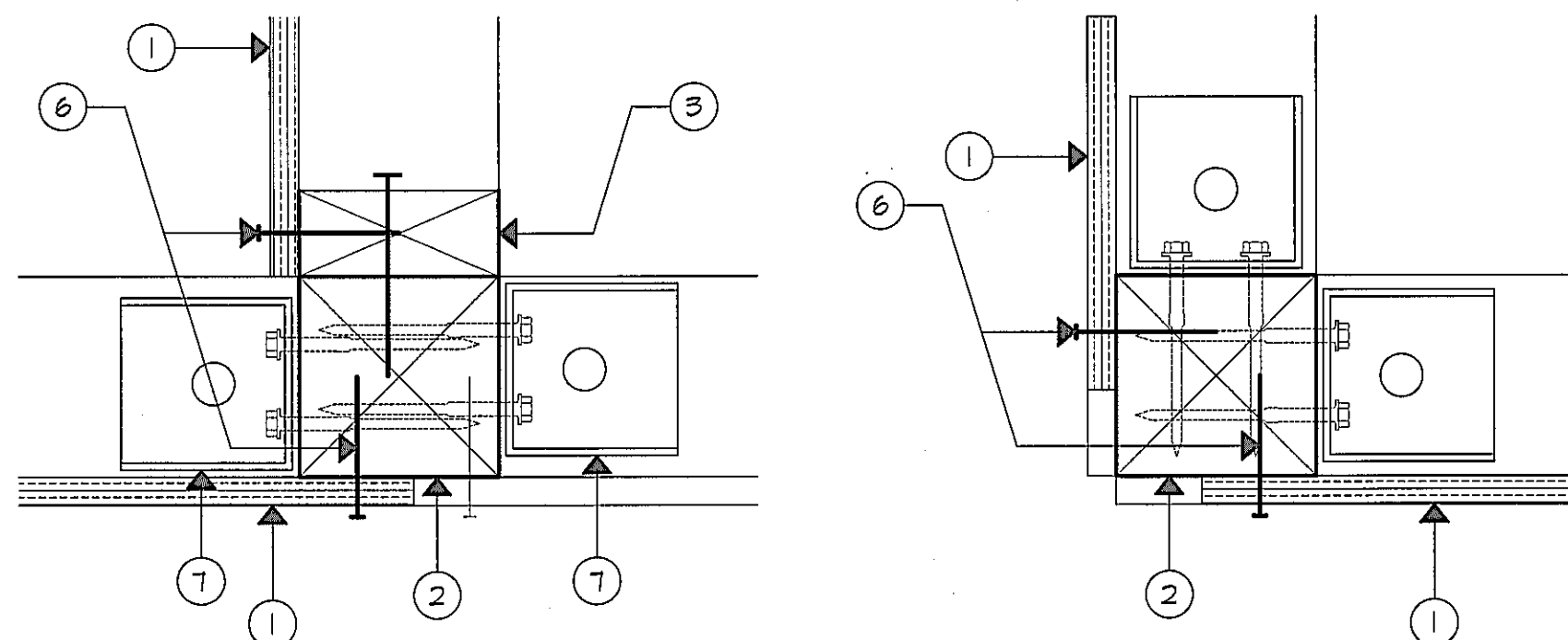
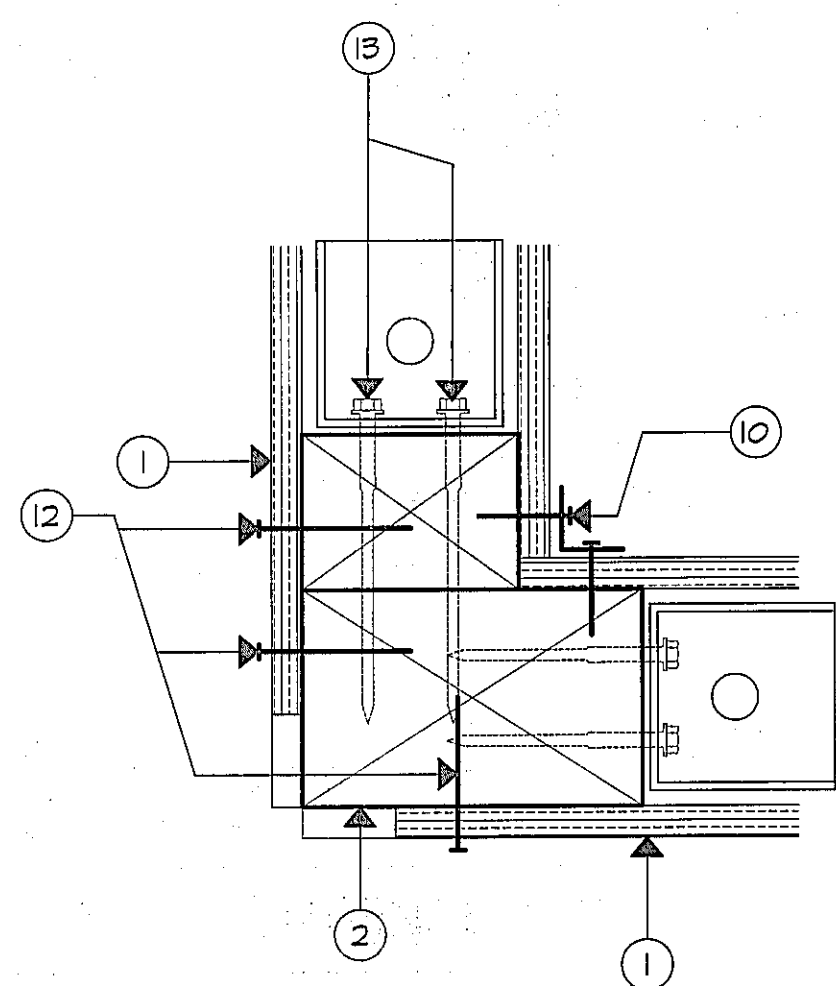
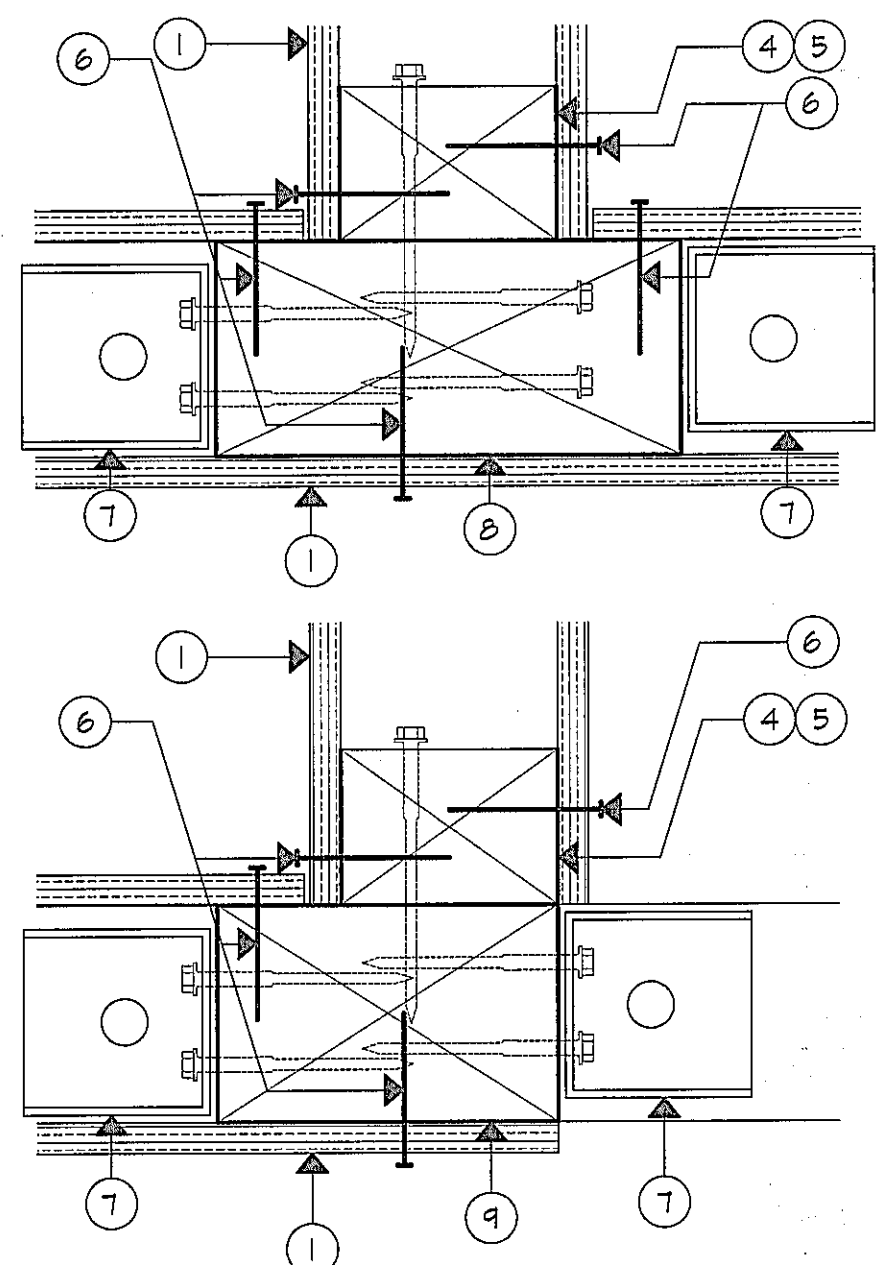


TYP. BEAM TO ISOLATED POST 12

TYP. BEAM TO POST IN WALL 9

FLUSH BEAM/JOIST DTL. TYP. 6

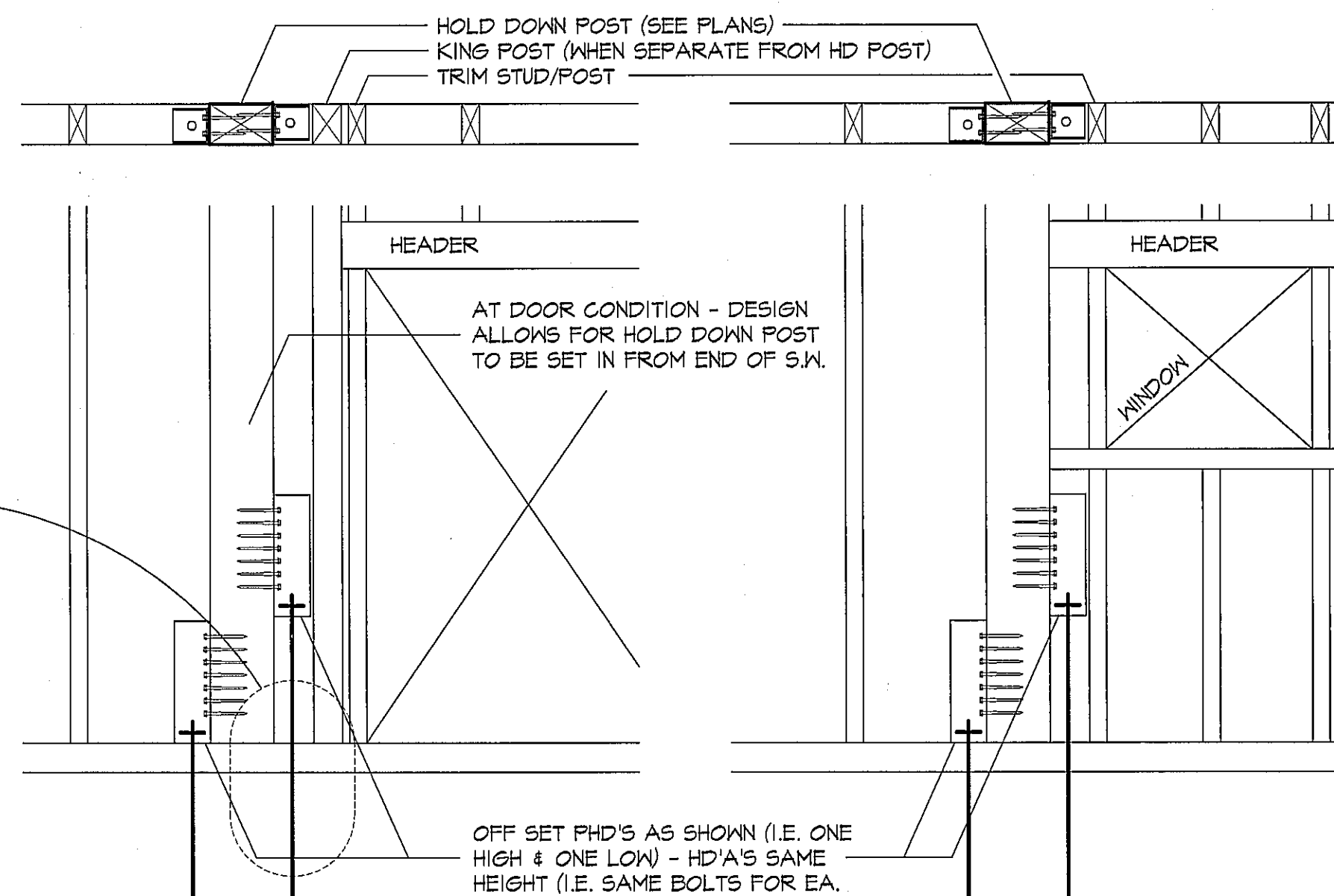
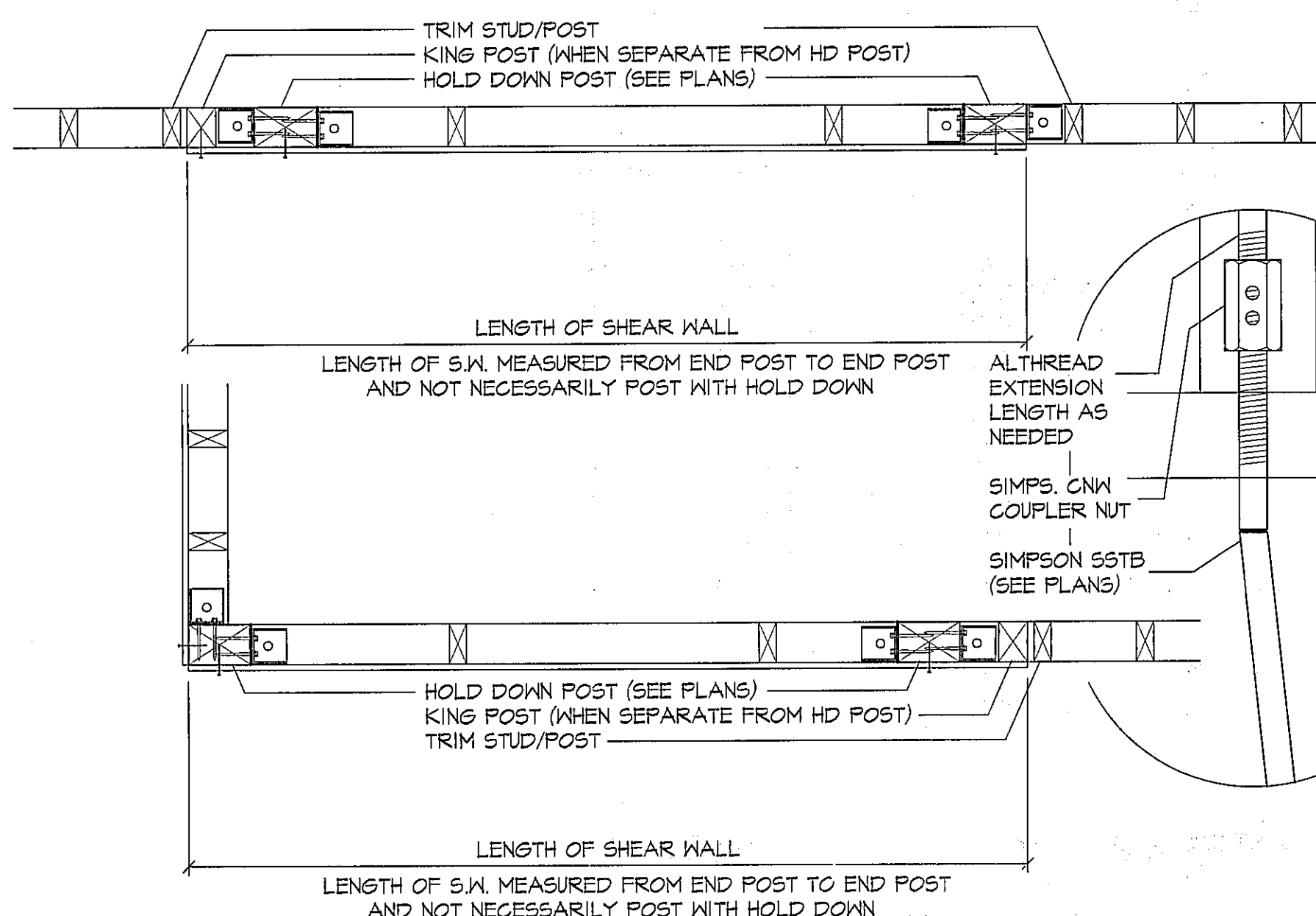
TYP. POST BASE W/ & W/O SILL 3



C DOUBLE PLYWD. TO DBL. PLYWD. DBL. PHD

B SINGLE PLYWD. TO SNGL. PLYWD. DBL.. PHD

A SINGLE PLYWD. TO SNGL. PLYWD. SNGL. PHD (OR NO PHD)



I. CALL OUT NOTES FOR DETAILS THIS SHEET

- SYMBOL: DESCRIPTION:
- ① PLYWOOD (SEE PLANS & S-3.1) - 4 PLY OR OSB ALLOWED - GRADE STAMP REQUIRED.
 - ② HOLD DOWN POST 4X4, 4X6, 6X6, 4X8, ETC. (SEE PLANS - DETAILS)
 - ③ CROSS WALL 2X W/ S.T. NAILING (SCHEDULE X)
 - ④ CROSS WALL 3X W/ S.T. SCREWS (SEE SCHEDULE Y)
 - ⑤ CROSS WALL 4X W/ S.T. SCREWS (SEE SCHEDULE Z)
 - ⑥ 8d COMMON (131" DIA.) OR 10d COMMON (148" DIA.) PLYWOOD NAILS SPACED AS E.N. (EDGE NAIL) AT ENDS & HD POSTS, PER PLANS AND SPECIFICATIONS (SEE FRAMING PLANS & S-3.1)
 - ⑦ HOLD DOWN, PHD OR HD (SEE PLANS & SPECIFICATIONS)
 - ⑧ 4X6 @ 4X CROSS WALL, 4X8 @ 6X CROSS WALL (4X WALL), 6X6 @ 4X CROSS WALL, (4X WALL), ETC.
 - ⑨ 4X8 @ 4X CROSS WALL, 4X10 @ 6X CROSS WALL (4X WALL), 6X8 @ 4X CROSS WALL, (6X WALL), ETC.
 - ⑩ A35'S SPACED AT DBL. SPACING AS SHOWN IN SCH. S-3.1 FOR EA. SIDE, I.E. IF ONLY ONE SIDE AT DBL. SIDED S.W. (EX. IF "P" S.W. PER SCH. A35'S @ 6' O/C, THEREFORE AT ONE -OR EACH SIDE- TO BE 12' O/C, I.E. DBL. SPACING OR 1/2 AS MANY PER SIDE)
 - ⑪ 3X SPACER IN RETURN WALL AT COMMON CORNER
 - ⑫ 10d COMMON (148" DIA.) PLYWOOD NAILS SPACED AS E.N. (EDGE NAIL) STAGGERED EACH FRAMING AT ENDS & HD POSTS, PER PLANS AND SPECIFICATIONS (SEE FRAMING PLANS & S-3.1)
 - ⑬ INCREASE SCREWS FROM 3" TO 4 1/2" AT RETURN STUD POST AT COMMON CORNER OR SIMILAR

M DBL. HOLD DOWN LAYOUT AT DOOR(S)/WINDOW(S)

II. SUBSTITUTIONS ALLOWED

- ITEM DESCRIPTION:
1. SDS1/4 SCREWS FOR 16d COMMON. NAILS
 2. AT SINGLE HOLD DOWN (I.E. PHD, HD'A, ETC.) O.K. TO LOCATE ON OPPOSITE SIDE OF POST (I.E. SIDE NOT CRITICAL).
 3. WHERE HOLD DOWN IS SHOWN TWICE AT A CORNER OR INTERSECTION THE SMALLER CAN BE DELETED PROVIDED THE CROSS (INTERSECTING) WALLS TIE BOTH TOGETHER FOR ALL PANELS (I.E. S.T. ALL TO POST W/ HOLD DOWN). NOTE: S.T. CAN BE ACHIEVED IN MANY DIFFERENT WAYS - THESE DETAILS ARE PROVIDED TO SHOW MOST COMMON, BUT NOT ALWAYS THE ONLY METHOD POSSIBLE. ALTERNATE METHODS ARE POSSIBLE - STRUCTURAL OBSERVATION TO ADDRESS ALL ALTERNATES NOT SHOWN HERE PROVIDED S.T. (I.E. LOAD PATH) IS ACHIEVED WITHIN CODE AND SPECIFIC DESIGN SHOWN HERE, I.E. IN THESE PLANS).

SCHEDULE Z 4X SIDE MEMBER (4X SIDE MEMBER TO 4X OR LARGER S.T. POST, STUD OR BLKG.)			
SHEAR WALL:	ITEM:	SUBSTITUTION ITEM:	SUBSTITUTION ITEM:
	① 16d BOX NAILS	② 16d COMM. NAILS	③ SIMPSON SDS1/4'S CAN BE USED AS FOLLOWS:
			3" LONG 4 1/2" LONG 6" LONG
△ & △	4" O/C	5 1/2" O/C	④ N/A ⑤ N/A ⑥ 9" O/C
M	4" O/C	4 1/2" O/C	④ N/A ⑤ N/A ⑥ 7 1/2" O/C
N	3" O/C	3 1/2" O/C	④ N/A ⑤ N/A ⑥ 5" O/C
△	2" O/C	2 1/4" O/C	④ N/A ⑤ N/A ⑥ 4" O/C
P	1 1/2" O/C	1 3/4" O/C	④ N/A ⑤ N/A ⑥ 3" O/C
△	1" O/C	1 1/2" O/C	④ N/A ⑤ N/A ⑥ 3" O/C
R	1" O/C	1 1/2" O/C	④ N/A ⑤ N/A ⑥ 3" O/C

- ① .162" DIA. X 3 1/2" LONG - CONSULT ENGR. WHEN USING SMALLER OR SHORTER NAILS
- ② .135" DIA. X 3 1/2" LONG - CONSULT ENGR. WHEN USING SMALLER OR SHORTER NAILS
- ③ .242" DIA. X LENGTH SHOWN - CONSULT ENGR. WHEN USING SMALLER OR SHORTER SCREWS OR OTHER THAN SDS1/4
- ④ N/A = NOT ALLOWED

SCHEDULE Y 3X SIDE MEMBER (3X SIDE MEMBER TO 3X OR LARGER S.T. POST, STUD OR BLKG.)			
SHEAR WALL:	ITEM:	SUBSTITUTION ITEM:	SUBSTITUTION ITEM:
	① 16d BOX NAILS	② 16d COMM. NAILS	③ SIMPSON SDS1/4'S CAN BE USED AS FOLLOWS:
			3" LONG 4 1/2" LONG 6" LONG
△ & △	4" O/C	5 1/2" O/C	④ N/A ⑤ 9" O/C ⑥ 11" O/C
M	4" O/C	4 1/2" O/C	④ N/A ⑤ 7 1/2" O/C ⑥ 9" O/C
N	3" O/C	3 1/2" O/C	④ N/A ⑤ 5" O/C ⑥ 6" O/C
△	2" O/C	2 1/4" O/C	④ N/A ⑤ 4" O/C ⑥ 5" O/C
P	1 1/2" O/C	1 3/4" O/C	④ N/A ⑤ 3" O/C ⑥ 3 1/2" O/C
△	1" O/C	1 1/2" O/C	④ N/A ⑤ 3" O/C ⑥ 3 1/2" O/C
R	1" O/C	1 1/2" O/C	④ N/A ⑤ 3" O/C ⑥ 3 1/2" O/C

- ① .162" DIA. X 3 1/2" LONG - CONSULT ENGR. WHEN USING SMALLER OR SHORTER NAILS
- ② .135" DIA. X 3 1/2" LONG - CONSULT ENGR. WHEN USING SMALLER OR SHORTER NAILS
- ③ .242" DIA. X LENGTH SHOWN - CONSULT ENGR. WHEN USING SMALLER OR SHORTER SCREWS OR OTHER THAN SDS1/4
- ④ N/A = NOT ALLOWED

SCHEDULE X 2X SIDE MEMBER (2X SIDE MEMBER TO 2X OR LARGER S.T. POST, STUD OR BLKG.)			
SHEAR WALL:	ITEM:	SUBSTITUTION ITEM:	SUBSTITUTION ITEM:
	① 16d BOX NAILS	② 16d COMM. NAILS	③ SIMPSON SDS1/4'S CAN BE USED AS FOLLOWS:
			3" LONG 4 1/2" LONG 6" LONG
△ & △	4" O/C	5 1/2" O/C	④ 9" O/C ⑤ 11" O/C ⑥ N/A
M	4" O/C	4 1/2" O/C	④ 7 1/2" O/C ⑤ 9" O/C ⑥ N/A
N	3" O/C	3 1/2" O/C	④ 5" O/C ⑤ 6" O/C ⑥ N/A
△	2" O/C	2 1/4" O/C	④ 4" O/C ⑤ 5" O/C ⑥ N/A
P	1 1/2" O/C	1 3/4" O/C	④ 3" O/C ⑤ 3 1/2" O/C ⑥ N/A
△	1" O/C	1 1/2" O/C	④ 3" O/C ⑤ 3 1/2" O/C ⑥ N/A
R	1" O/C	1 1/2" O/C	④ 3" O/C ⑤ 3 1/2" O/C ⑥ N/A

- ① .162" DIA. X 3 1/2" LONG - CONSULT ENGR. WHEN USING SMALLER OR SHORTER NAILS
- ② .135" DIA. X 3 1/2" LONG - CONSULT ENGR. WHEN USING SMALLER OR SHORTER NAILS
- ③ .242" DIA. X LENGTH SHOWN - CONSULT ENGR. WHEN USING SMALLER OR SHORTER SCREWS OR OTHER THAN SDS1/4
- ④ N/A = NOT ALLOWED

Z NAIL OPTIONS SCHEDULE 4X SIDE MEMBER

Y NAIL OPTIONS SCHEDULE 3X SIDE MEMBER

X NAIL OPTIONS SCHEDULE 2X SIDE MEMBER

N SPECIFICATIONS & GENERAL NOTES:

REVISIONS BY

HRD ENGINEERING
STRUCTURAL DESIGN AND DRAFTING
7463 VARNA AVE., SECOND FLOOR
N. HOLLYWOOD, CA 91605
PHONE NO. (805) 431-5415 - FAX NO. (805) 888-4840

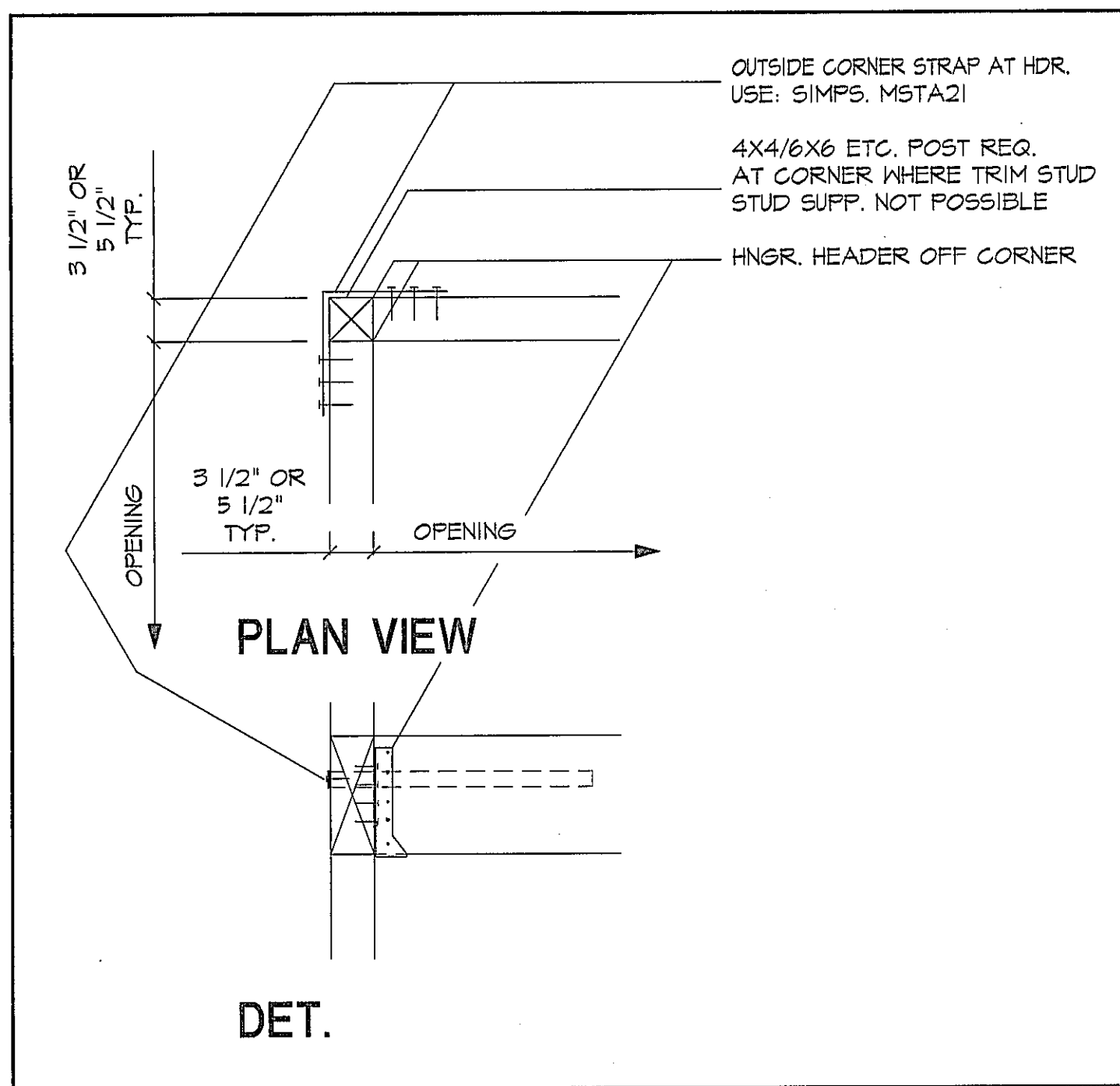
FRAMING DETAILS

TWO STORY SINGLE FAMILY RESIDENCE
901 LAUREL CYN.
LA, CA 90046 FOR: L.I. INVESTMENTS, LLC

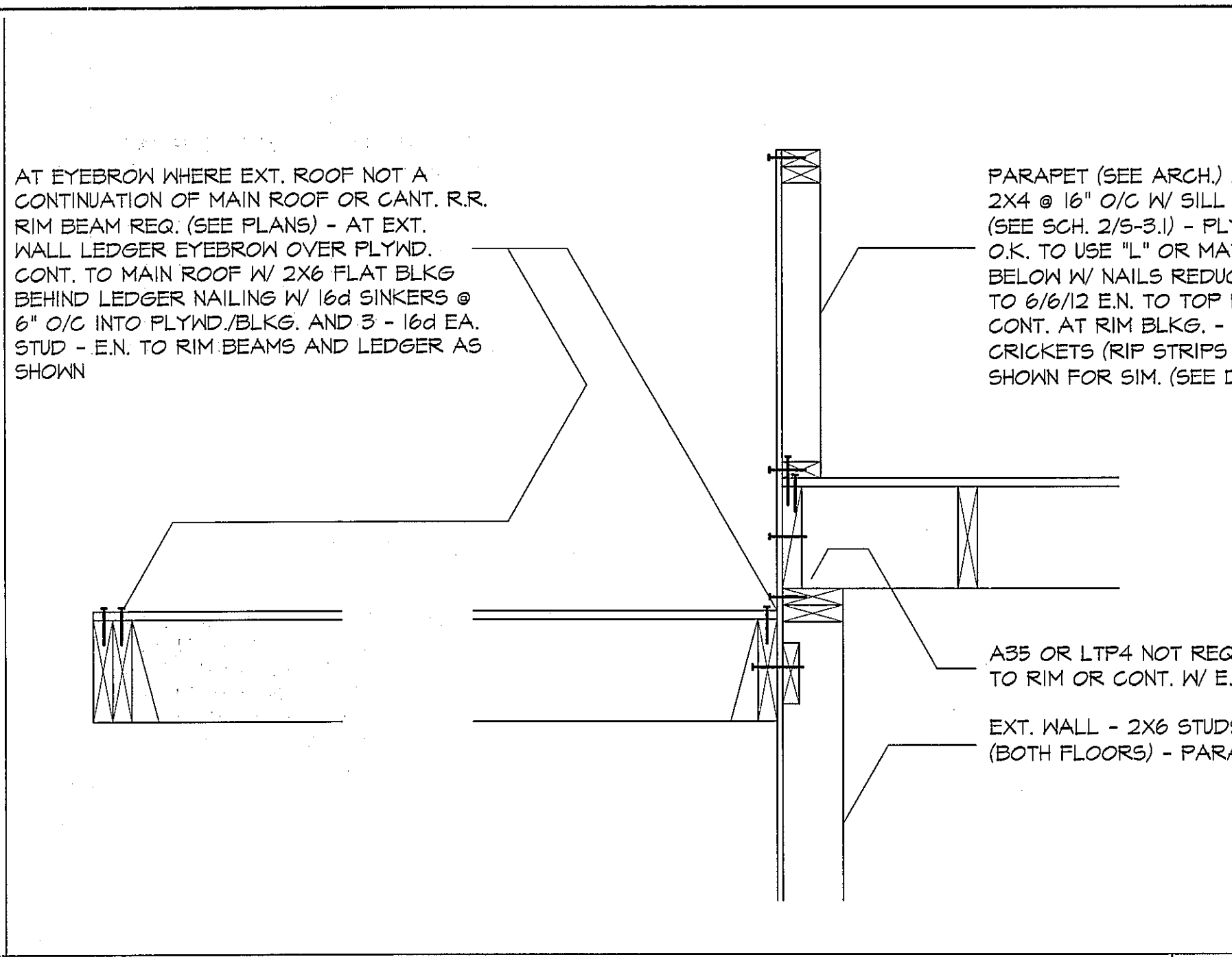
REGISTERED PROFESSIONAL ENGINEER
No. SE2628
STATE OF CALIFORNIA

Date 11-27-12
Scale
Drawn hrd
Job
Sheet
S-3.4
of

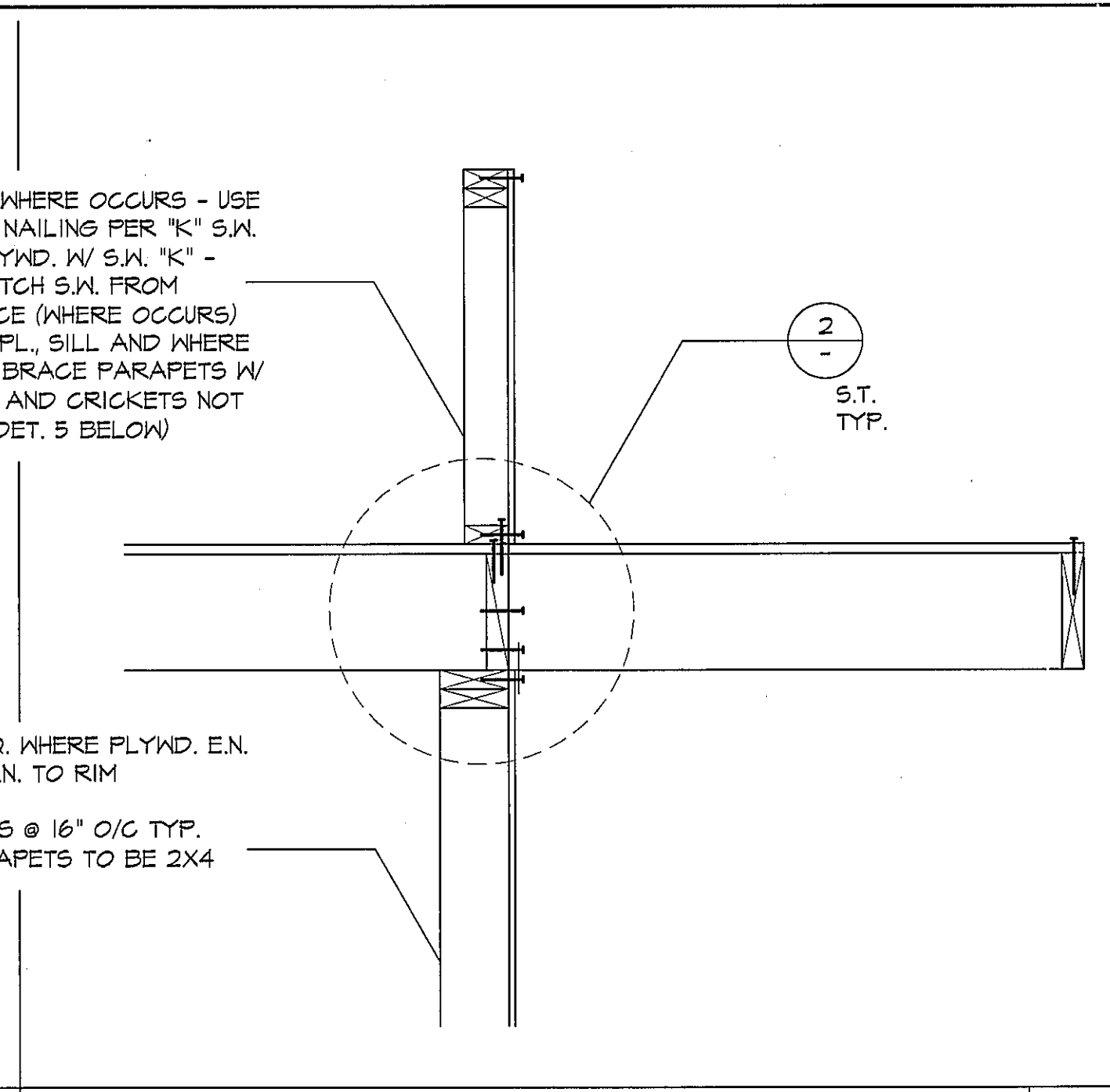
THESE FRAMING DETAILS AND SPECIFICATIONS ARE THE SOLE PROPERTY OF HRD ENGINEERING FOR THE EXCLUSIVE USE OF ITS CLIENTS FOR THIS SPECIFIC PROJECT WITH APPROVAL BY THE CLIENT. ANY REUSE, REPRODUCTION, TRANSFER OF USE OR PERMISSION WITHOUT WRITTEN APPROVAL IS PROHIBITED.
COPYRIGHT © HRD ENGINEERING



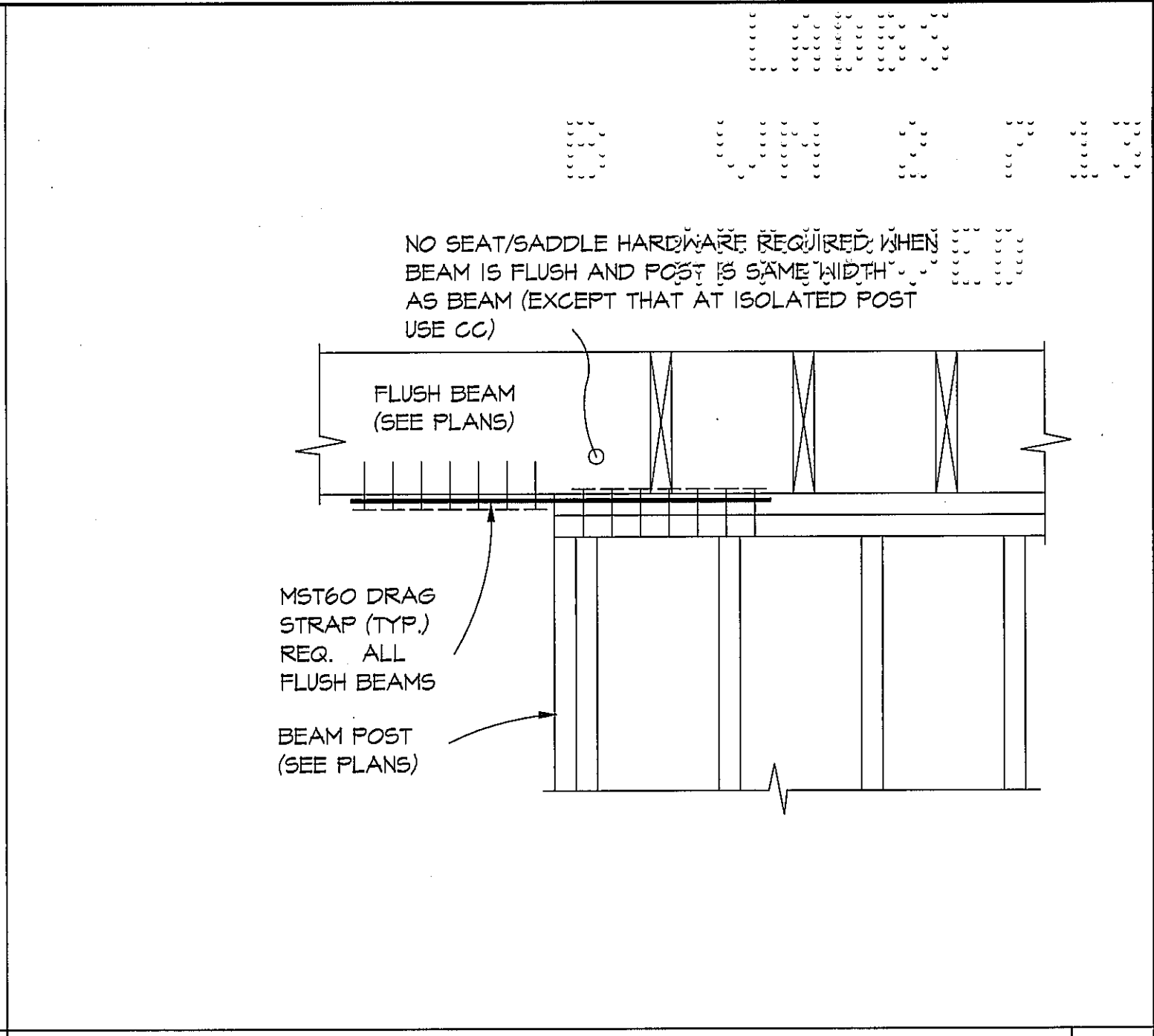
WIND. HDR. SUPPORT @ CORNER POST CONDITION 10



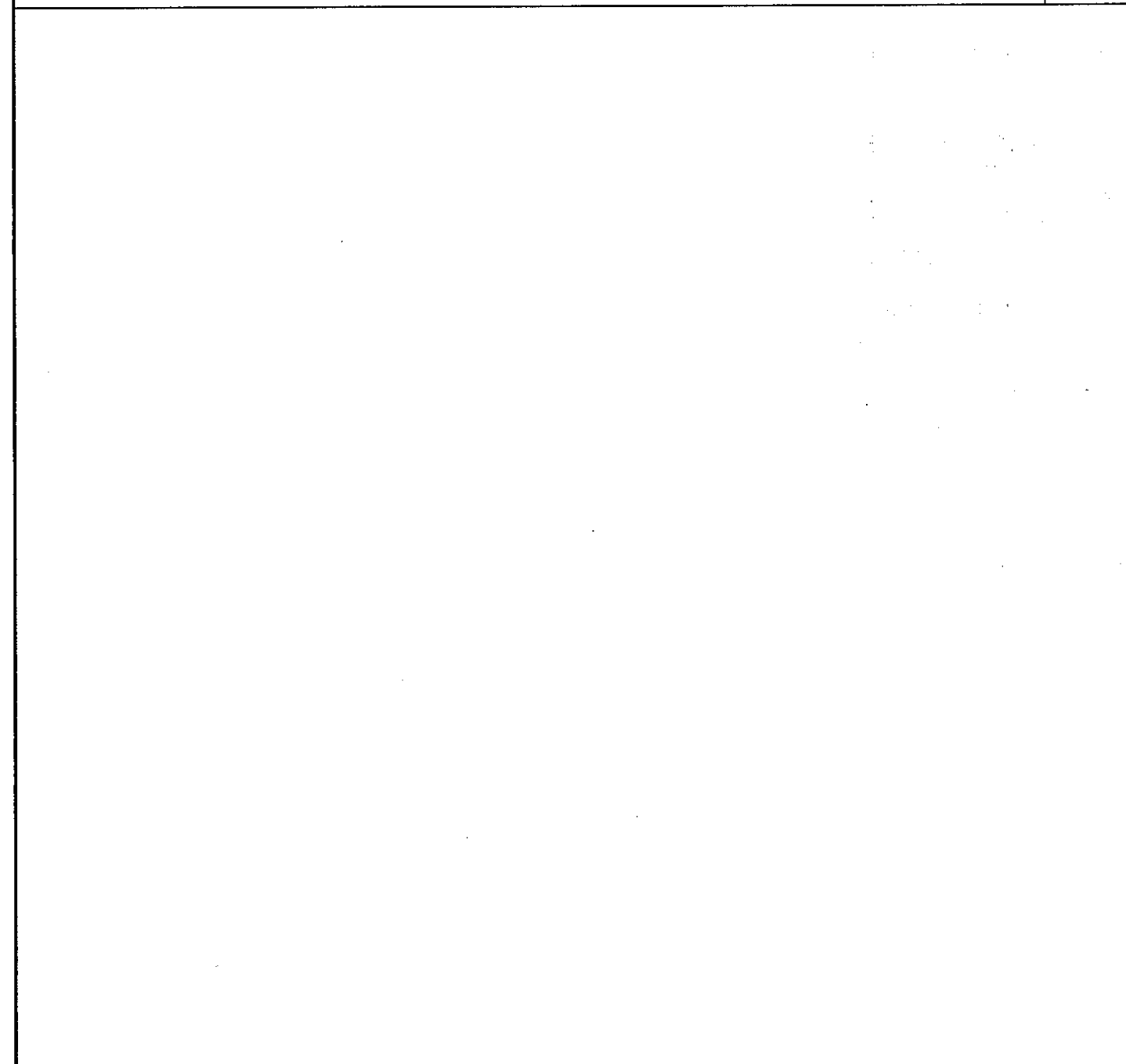
S.T. AT EYEBROW ROOF (TYP.) 7



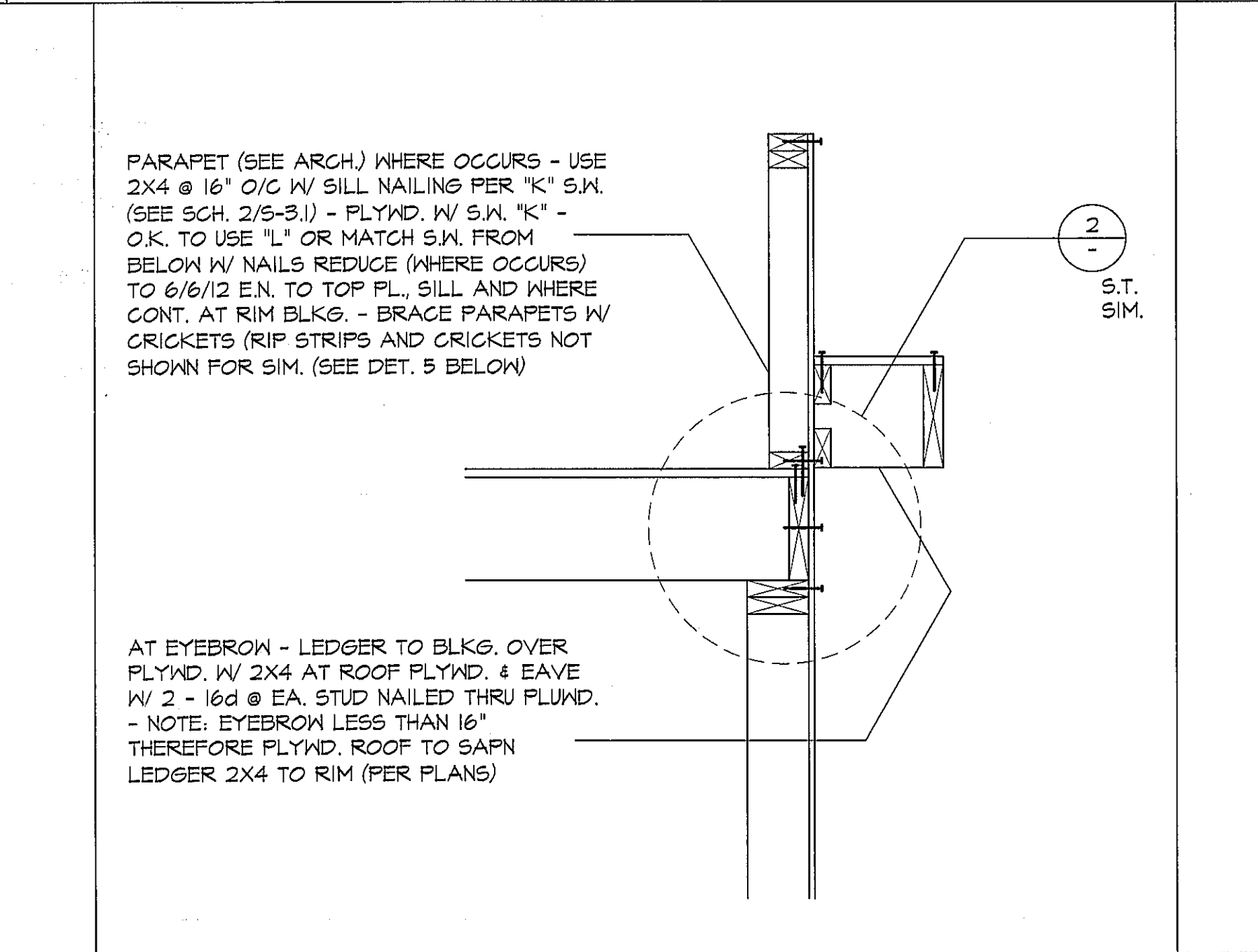
S.T. AT CANT. ROOF (TYP.) 4



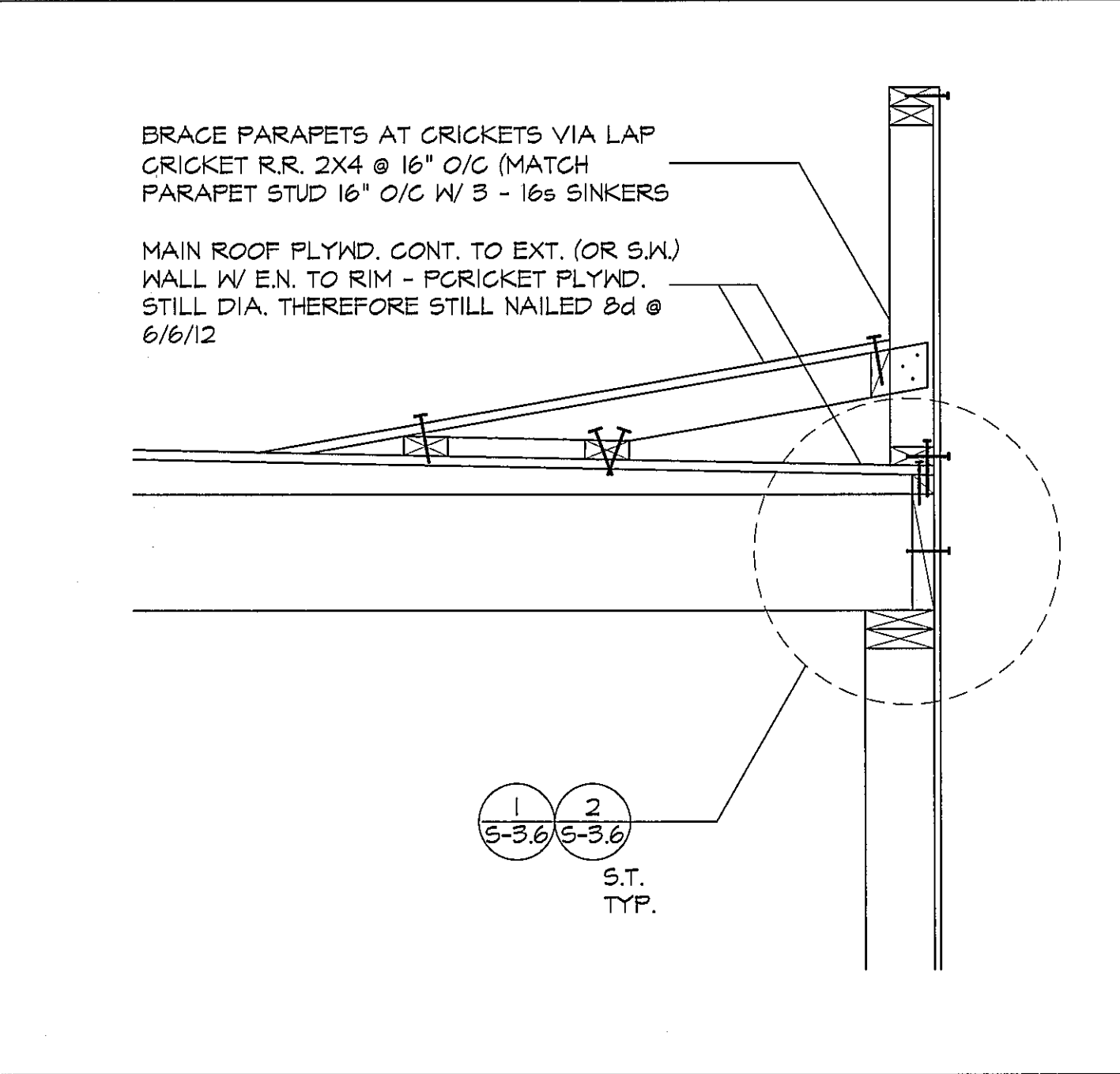
DRAG STRAP CONN. DETAIL 1



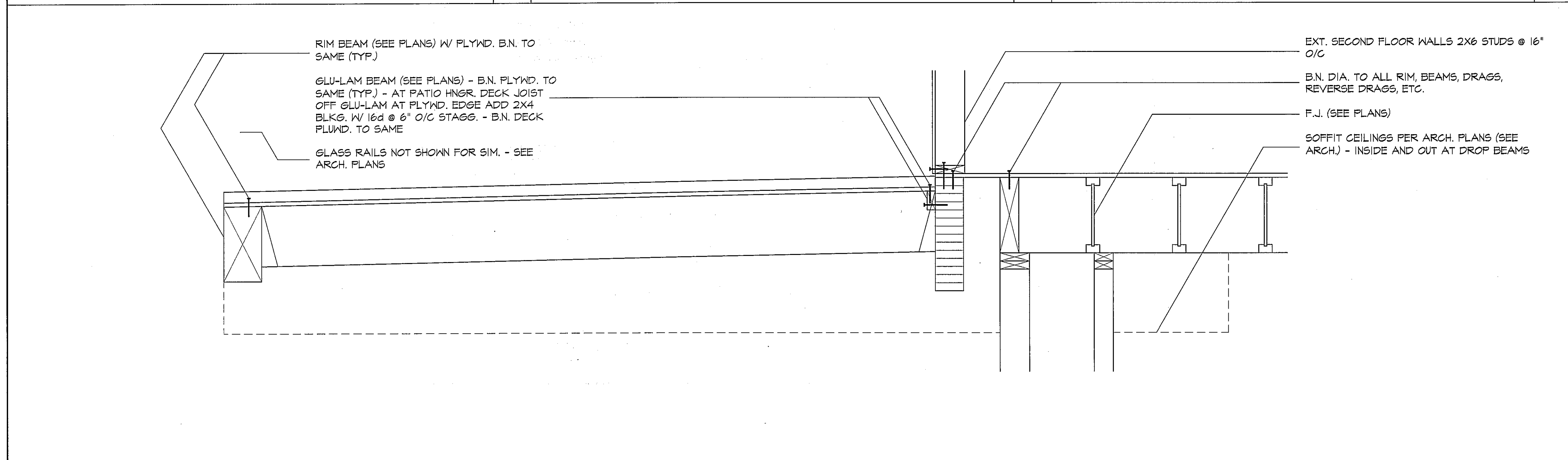
11 S.T. AT CANT. ROOF (TYP.) 8



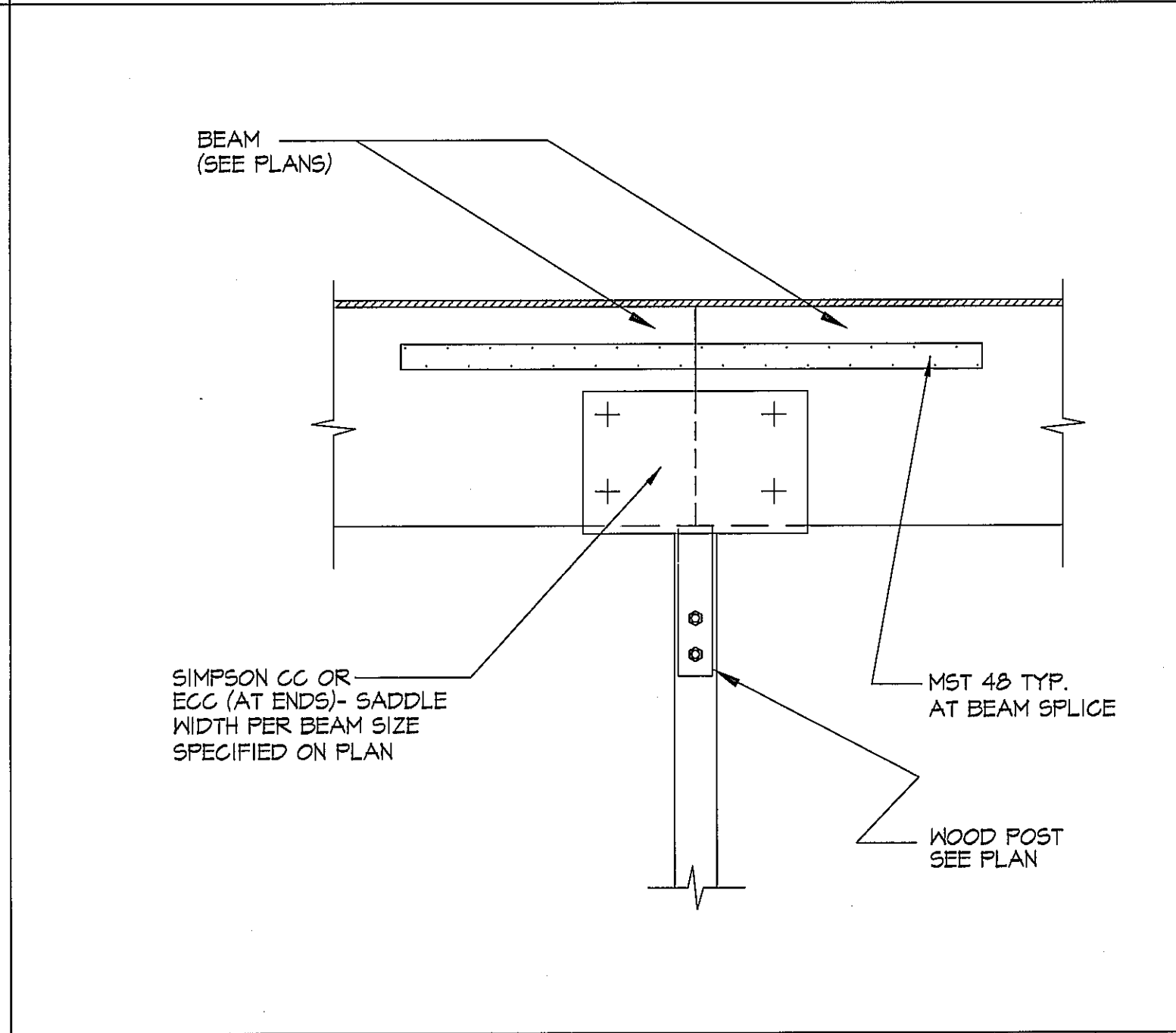
CRICKET AT FLAT ROOF (TYP.) 5



S.T. (SHEAR TRANSFER) AT ROOF (TYP.) 2



FRAMING SECTION AT PATIO 6



TYP. RIDGE BEAM TO WOOD POST 3

REVISIONS	BY

THESE PLANS, DRAWINGS, DESIGNS AND SPECIFICATIONS ARE THE SOLE PROPERTY OF HRD ENGINEERING AND DRAFTING. NO PART OF THESE PLANS OR SPECIFICATIONS MAY BE REPRODUCED OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, ELECTRONIC OR MECHANICAL, WITHOUT WRITTEN APPROVAL. COPYRIGHT © HRD ENGINEERING.

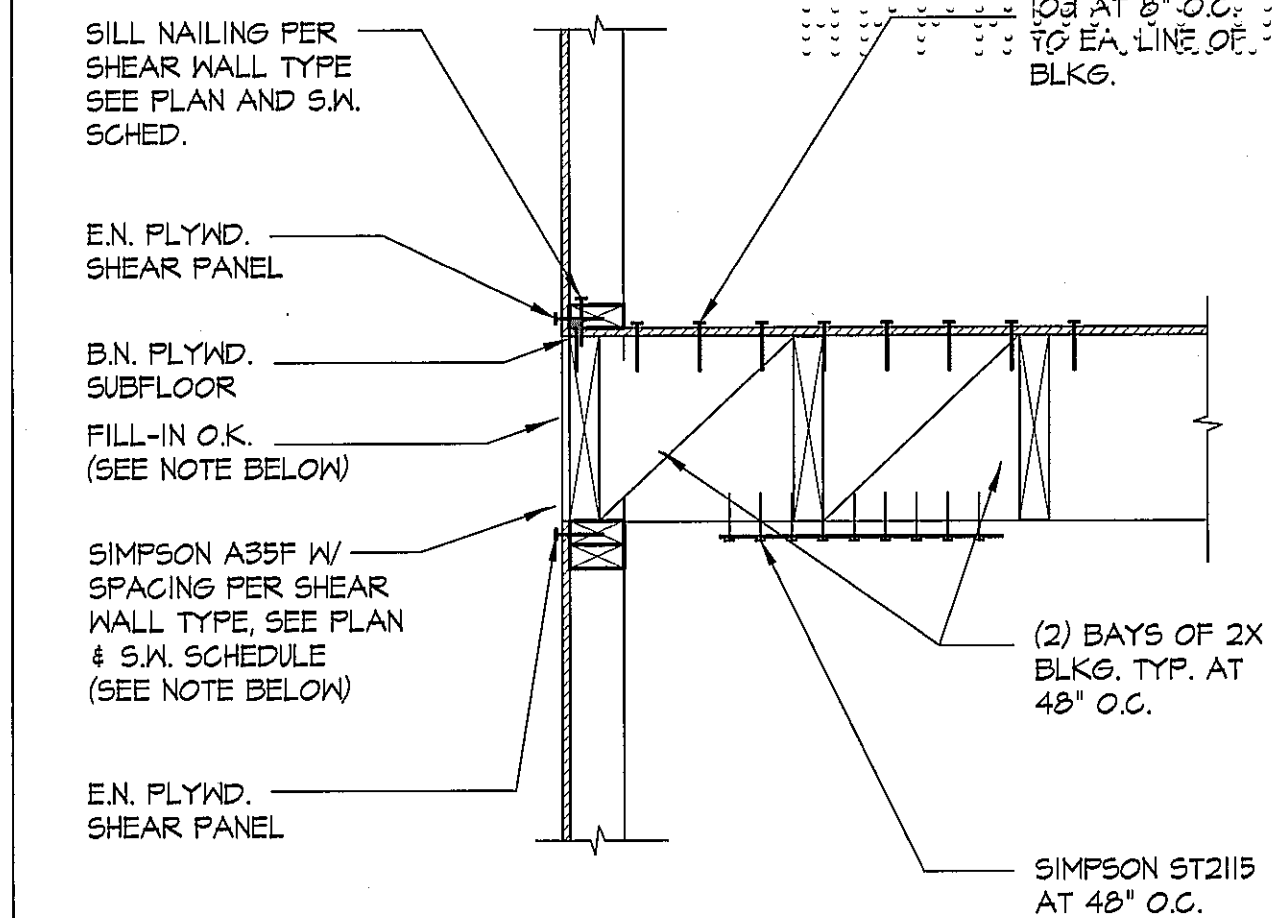
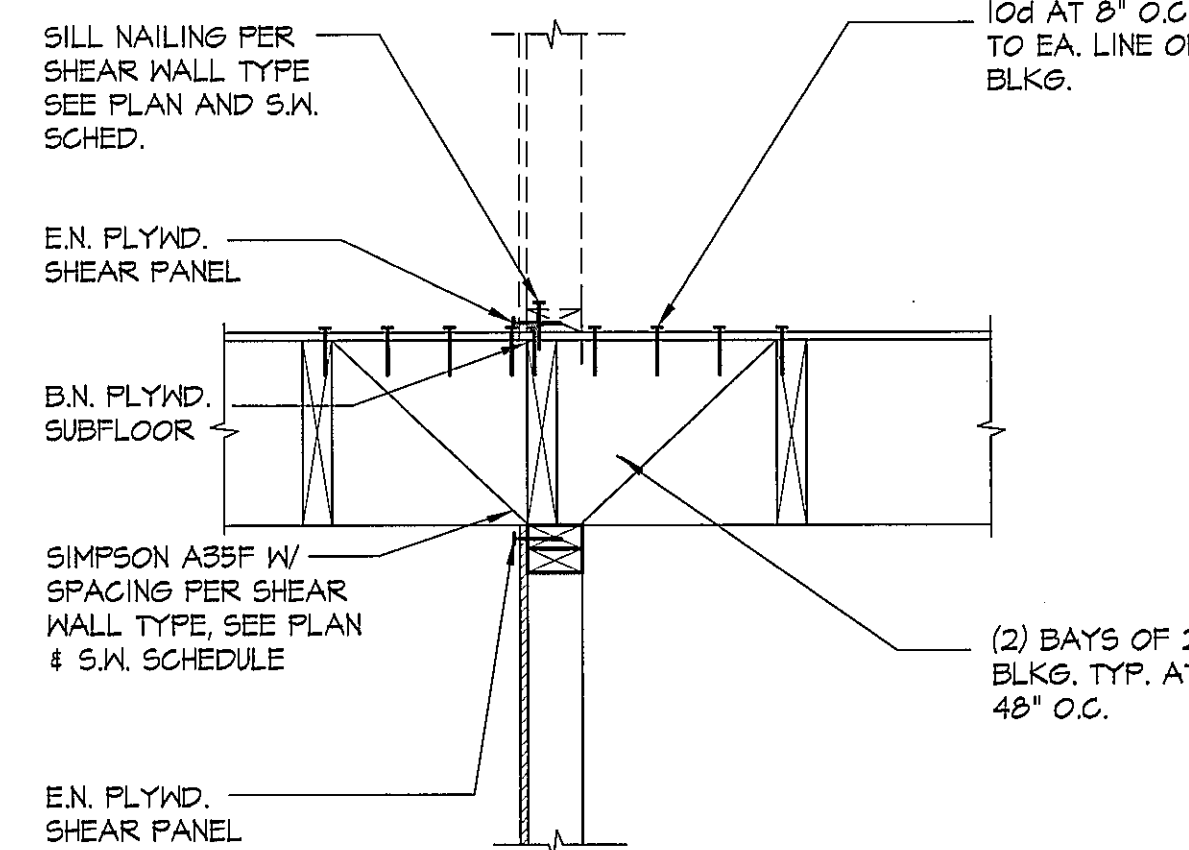
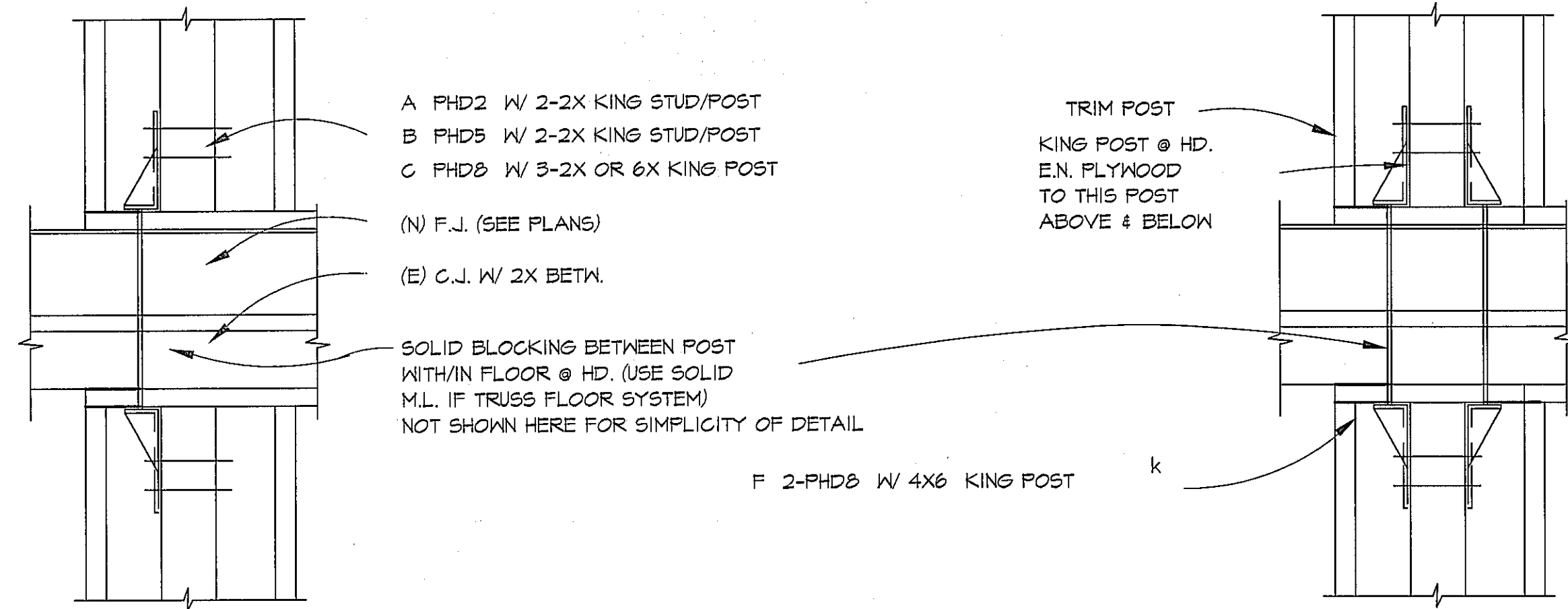
HRD ENGINEERING
STRUCTURAL DESIGN AND DRAFTING
7463 VARNA AVE., SECOND FLOOR
N. HOLLYWOOD, CA 91605
PHONE NO. (805) 431-5415 - FAX NO. (805) 888-4840

FRAMING DETAILS

TWO STORY SINGLE FAMILY RESIDENCE
901 LAUREL CYN.
LA, CA 90046 FOR: L.I. INVESTMENTS, LLC

PROFESSIONAL ENGINEER
No. SE2628
STATE OF CALIFORNIA

Date 11-27-12
Scale
Drawn hrd
Job
Sheet
6-3.5
of

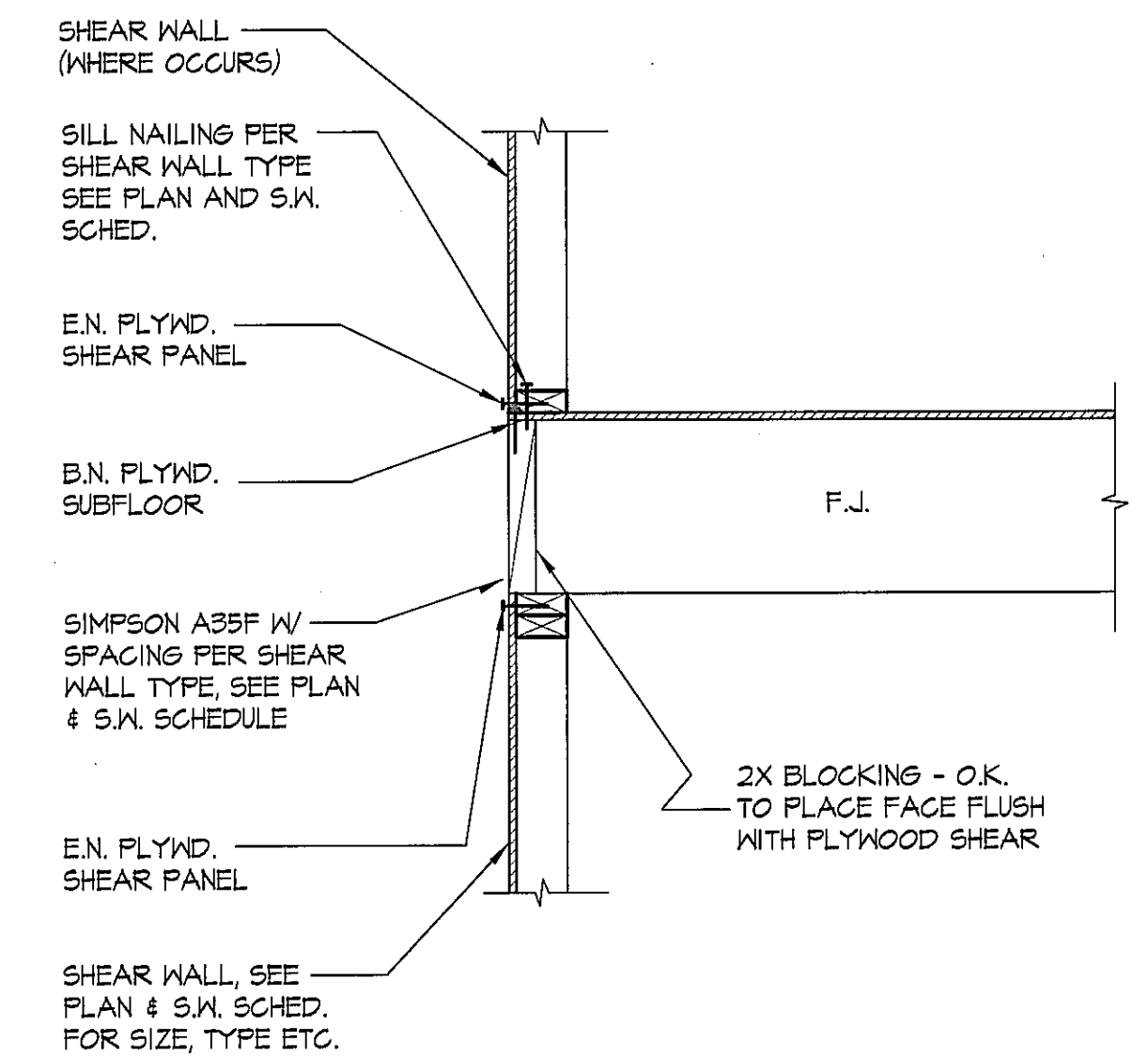
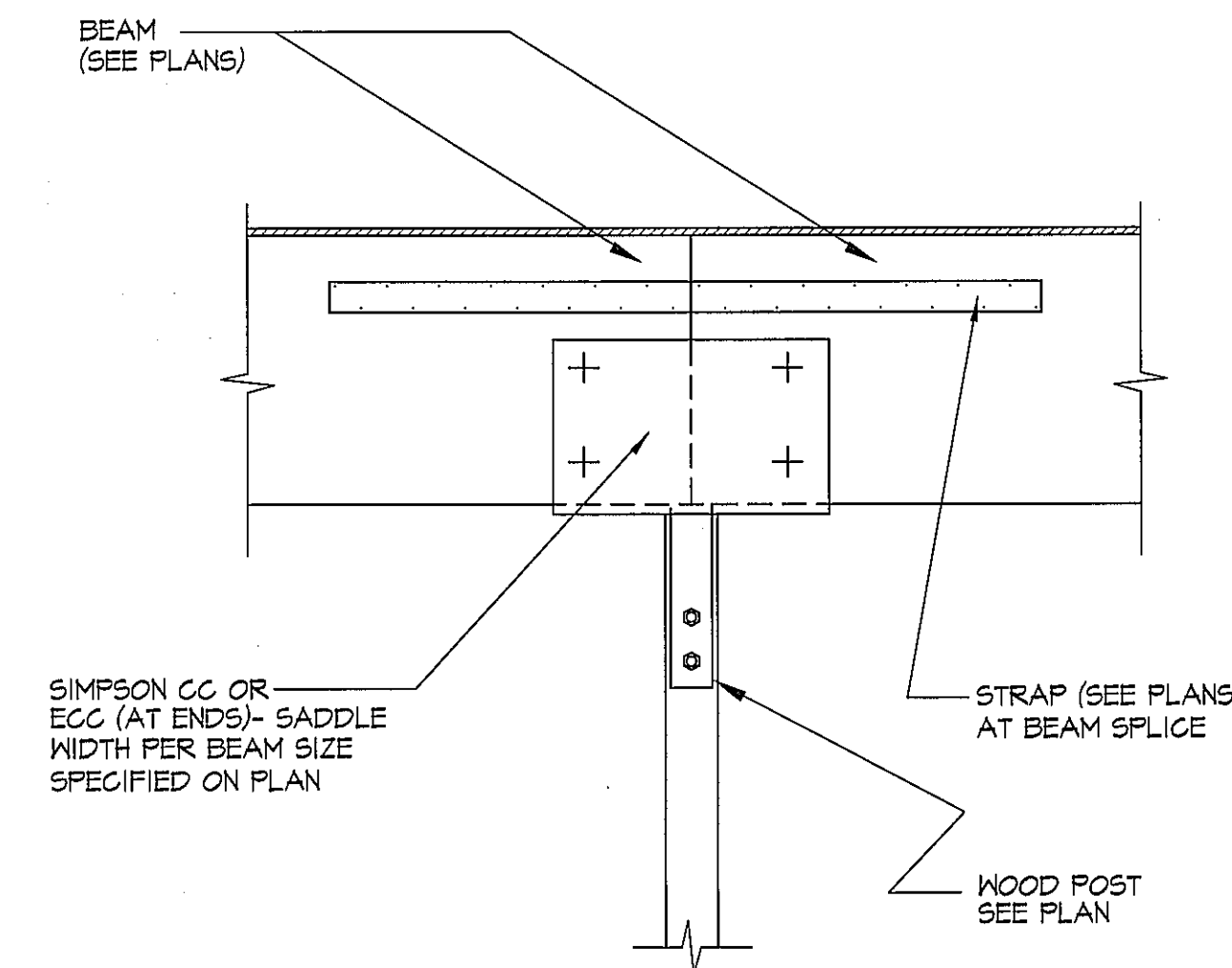
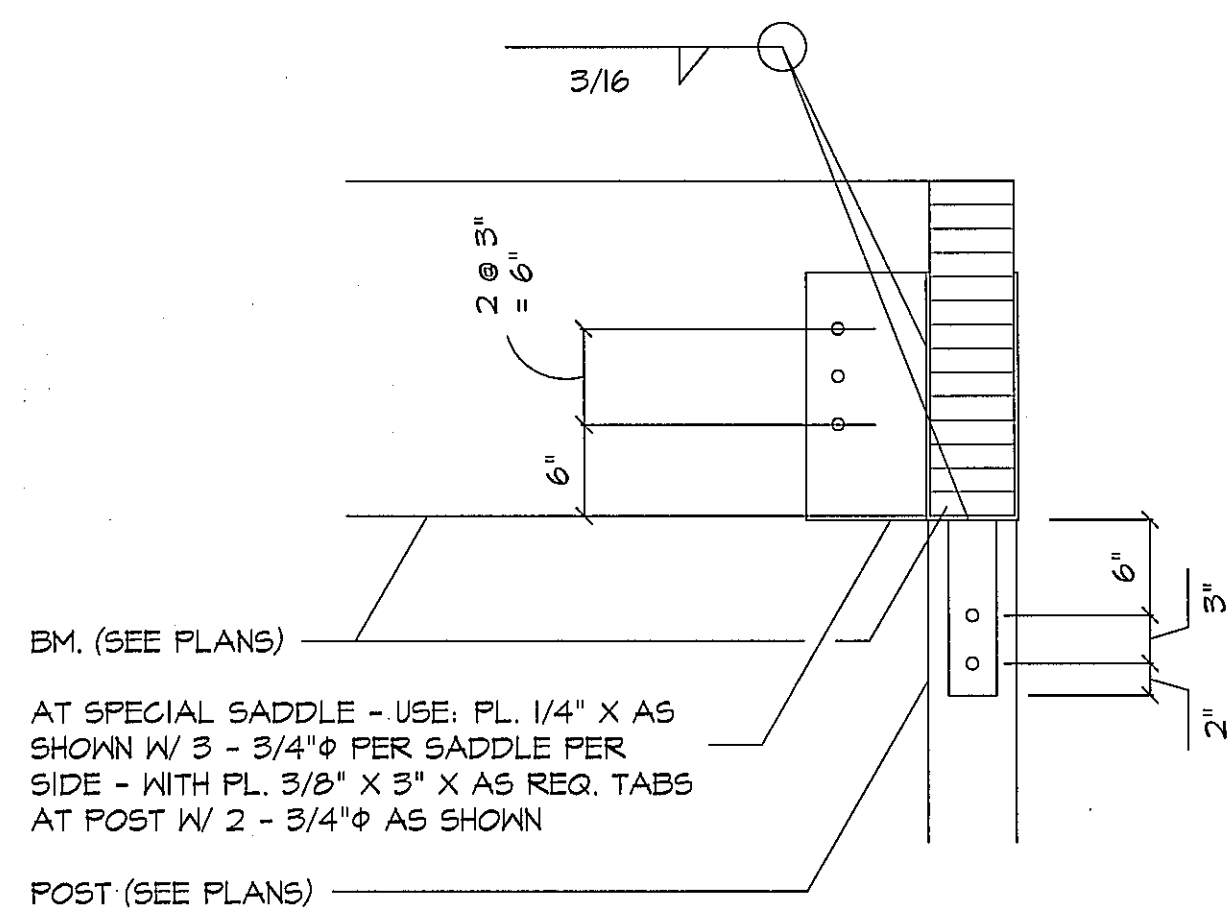
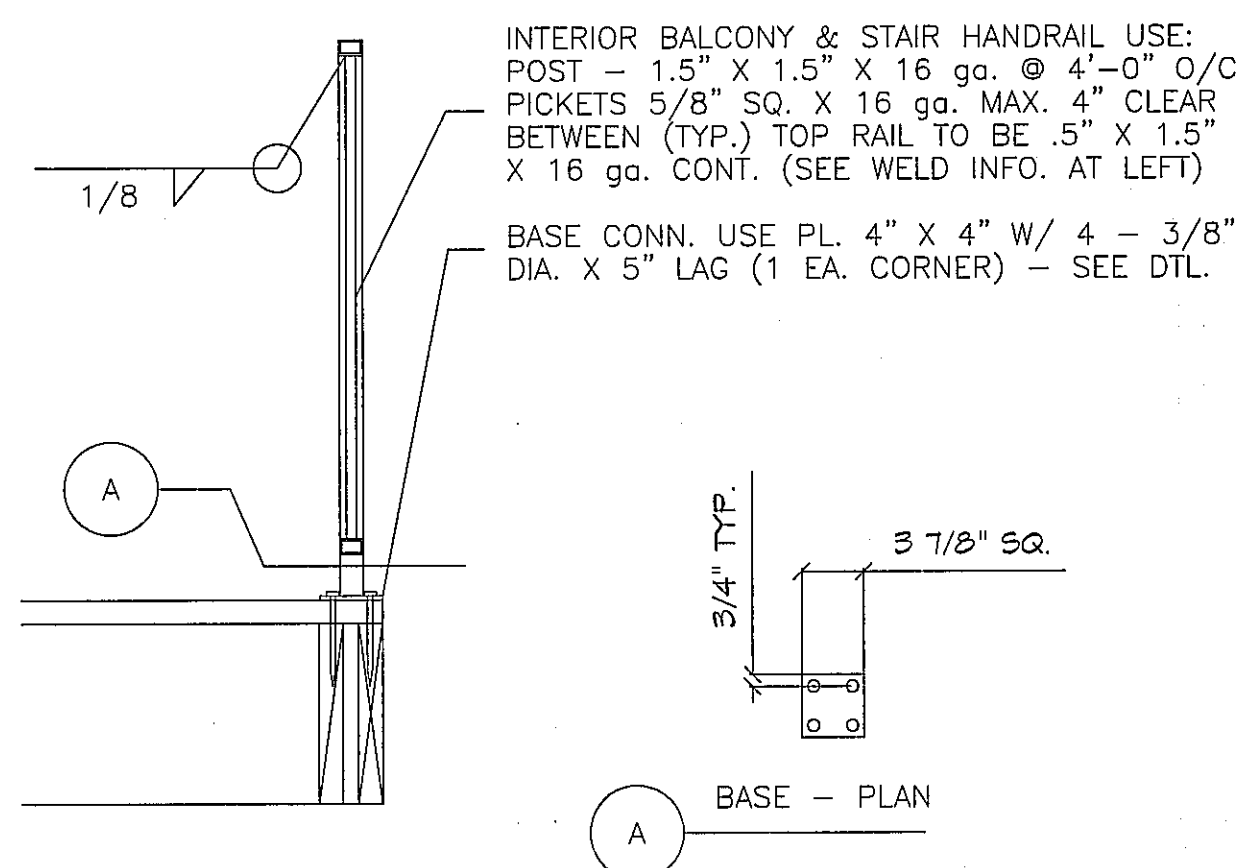


NOTE: SHEAR TRANSFER (S.T.) CAN BE ACHIEVED IN SEVERAL DIFFERENT WAYS. HARDWARE IS REQ. WHEN PLYWOOD BREAKS AT PLATES, HOWEVER WHEN PLYWOOD BREAKS ON RIM JOIST/BLKG. HARDWARE CAN BE DELETED. EN. TO SAME RIM JST./BLKG., SILL & TOP PL. REQ.

HOLD DOWN FLOOR TO FLOOR 7

SHEAR TRANSFER AT INTERIOR WALL 4

SHEAR TRANSFER AT F.J. PARALLEL TO WALL 1

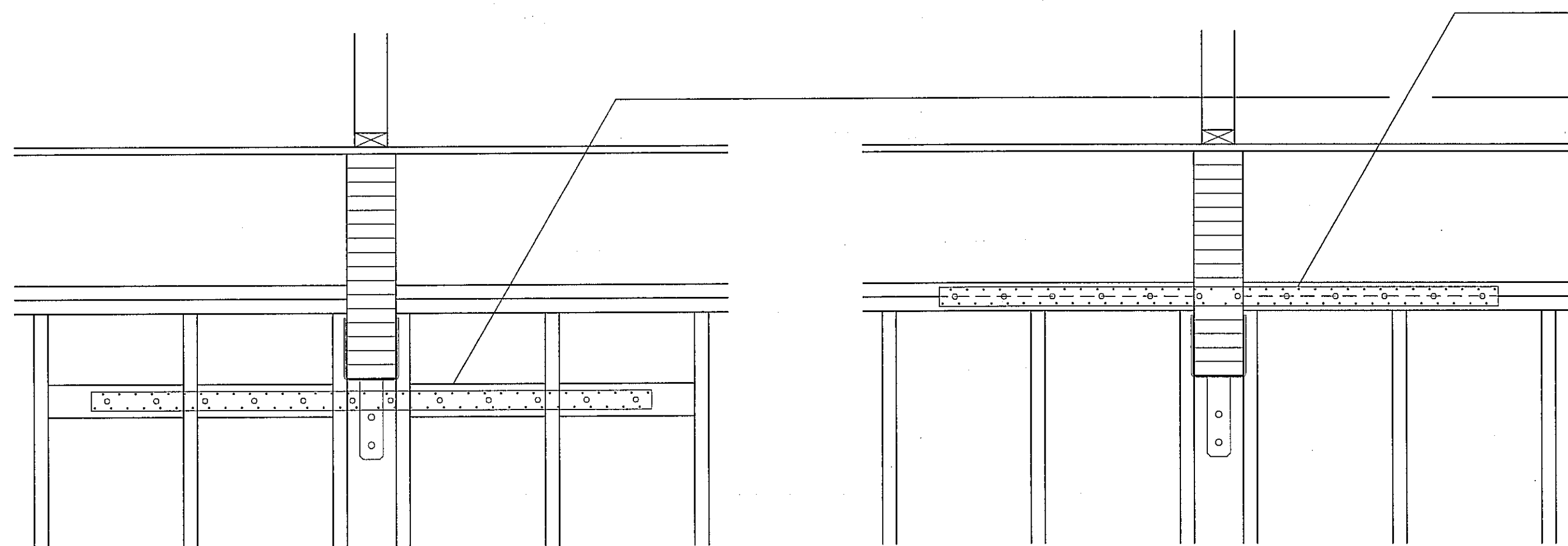


TYP. HANDRAIL DETAIL 11

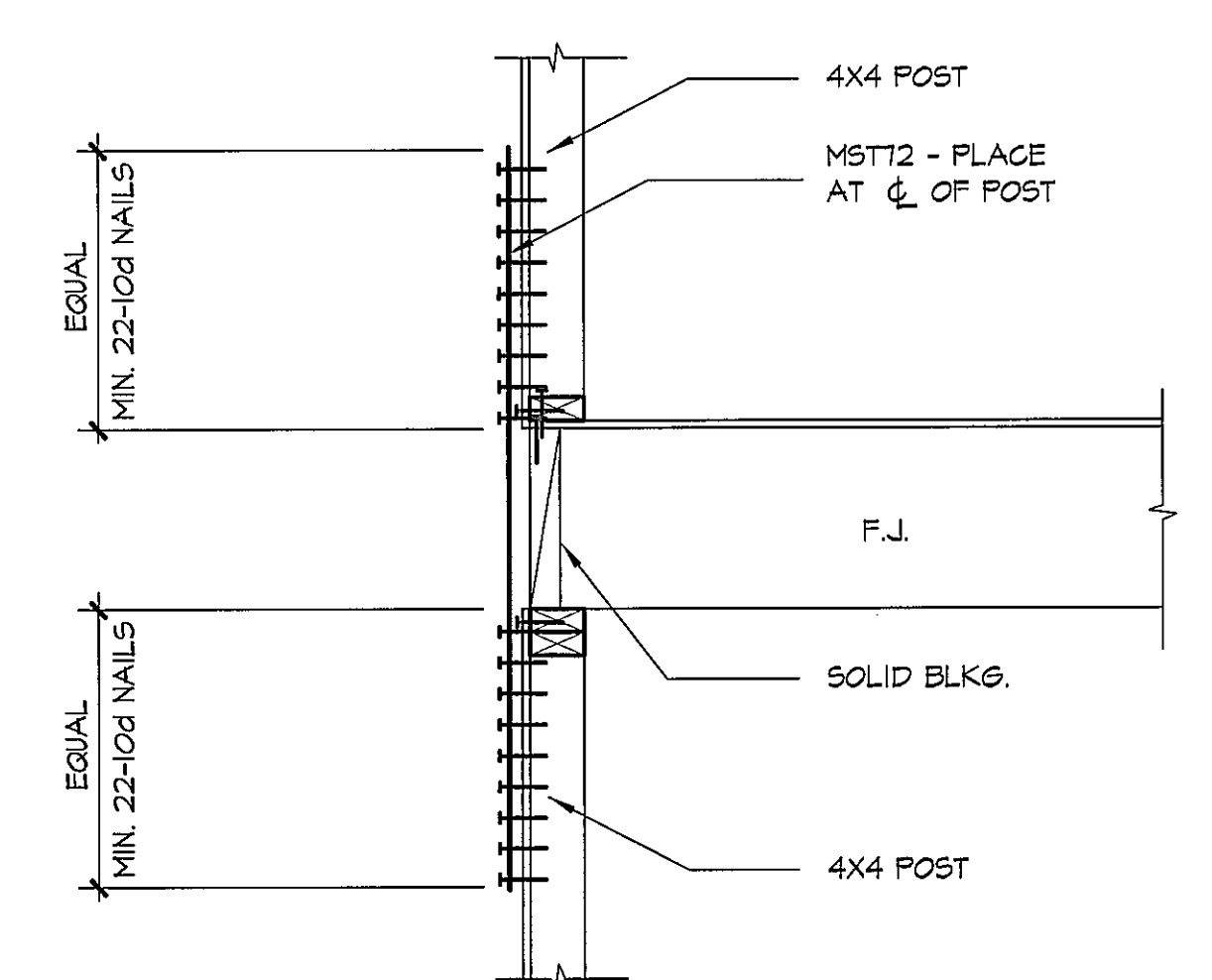
BM. TO BM./POST SADDLE DET. 8

TYP. BEAM TO WOOD POST W/ DRAG 5

SHEAR TRANSFER AT F.J. PARALLEL TO WALL 2



AT DROP BEAM WHERE PLATES ARE CUT - STRAP ACROSS W/ MST60 - EXCEPT AT CORNER, CANT. OR CONTINUOUS BEAM OVER WALL AS FOLLOWS:
 A) AT CONT. OR CANT. BEAM - MOVE STRAP BELOW BEAM - ADD 4X4 BLKG. AND STRAP OVER PLYND. NAILED THRU PLYND. INTO 4X4 BLKG. - PLYND. NOT SHOWN FOR SIM. - USE MST60
 C) AT CORNER - RAP STRAP AROUND CORNER SIM. TO DET. 10/S-3.5 - REDUCE STRAP TO MST21
 AT DROP BEAM - SADDLE BM. TO POST - POST TO MATCH BM. WIDTH X WALL WIDTH (6X6 IN MANY CASES) W/ SIMP. CCS 1/4-6, ECCS 1/4-6 IN-LIEU-OF CC OR ECC SIMP. CCQ OR ECCQ IS ALLOWED U.N.O. ON PLANS



A) ALTERNATE STRAP DET.

B) PRIMARY STRAP DET.

C) CORNER BEAM/POST STRAP DET.

CUT PLATES DRAG/CHORD SPLICE CONN STRAP DET. 6

HOLD DOWN STRAP FLOOR TO FLOOR 3

REVISIONS	BY

HRD ENGINEERING
 STRUCTURAL DESIGN AND DRAFTING
 7463 VARNA AVE., SECOND FLOOR
 N. HOLLYWOOD, CA 91605
 PHONE NO. (805) 431-5415 - FAX NO. (805) 888-4840
 COPYRIGHT © HRD ENGINEERING

THESE PLANS/DRAWINGS DESIGNED AND SPECIFICATIONS ARE THE SOLE PROPERTY OF HRD ENGINEERING FOR THE EXCLUSIVE USE OF ITS CLIENTS FOR THE PROJECT AND NOT TO BE REPRODUCED, COPIED, TRANSFER OF USE NOT PERMITTED WITHOUT WRITTEN APPROVAL. HRD ENGINEERING TO USE.

FRAMING DETAILS

TWO STORY SINGLE FAMILY RESIDENCE
 901 LAUREL CYN.
 LA, CA 90046 FOR: L.I. INVESTMENTS, LLC

REGISTERED PROFESSIONAL ENGINEER
 No. SE2628
 STATE OF CALIFORNIA

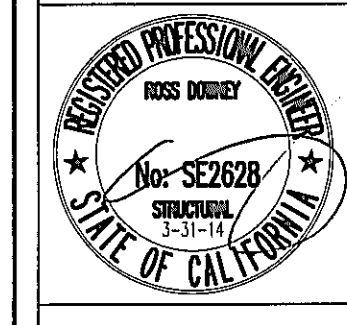
Date 11-27-12
 Scale
 Drawn hrd
 Job
 Sheet
 S-3.6
 of

REVISIONS	BY

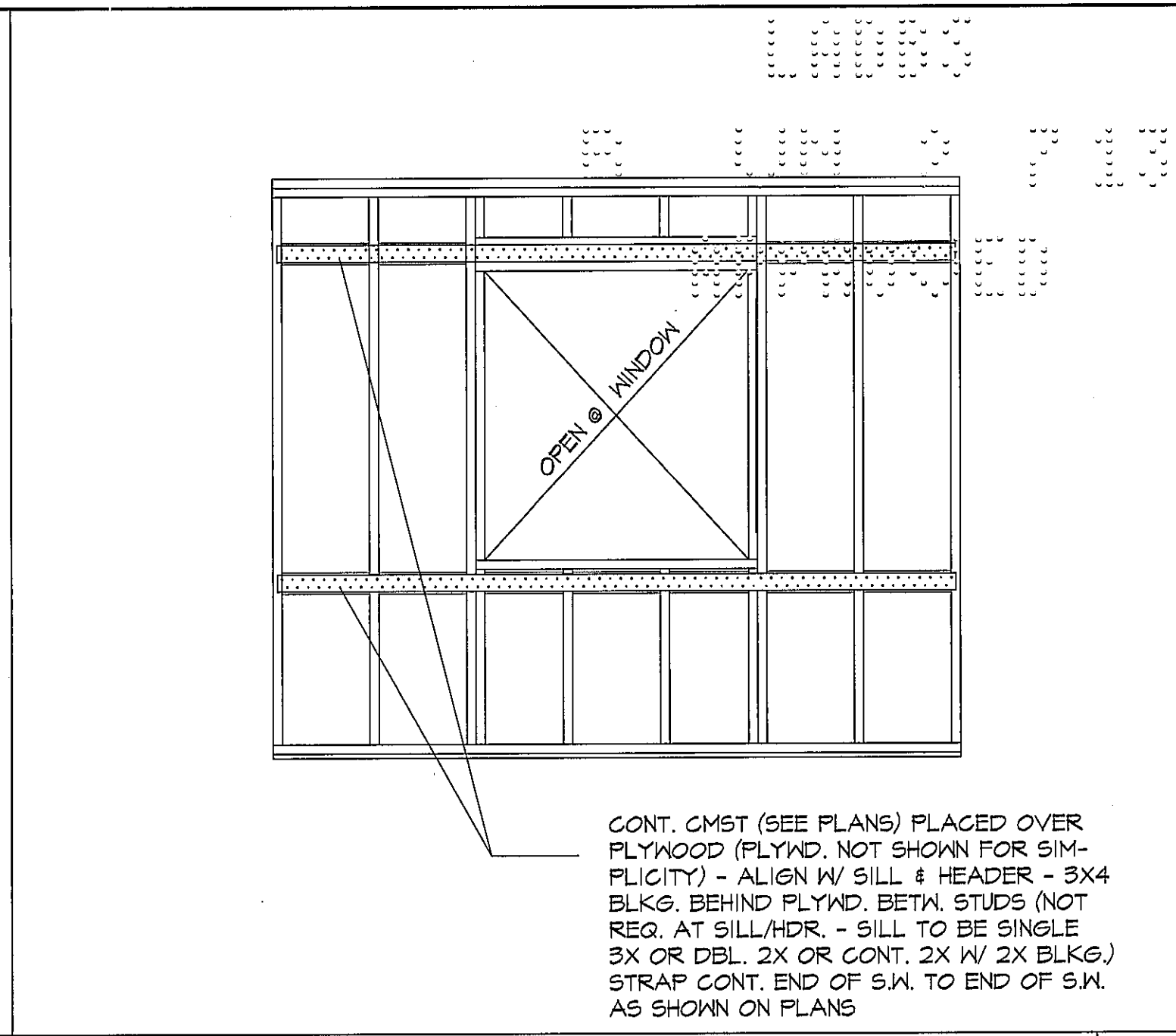
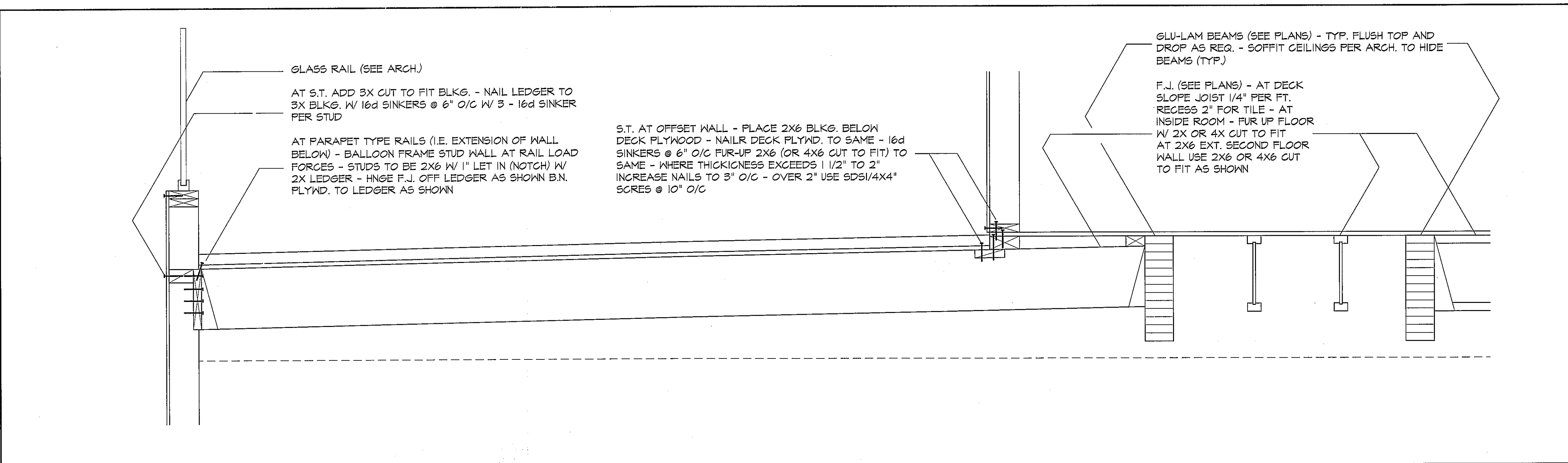
HRD ENGINEERING
 STRUCTURAL DESIGN AND DRAFTING
 7463 VARNA AVE., SECOND FLOOR
 N. HOLLYWOOD, CA 91605
 PHONE NO. (818) 431-5415 - FAX NO. (818) 888-6840
 THESE PLANS DRAWINGS, DESIGN AND SPECIFICATIONS ARE THE PROPERTY OF HRD ENGINEERING FOR THE EXCLUSIVE USE OF ITS CLIENTS FOR THE SPECIFIC PROJECT WITH APPROVAL BY HRD ENGINEERING TO FORM, REPRODUCE, TRANSMIT, COPY, REPRODUCE, OR OTHERWISE DISSEMINATE WITHOUT WRITTEN APPROVAL. COPYRIGHT © HRD ENGINEERING.

FRAMING DETAILS

TWO STORY SINGLE FAMILY RESIDENCE
 901 LAUREL CYN.
 LA, CA 90046 FOR: L.I. INVESTMENTS, LLC

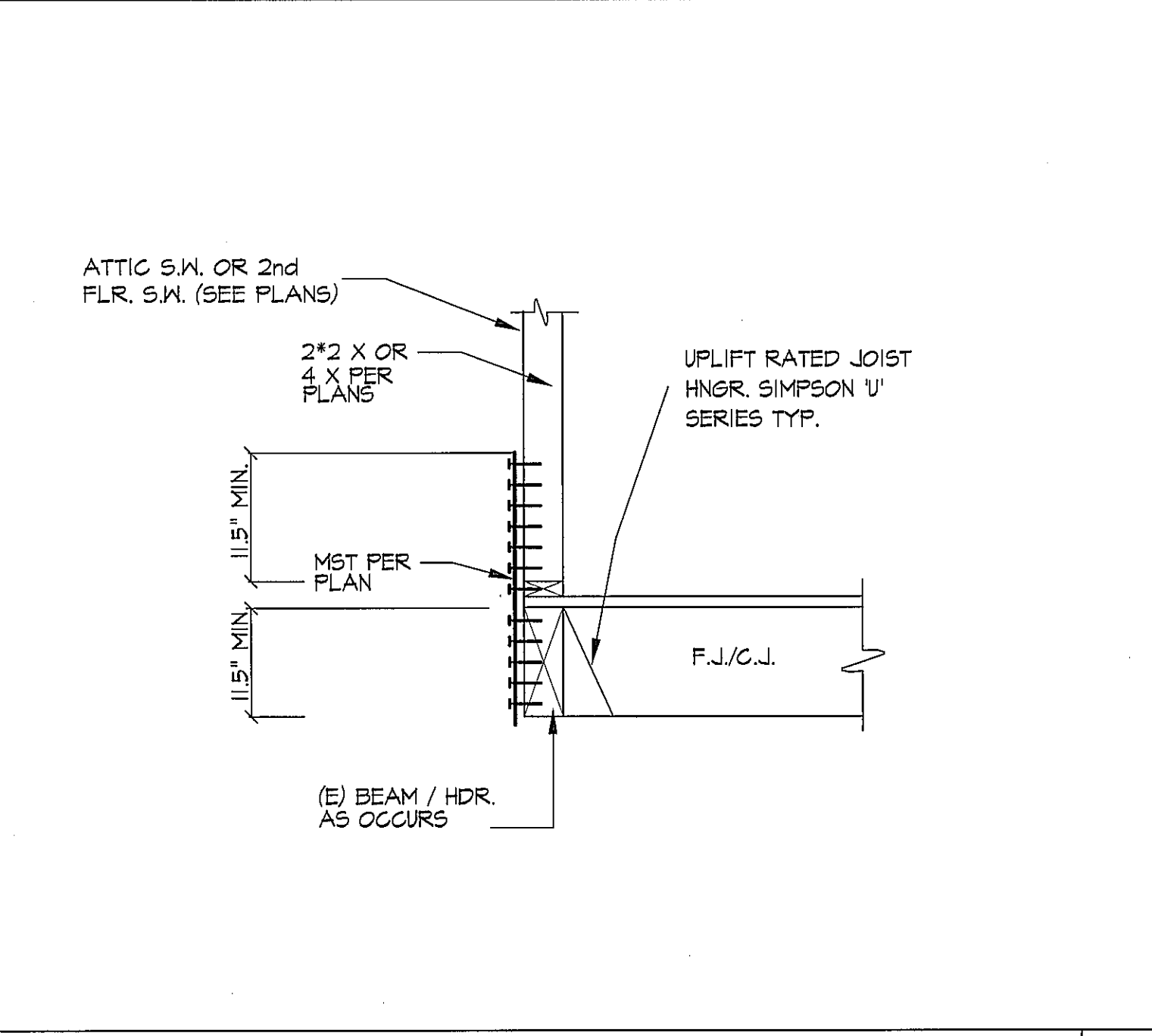
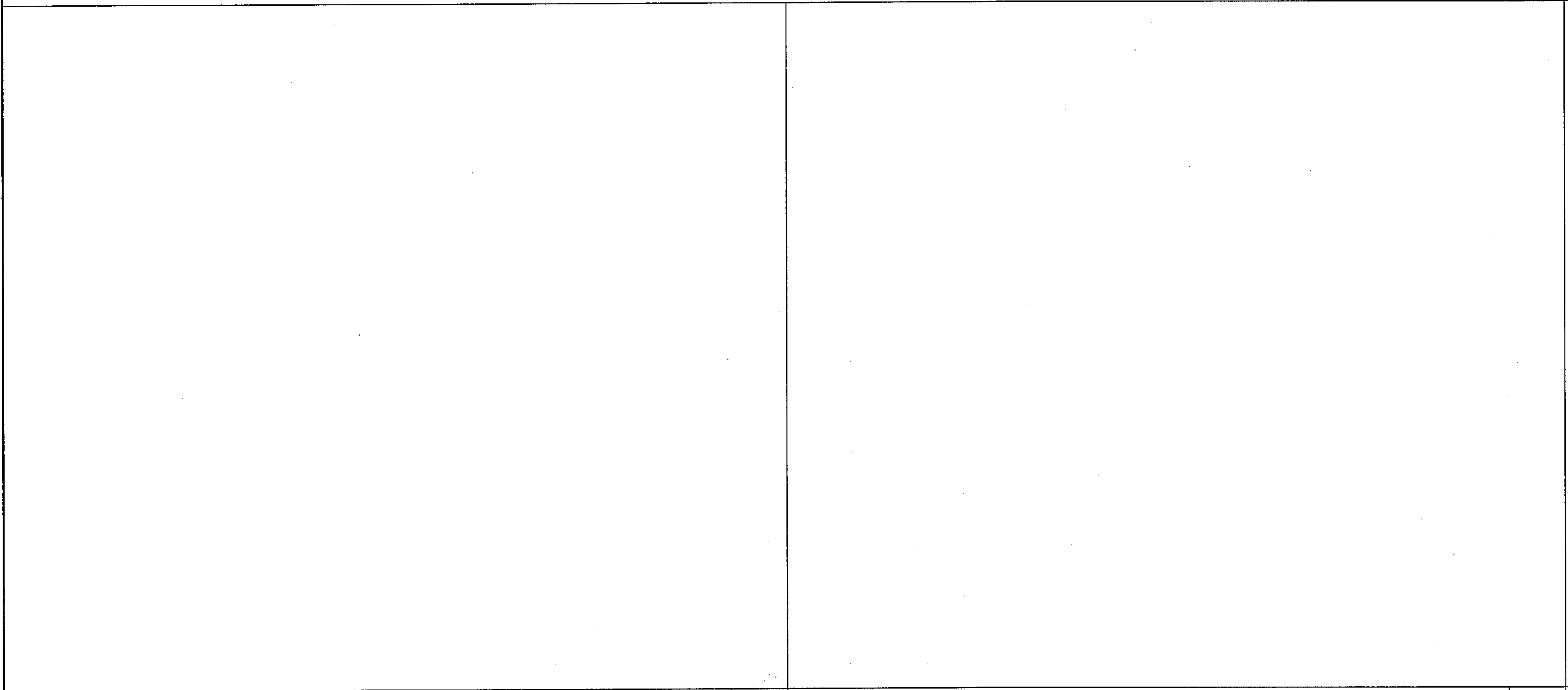


Date	11-27-12
Scale	
Drawn	hrd
Job	
Sheet	S-3.7
of	



FRAMING SECTION/DET. AT TOP OF STAIRS PATIO 4

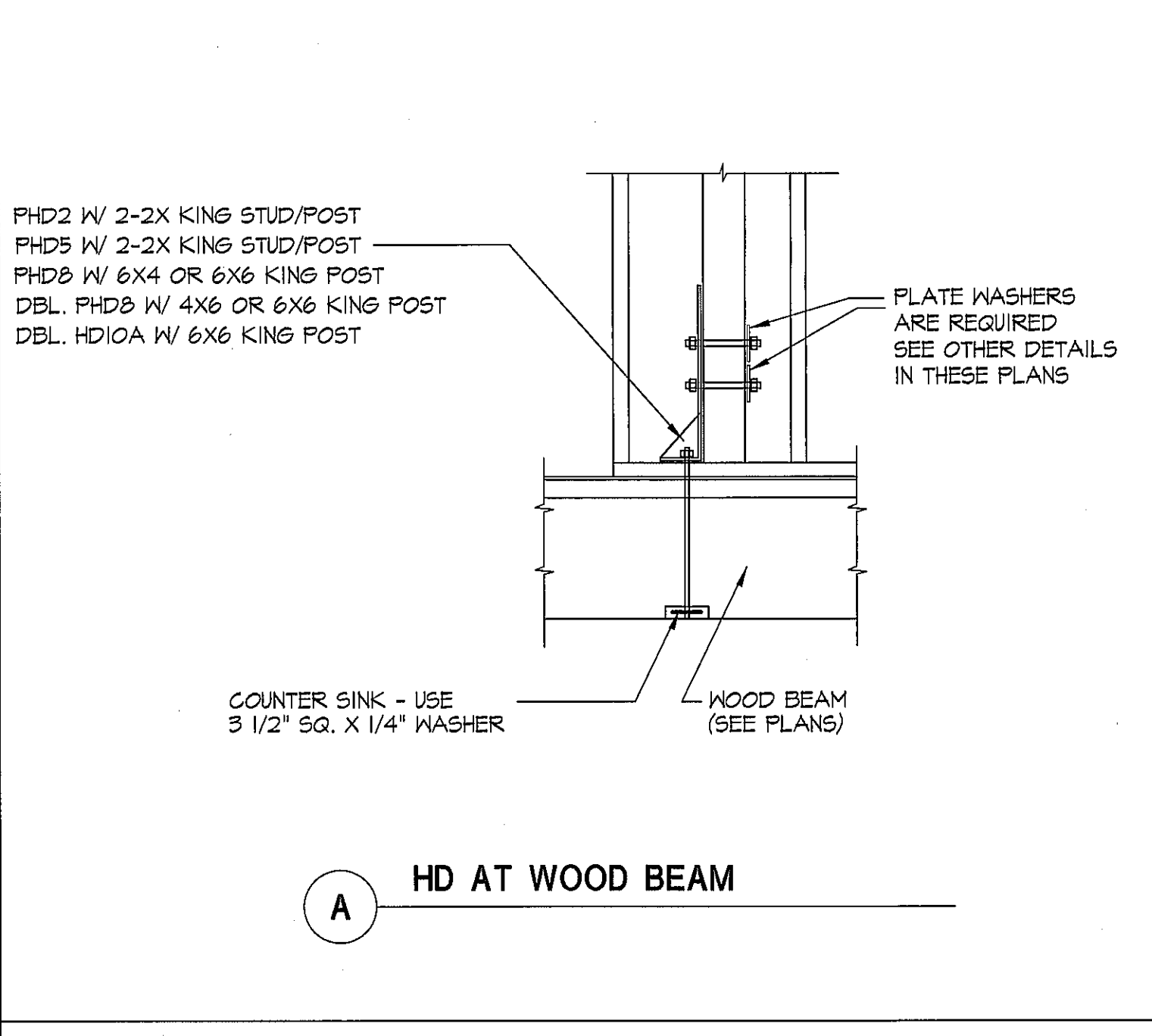
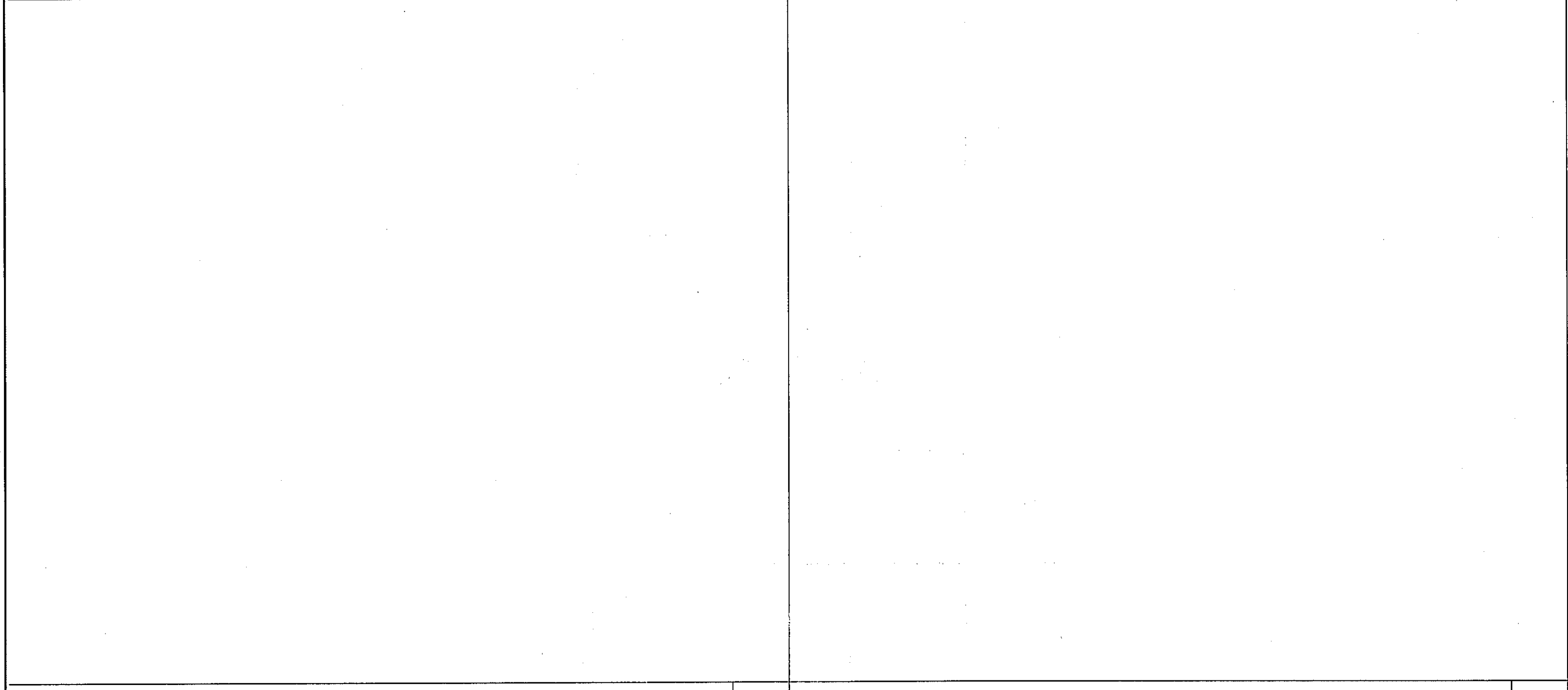
SHEAR WALL W/ OPENING INSIDE 1



11 8

HOLD DOWN STRAP TO FLUSH BM. 5

DIAGONAL STRAPS AT OPEN'G CORNERS (TYP.) 2

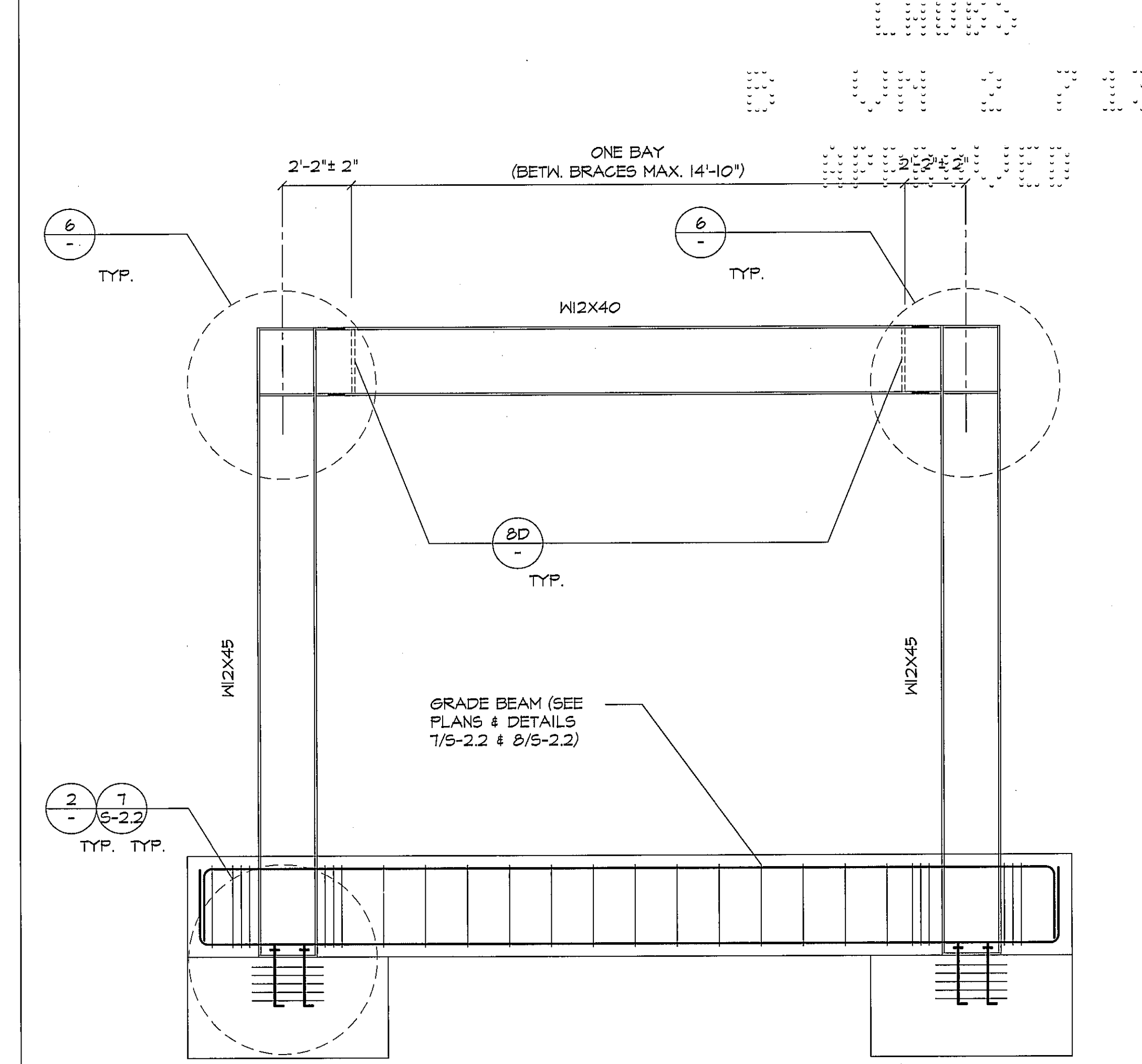
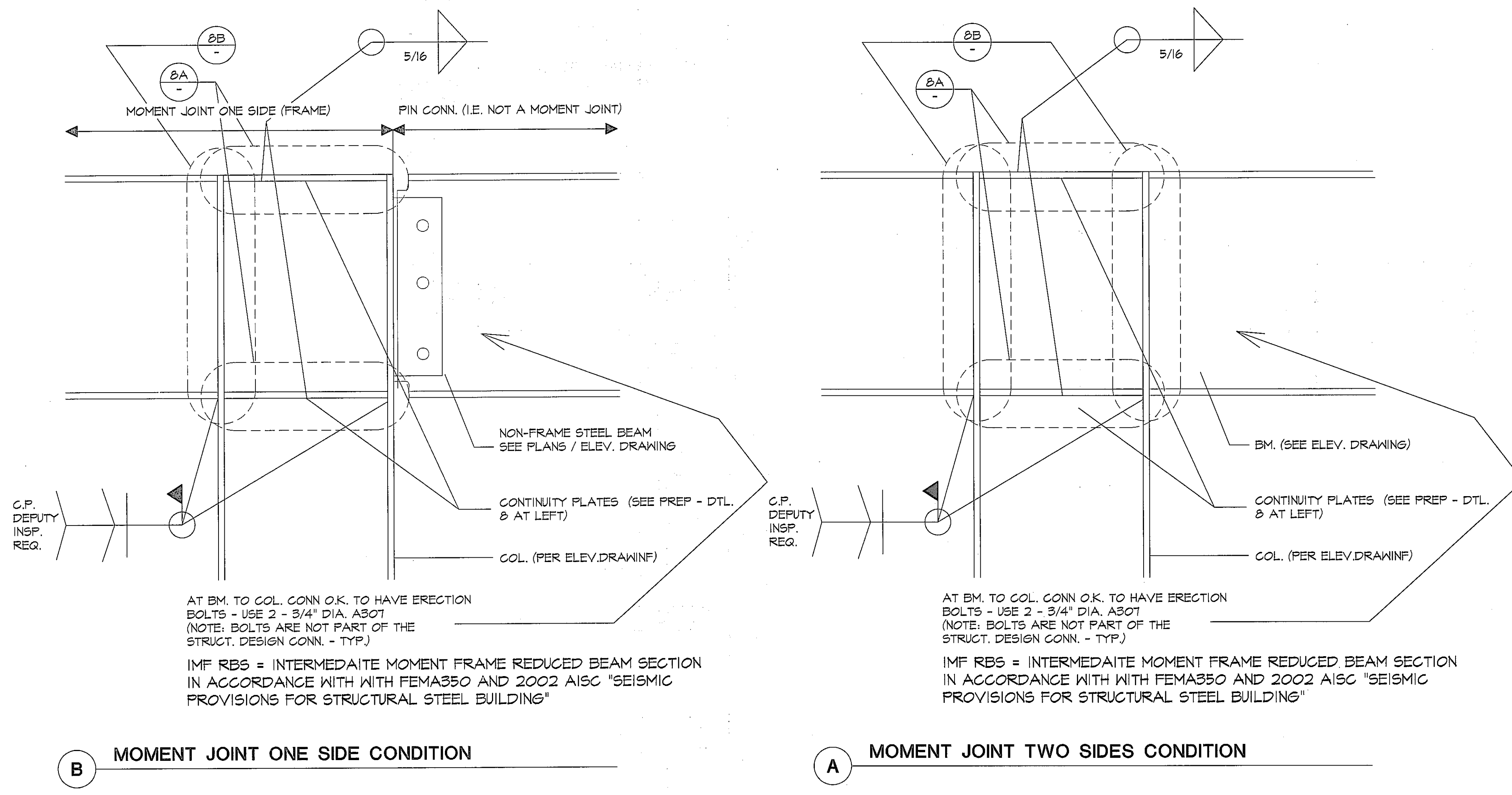


12 9

A HD AT WOOD BEAM

B HD AT STEEL BEAM

HOLD DOWN AT WOOD/STEEL BEAM BELOW 3



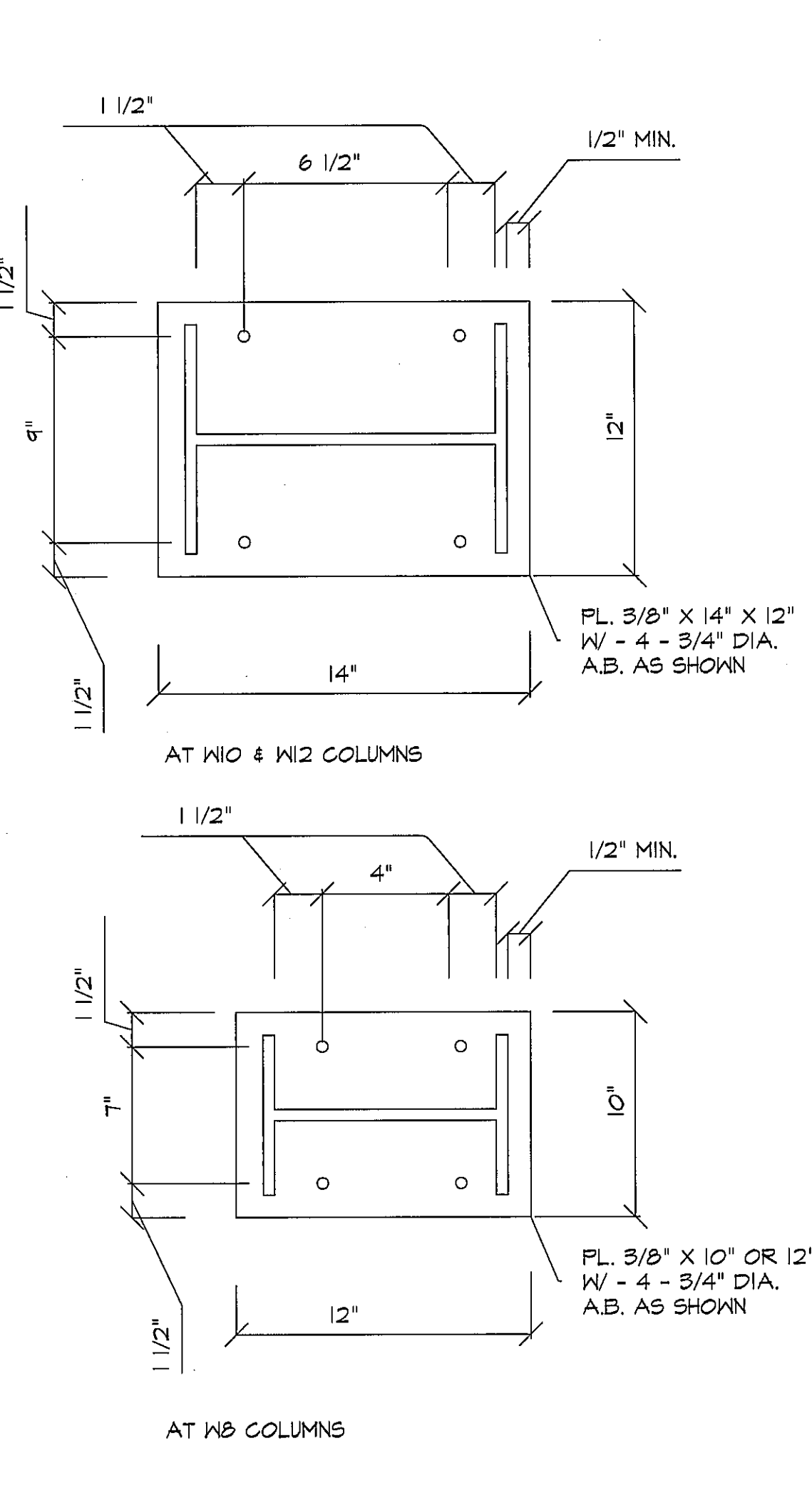
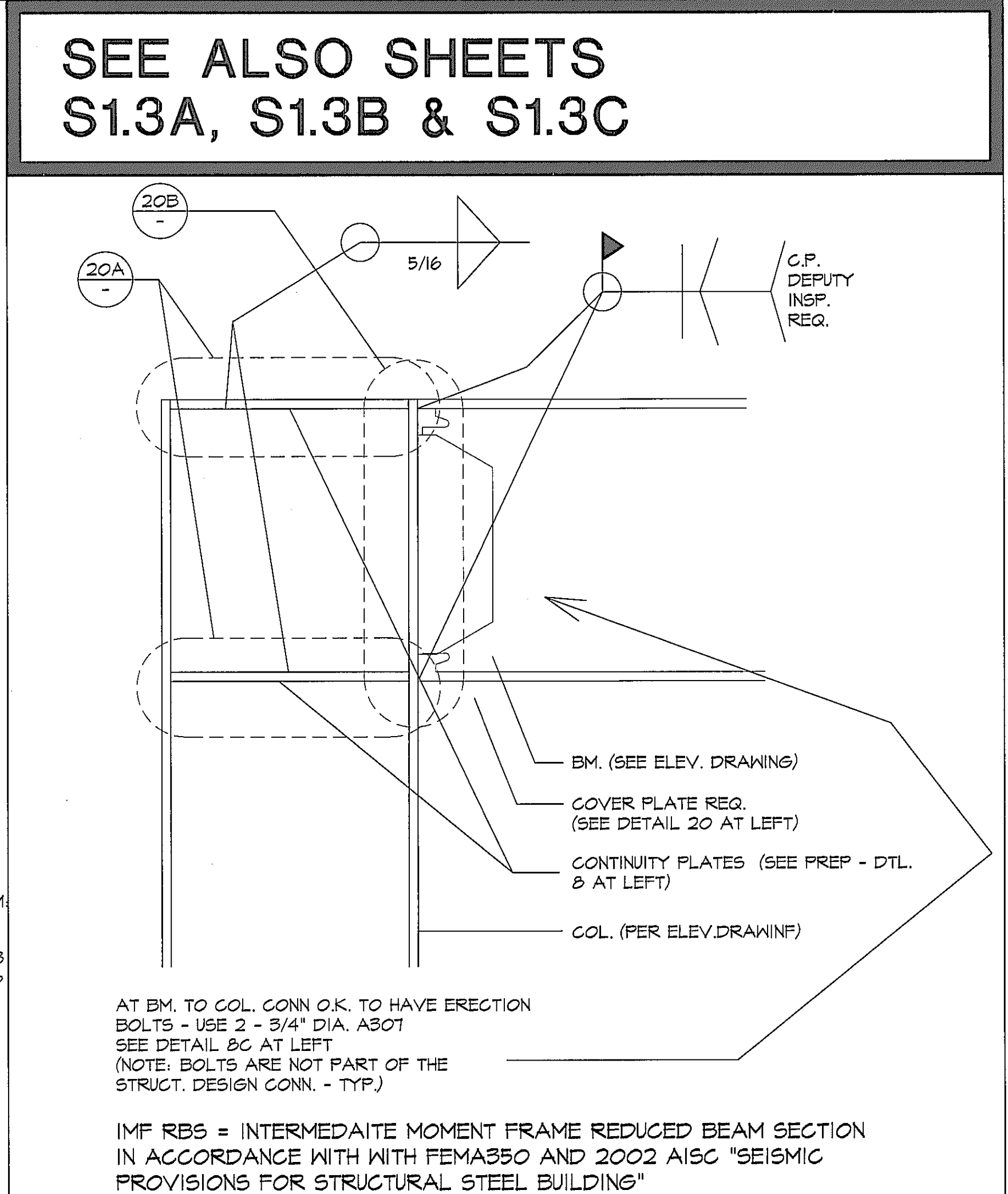
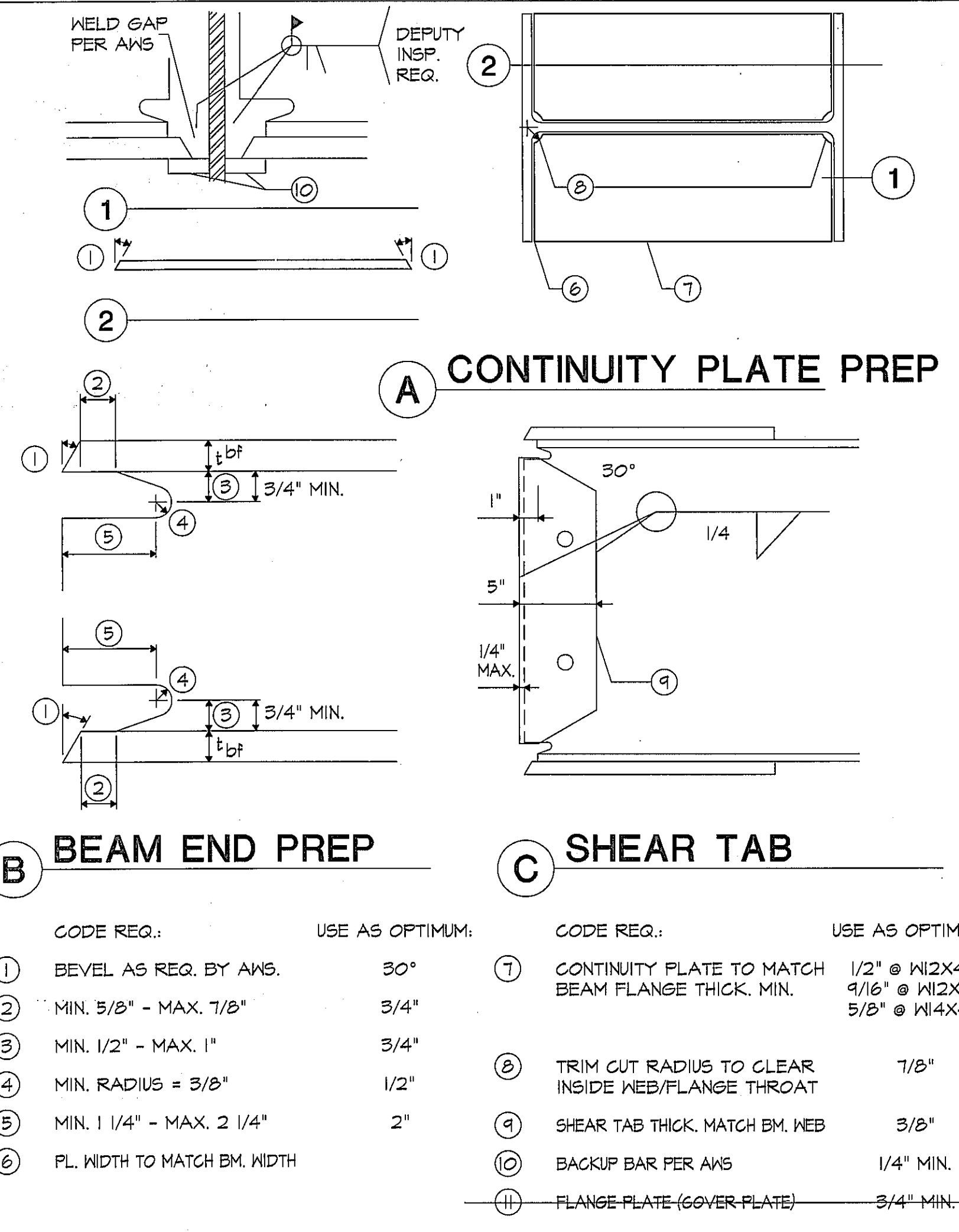
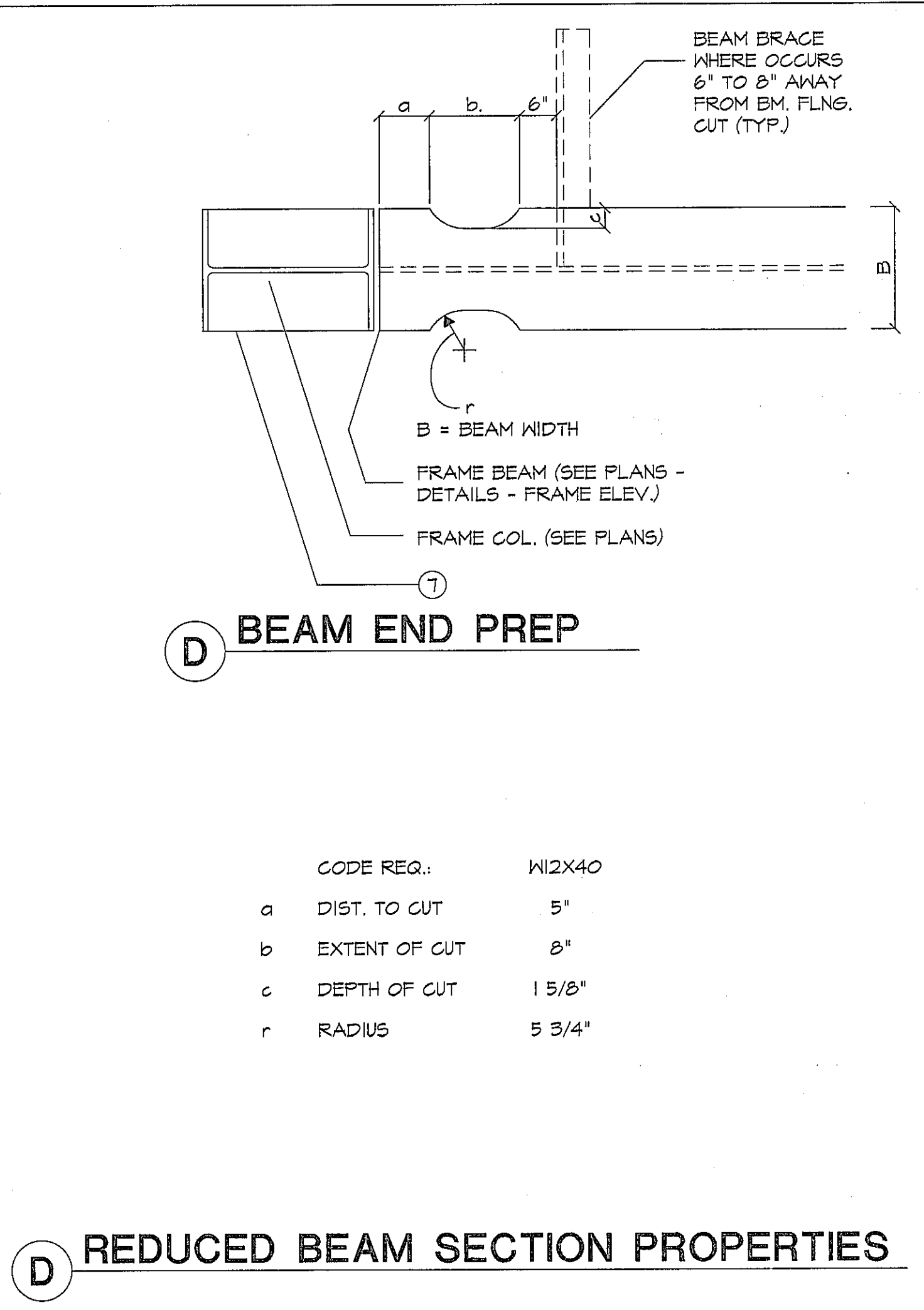
REVISIONS BY

THESE DRAWINGS, DESIGNS AND SPECIFICATIONS ARE THE SOLE PROPERTY OF HRD ENGINEERING FOR THE EXCLUSIVE USE OF ITS CLIENTS FOR THE PROJECT AND ANY REUSE, REPRODUCTION, TRANSMISSION, OR ANY OTHER USE WITHOUT WRITTEN APPROVAL OF HRD ENGINEERING. COPYRIGHT © HRD ENGINEERING.

HRD ENGINEERING
 STRUCTURAL DESIGN AND DRAFTING
 7463 VARNA AVE., SECOND FLOOR
 N. HOLLYWOOD, CA 91605
 PHONE NO. (805) 431-5415 - FAX NO. (805) 888-4840

MOMENT JOINT CONN. DETAIL INTER. FOR IMF-RBS COL. TO BEAM (TYP.) 7

FRAME ELEVATION GRID LINE 2 PARCEL 1 1



RBS GEOMETRY AND WELD PREP (BEAM TO COL.) FOR SMF 8

MOM. JOINT CONN. DTL. IMF RBS BM. TO COL. 6

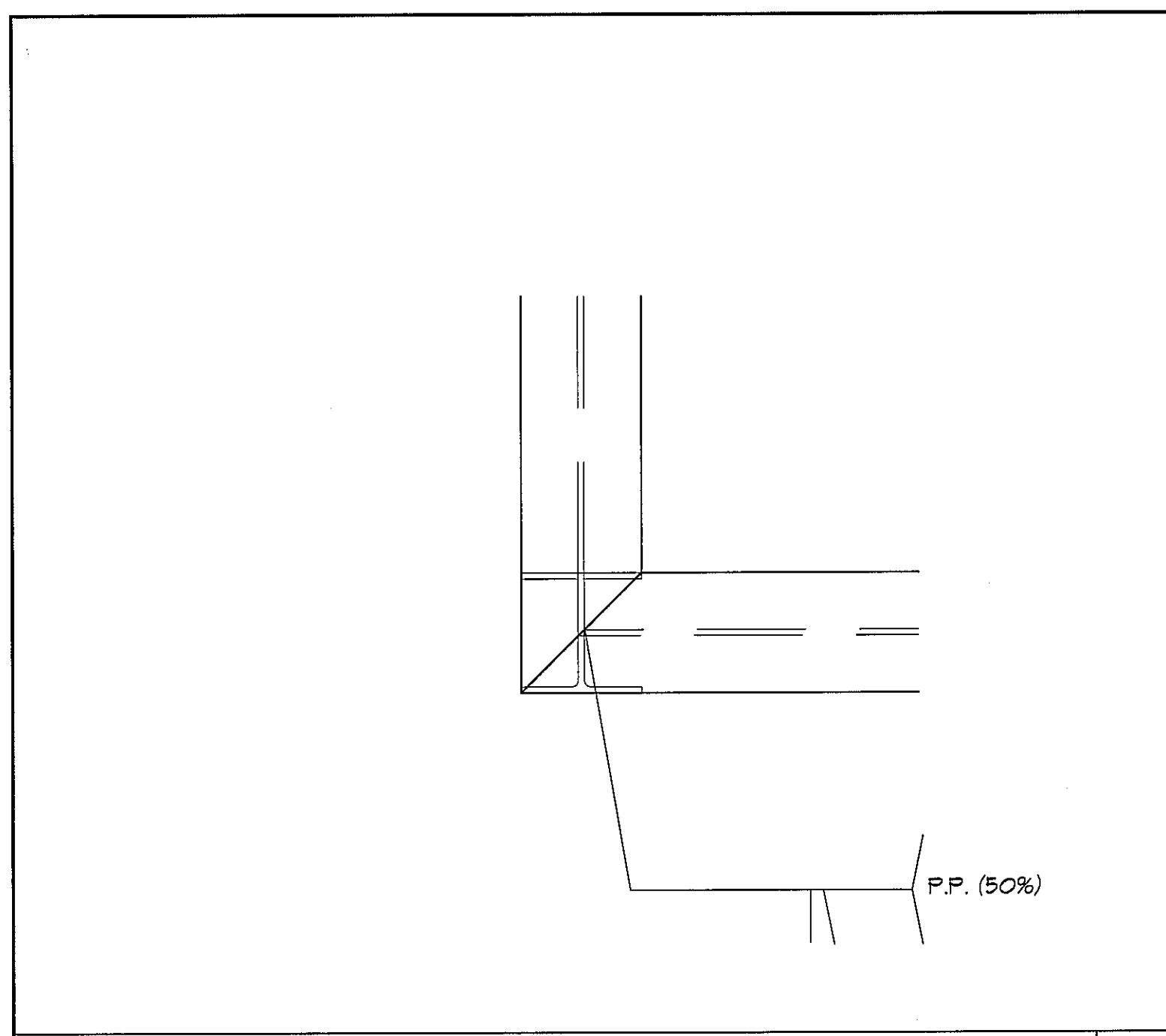
BASE PLATE DETAIL 2

FRAMING DETAILS

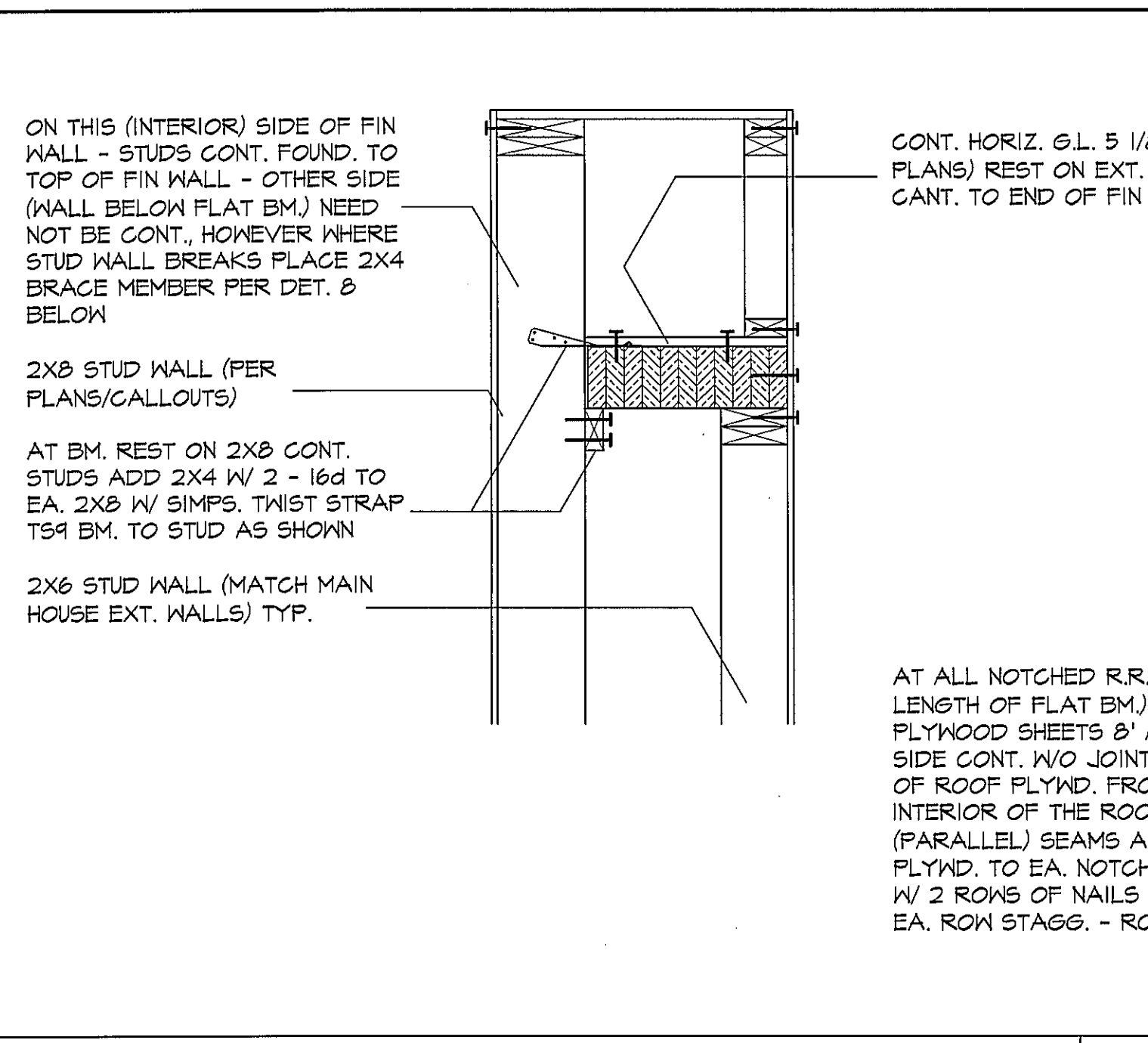
TWO STORY SINGLE FAMILY RESIDENCE
 901 LAUREL CYN.
 LA, CA 90046 FOR: L.I. INVESTMENTS, LLC

RECORDED PROFESSIONAL ENGINEER
 No. SE2628
 STATE OF CALIFORNIA

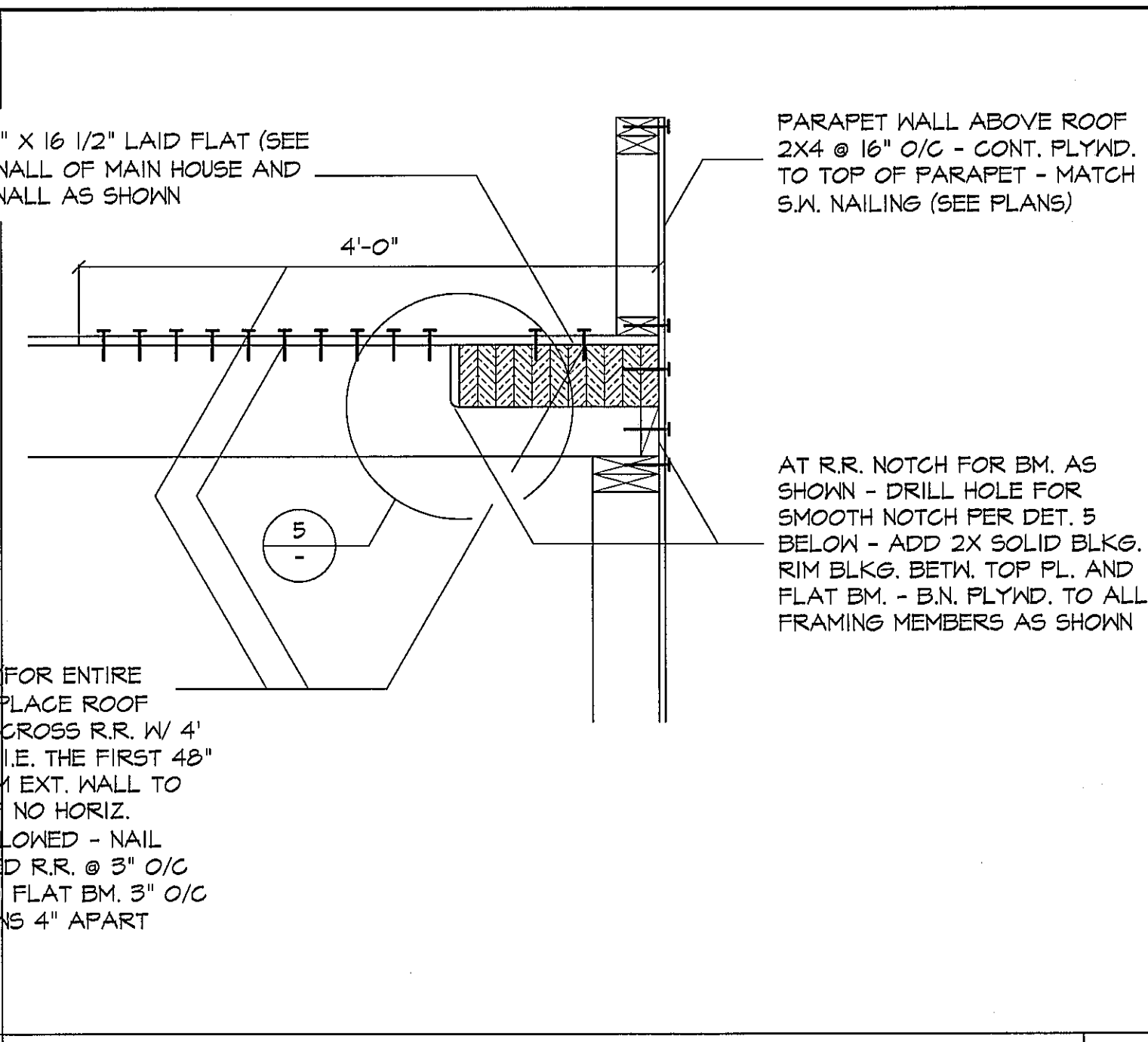
Date 11-27-12
 Scale
 Drawn hrd
 Job
 Sheet S-3.8



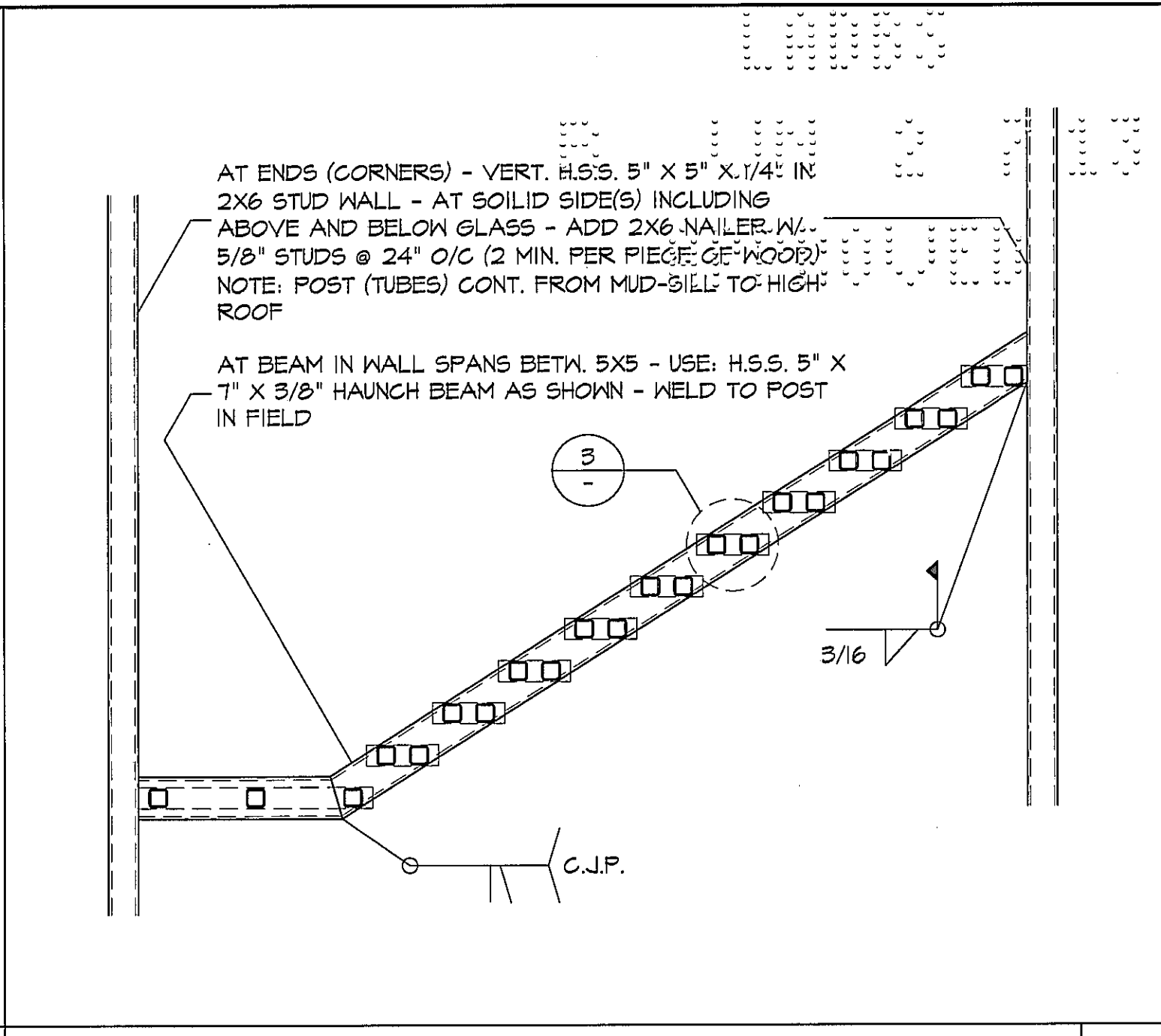
CORNER BM. TO BM. MITTER CONN. DET. 10



CANT. HORIZ. BM. AT FIN WALL 7



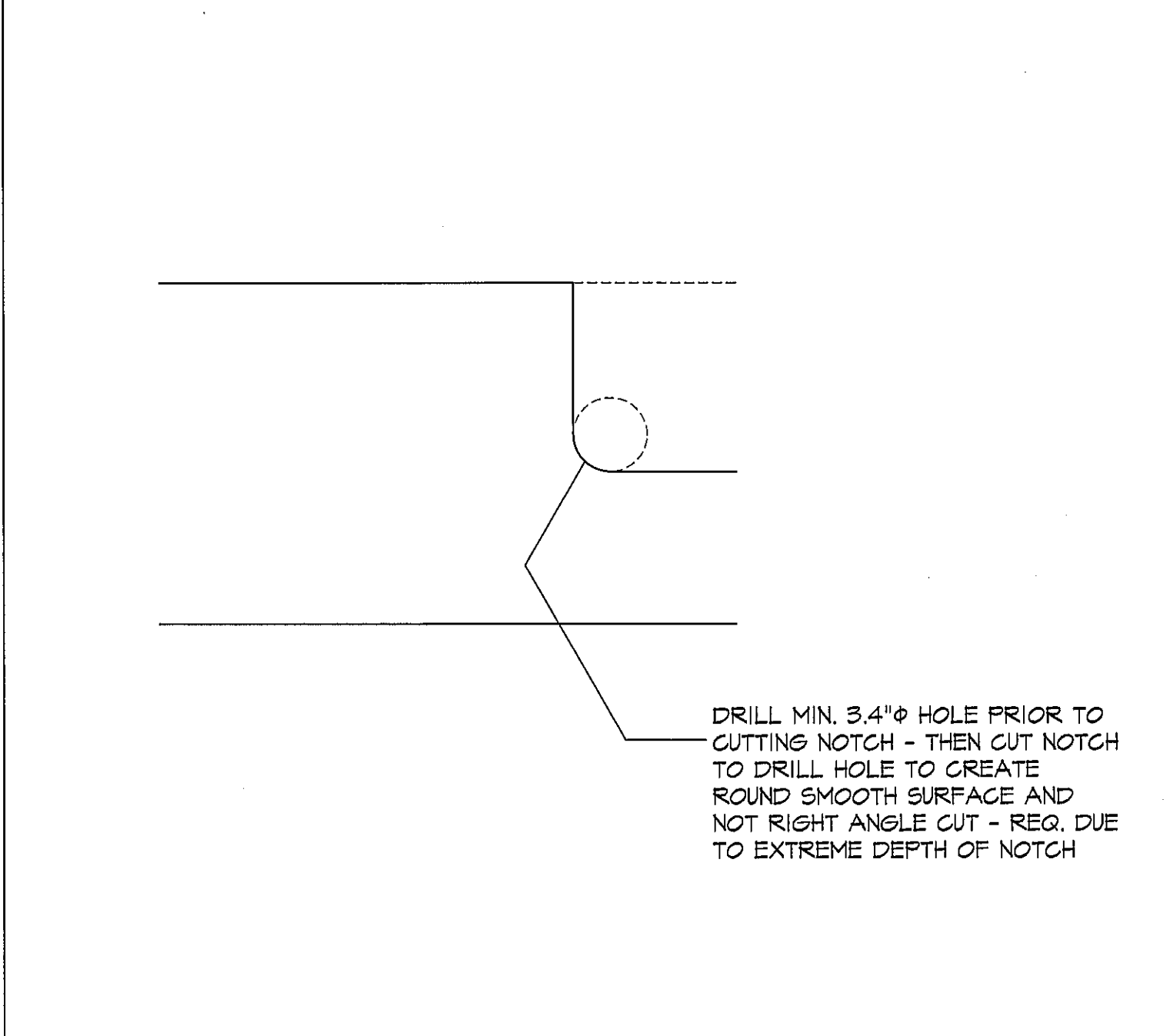
CANT. HORIZ. BM. AT FIN AT MAIN ROOF 4



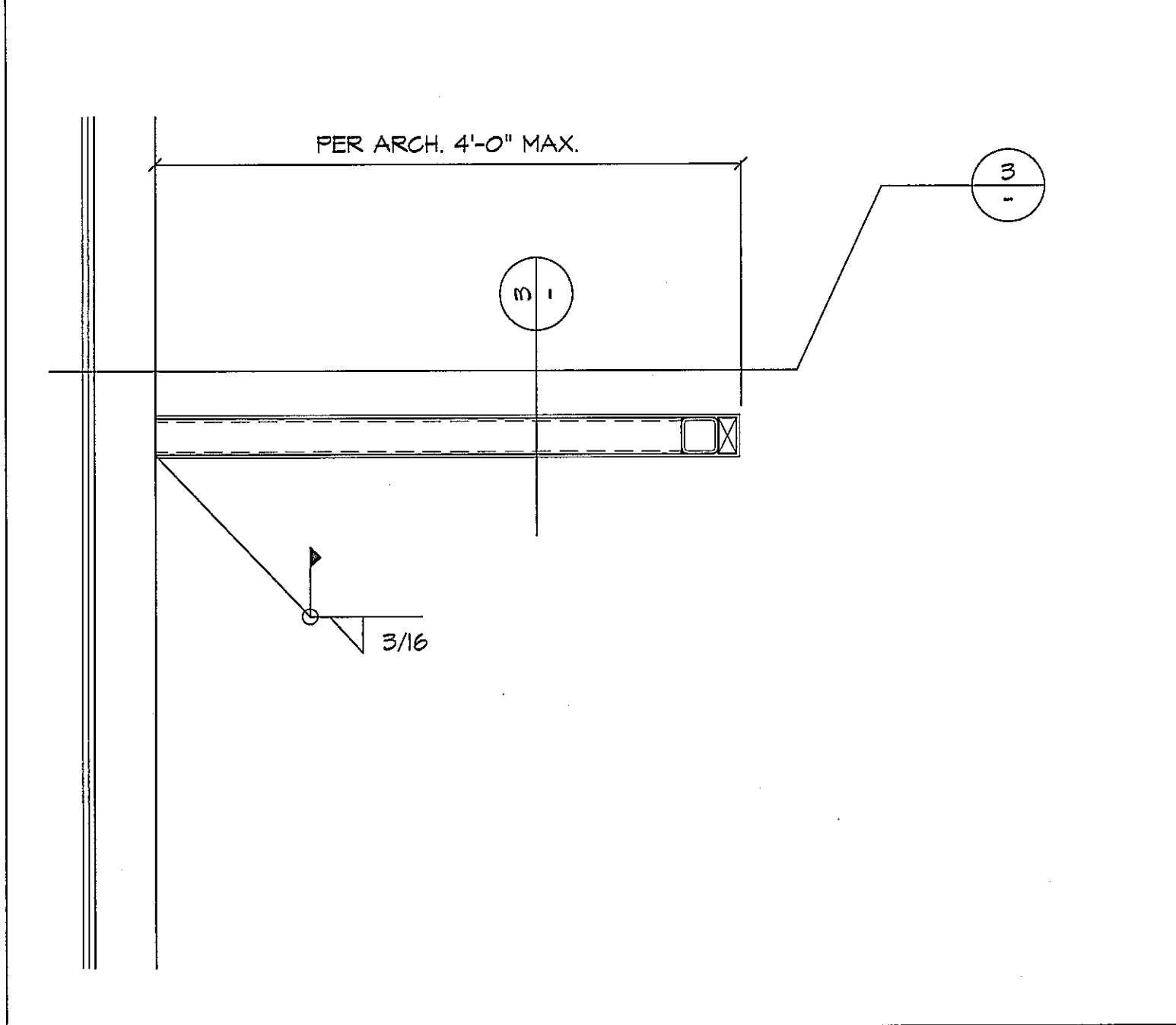
FLOATING STAIRS ELEV VIEW 1



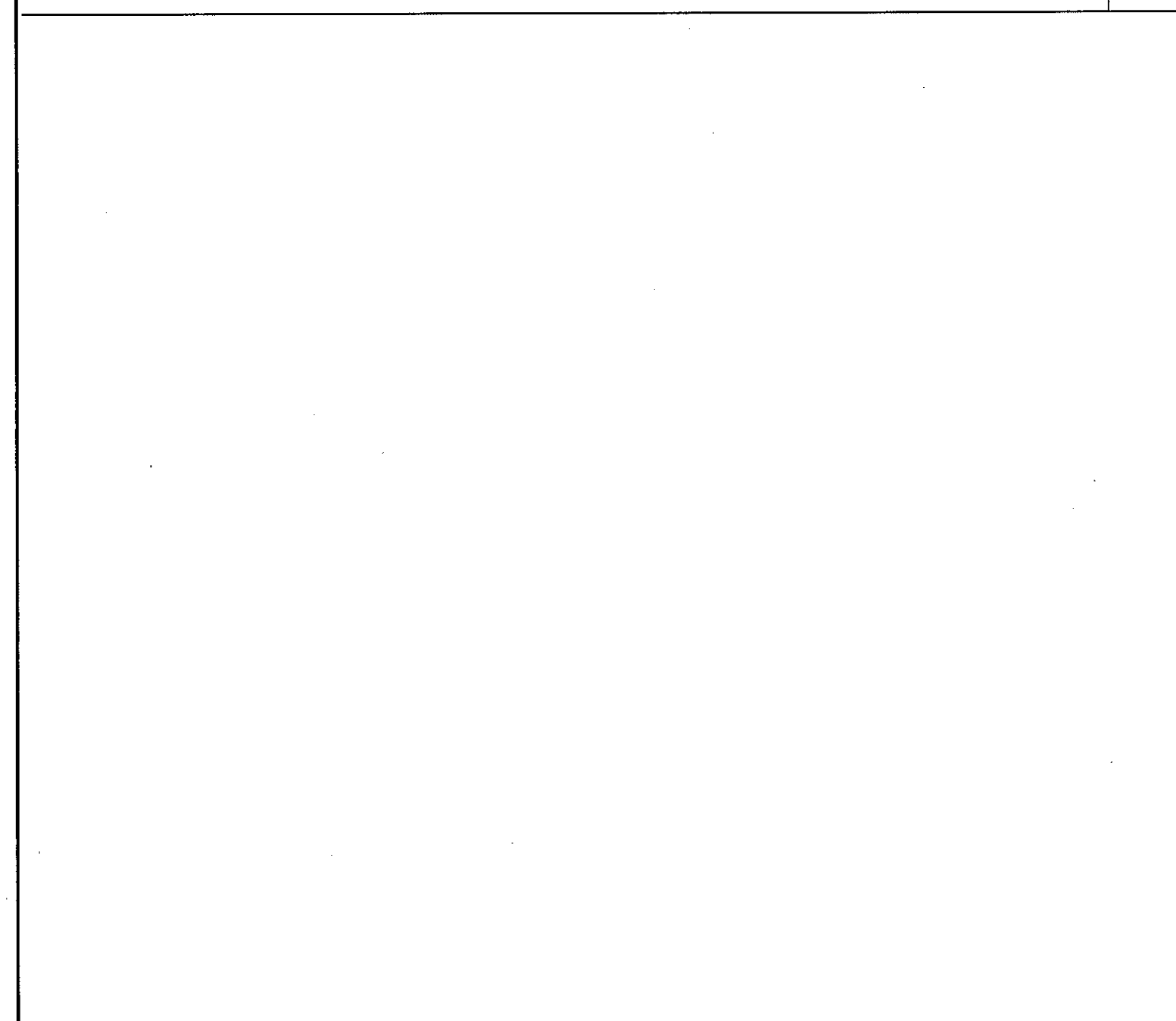
2X6 STUD WALL SPLICE OPTION 8



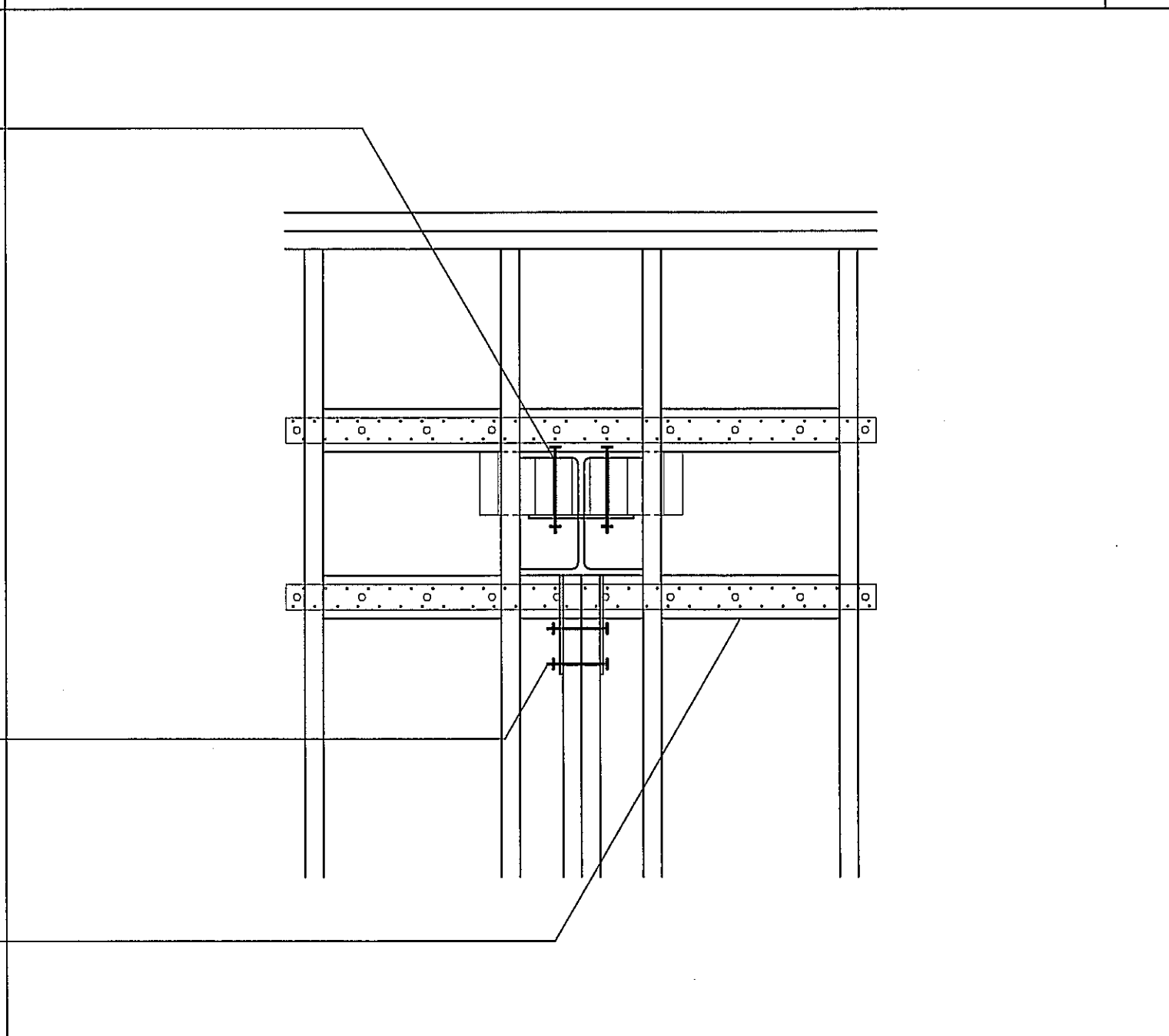
DET. DRILLED HOLE NOTCH SPEC. 5



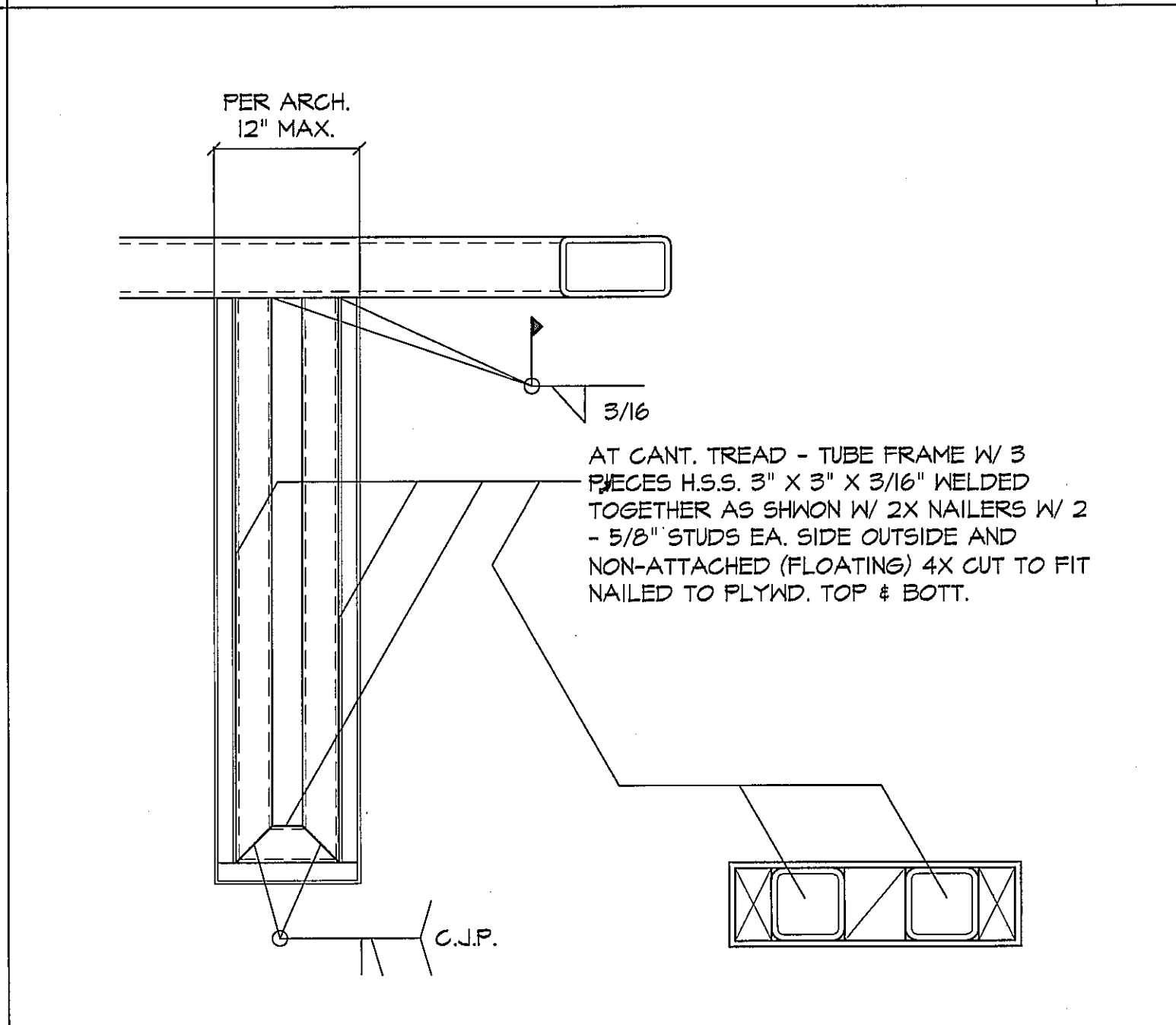
FLOATING STAIRS WALL SECTION 2



STEEL BM. AT FIN WALL SECTION 9



STEEL BM. AT FIN WALL ELEV. 6



FLOATING STAIRS TREAD ASSEMBLY 3

REVISIONS	BY

THESE PLANS, DRAWINGS, DESIGNS AND SPECIFICATIONS ARE HEREBY OFFERED TO THE CLIENT FOR THE EXCLUSIVE USE OF THIS PROJECT WITH APPROVAL BY HRD ENGINEERING TO USE, REPRODUCE OR TRANSMIT IN ANY MANNER WITHOUT WRITTEN APPROVAL. COPYRIGHT © HRD ENGINEERING.

HRD ENGINEERING
STRUCTURAL DESIGN AND DRAFTING
7463 VARNA AVE., SECOND FLOOR
N. HOLLYWOOD, CA 91605
PHONE NO. (805) 431-5415 - FAX NO. (805) 888-4840

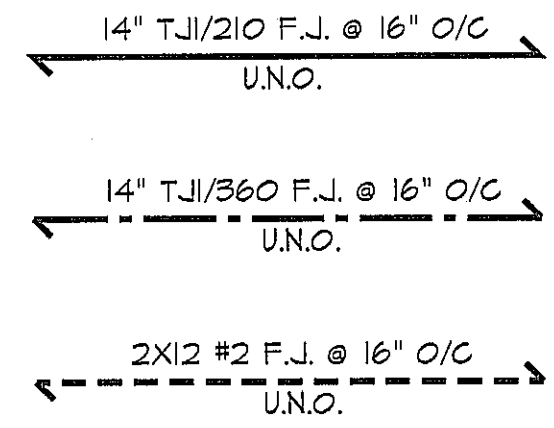
FRAMING DETAILS

TWO STORY SINGLE FAMILY RESIDENCE
901 LAUREL CYN.
LA, CA 90046 FOR: L.I. INVESTMENTS, LLC

PROFESSIONAL ENGINEER
ROSS DONNEY
No. SE2828
STRUCTURAL
5-14-14
STATE OF CALIFORNIA

Date 1-14-13
Scale
Drawn hrd
Job
Sheet
S-3.9
of

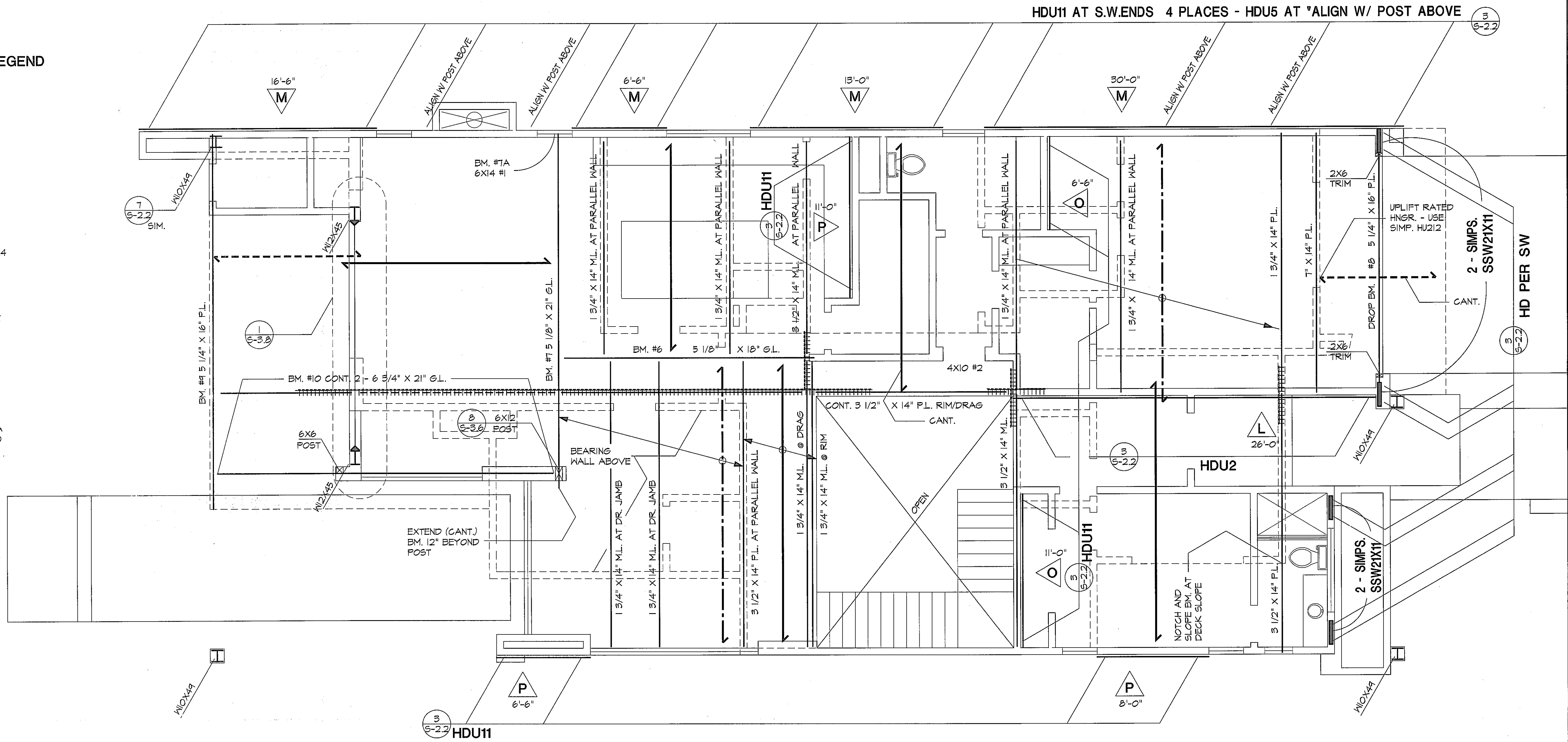
SECOND FLOOR FRAMING LEGEND



||||| M5T12 (U.N.O.) SEE DTL. 8/5-3.4

NOTES:

1. ALL EXT. WALLS TO BE 2X6 @ 16" O/C - ALL INTERIOR WALLS TO BE 2X4 @ 16" O/C, EXCEPT FOR SHEAR WALLS - USE 3X4 OR 2X6 STUDS @ 16" O/C.
2. ALL G.L. (GLULAM) BEAMS TO HAVE STANDARD CAMBER RADIUS 2000' (ALTERNATE RADIUS ALLOWED WHEN APPROVED IN WRITING IN ADVANCE OF INSTALLATION).
3. HIGH STRESSED DIAPHRAGM HATCHED AREA HERE - 4X4 BLKG. ALL EDGES W/ 10d COMM. @ 2/3/10



SECOND FLOOR FRAMING PLAN

SCALE: 1/4"=1'-0"

REVISIONS	BY

THESE PLANS, DRAWINGS, DESIGNS AND SPECIFICATIONS ARE THE SOLE PROPERTY OF HRD ENGINEERING FOR THE PROJECT AND NOT TO BE REPRODUCED, COPIED, OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, WITHOUT WRITTEN APPROVAL. HRD ENGINEERING IS NOT PERMITTED TO USE ANY PART OF THESE PLANS FOR ANY OTHER PROJECT WITHOUT WRITTEN APPROVAL. COPYRIGHT © HRD ENGINEERING.

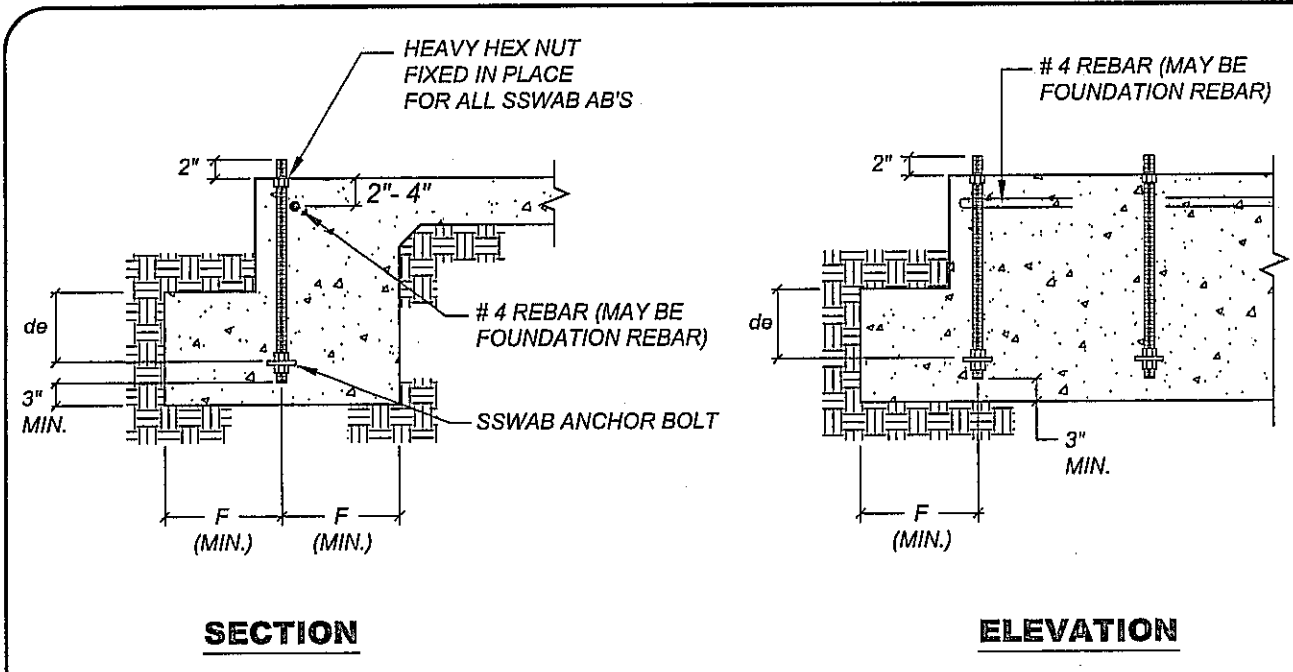
HRD ENGINEERING
 STRUCTURAL DESIGN AND DRAFTING
 7463 VARNA AVE. SECOND FLOOR
 N. HOLLYWOOD, CA 91605
 PHONE NO. (805) 431-5415 - FAX NO. (805) 888-4840

SECOND FLOOR FRAMING PLAN

TWO STORY SINGLE FAMILY RESIDENCE
 901 LAUREL CYN.
 LA, CA 90046 FOR:
 L.I. INVESTMENTS, LLC



Date	12-18-12
Scale	
Drawn	hrd
Job	
Sheet	5-5.1

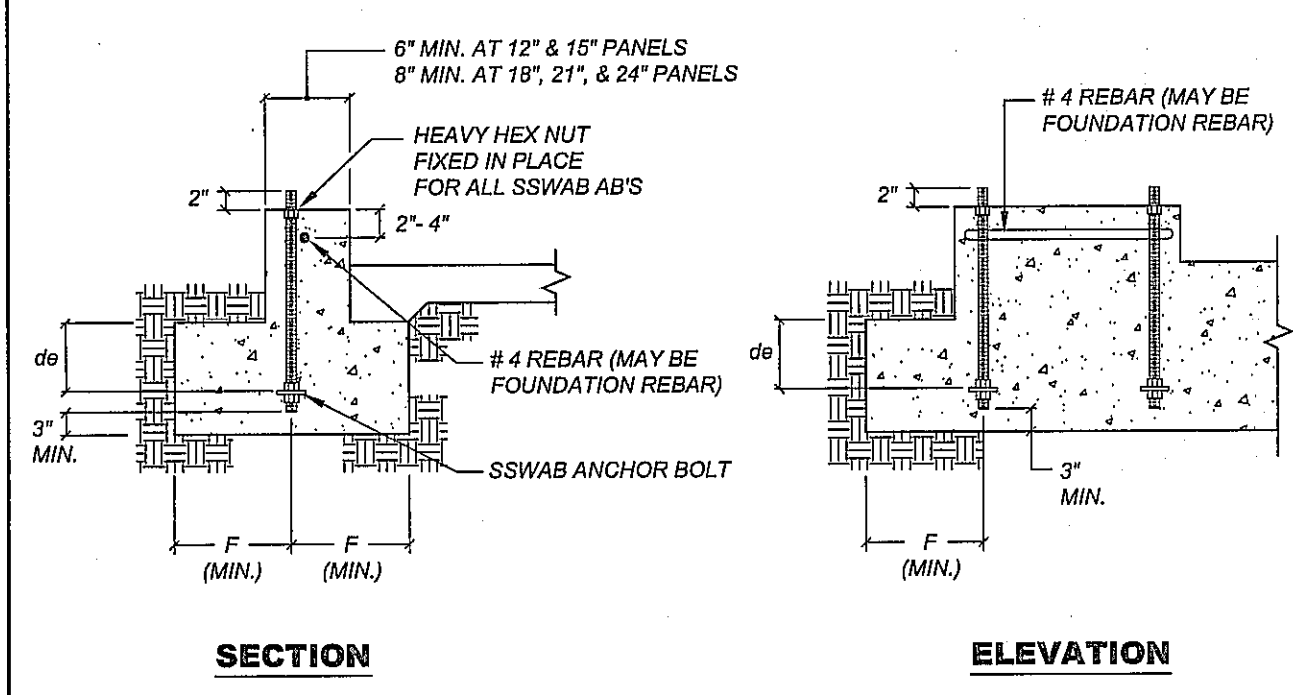


SECTION **ELEVATION**

NOTE:
1. REFER TO EMBEDMENT SCHEDULE ON DETAIL 4
2. FOUNDATION AND STEM WALL (SIZE AND REINFORCEMENT) BY OTHERS

ENGINEER OF RECORD IS PERMITTED TO MODIFY DETAILS FOR SPECIFIC CONDITIONS.

SSW ANCHORAGE (SLAB ON GRADE) 1

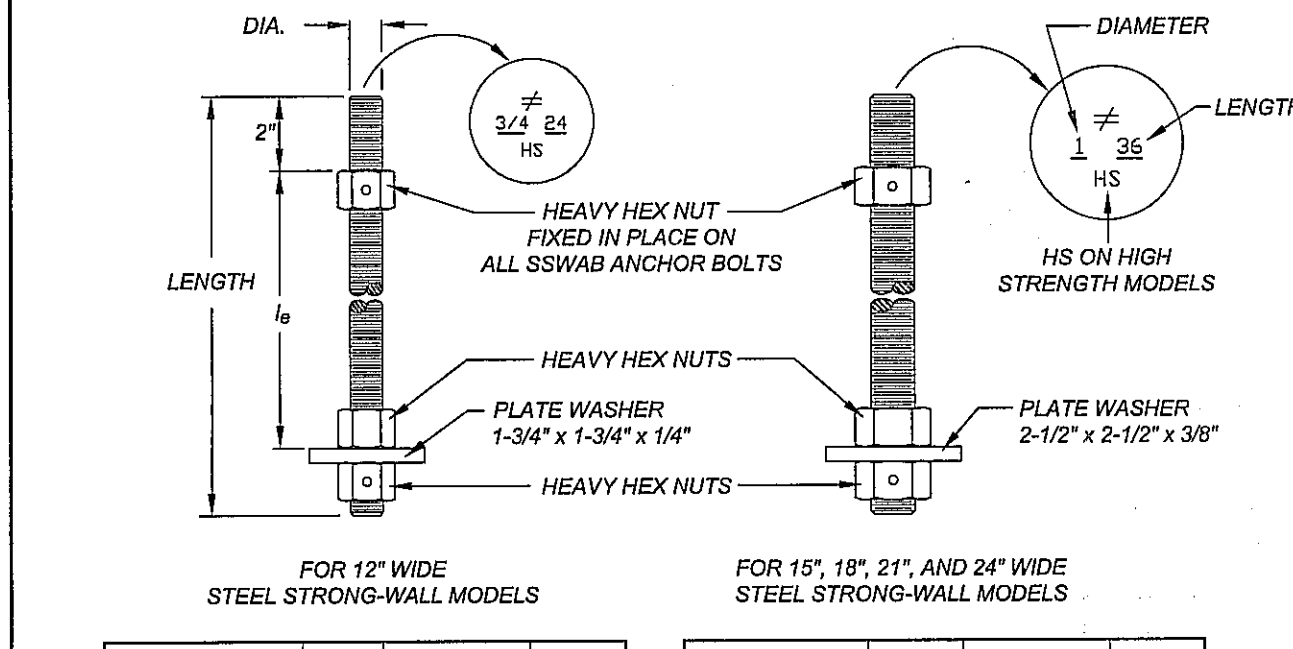


SECTION **ELEVATION**

NOTE:
1. REFER TO EMBEDMENT SCHEDULE ON DETAIL 4
2. FOUNDATION AND STEM WALL(SIZE AND REINFORCEMENT) BY OTHERS

ENGINEER OF RECORD IS PERMITTED TO MODIFY DETAILS FOR SPECIFIC CONDITIONS.

SSW ANCHORAGE (CURB / STEM WALL) 2



FOR 12" WIDE STEEL STRONG-WALL MODELS				FOR 15", 18", 21", AND 24" WIDE STEEL STRONG-WALL MODELS			
MODEL NO.	DIA.	TOTAL LENGTH	l_o	MODEL NO.	DIA.	TOTAL LENGTH	l_o
SSWAB3x4x24	3/4"	24"	20"	SSWAB1x24	1"	24"	20"
SSWAB3x4x24HS	3/4"	24"	20"	SSWAB1x24HS	1"	24"	20"
SSWAB3x4x36HS	3/4"	36"	32"	SSWAB1x36HS	1"	36"	32"

STEEL STRONG-WALL ANCHOR BOLT 3

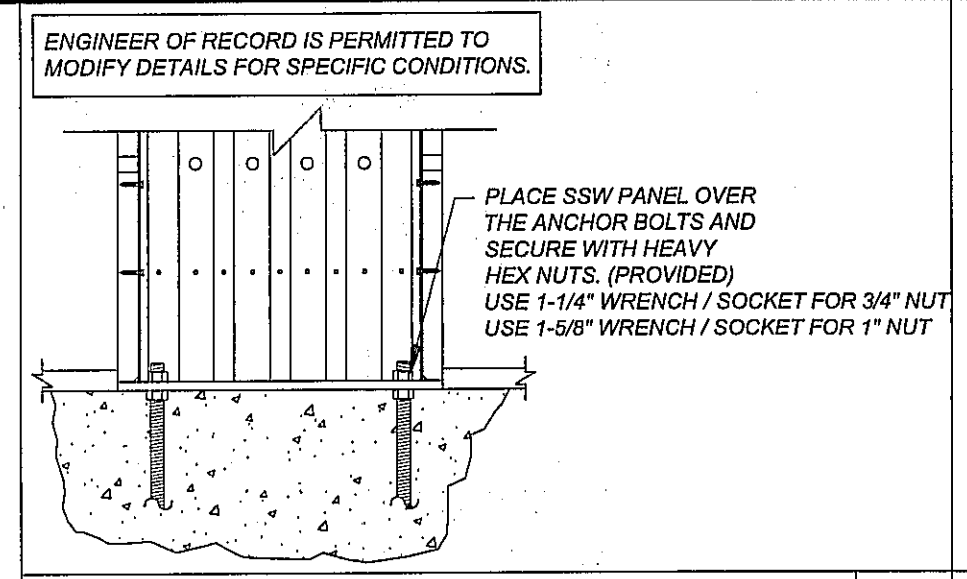
Embedment Schedule for 1997 UBC Loads*									
Wall Model	Wind					Seismic			
	2500 psi	3000 psi	2500 psi	3000 psi	3000 psi	d_e	F	d_e	F
pH = 0.85 (reinforcement not provided per Sect. 1923.3.2)									
12" Wall	12"	12"	12"	12"	11"	11"	10"	10"	10"
15" Wall	13"	13"	12"	12"	11"	11"	10"	10"	10"
18" Wall	15"	15"	15"	15"	14"	14"	13"	13"	13"
21" Wall	16"	16"	15"	15"	14"	14"	13"	13"	13"
24" Wall	16"	15"	15"	15"	14"	14"	13"	13"	13"
pH = 0.85 (reinforcement provided per Sect. 1923.3.2)									
12" Wall	10"	10"	10"	10"	10"	9"	9"	9"	9"
15" Wall	11"	11"	11"	11"	10"	10"	9"	9"	9"
18" Wall	13"	13"	13"	13"	12"	12"	11"	11"	11"
21" Wall	14"	14"	13"	13"	12"	12"	11"	11"	11"
24" Wall	14"	14"	13"	13"	12"	12"	11"	11"	11"

Embedment Schedule for 2000 IBC Loads*									
Wall Model	SDC A & B or Wind					SDC C through F			
	2500 psi	3000 psi	2500 psi	3000 psi	3000 psi	d_e	F	d_e	F
12" Wall	10"	10"	10"	10"	10"	9"	9"	9"	9"
15" Wall	12"	12"	11"	11"	11"	10"	10"	10"	10"
18" Wall	14"	14"	14"	14"	13"	13"	12"	12"	12"
21" Wall	15"	15"	14"	14"	13"	13"	12"	12"	12"
24" Wall	15"	15"	14"	14"	13"	13"	12"	12"	12"

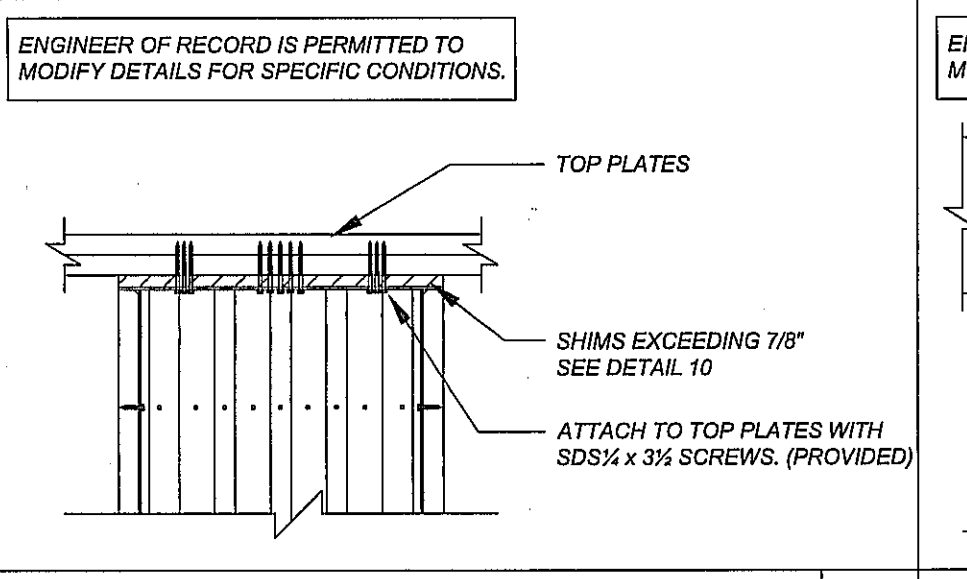
Anchor Bolt Embedment Schedule General Notes

- Anchor bolts shall be high strength ASTM A449 or equivalent. Nuts shall be heavy hex ASTM A563 Grade DH or heavy hex ASTM A194 Grade 2H.
- Where justified by analysis, the registered design professional may specify alternate embedment or bolt grade.
- Footings dimensions and rebar requirements are for anchorage only.

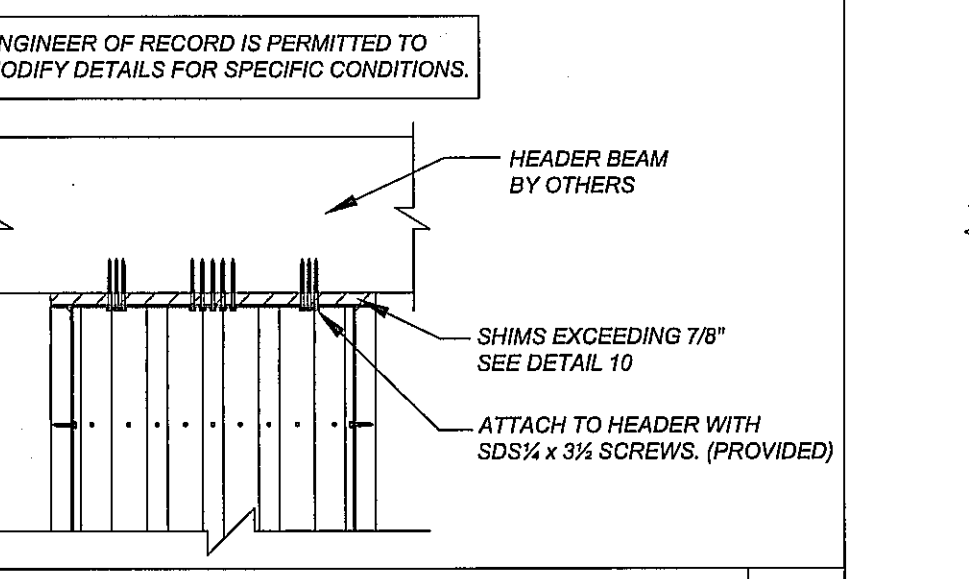
SSW EMBEDMENT SCHEDULE 4



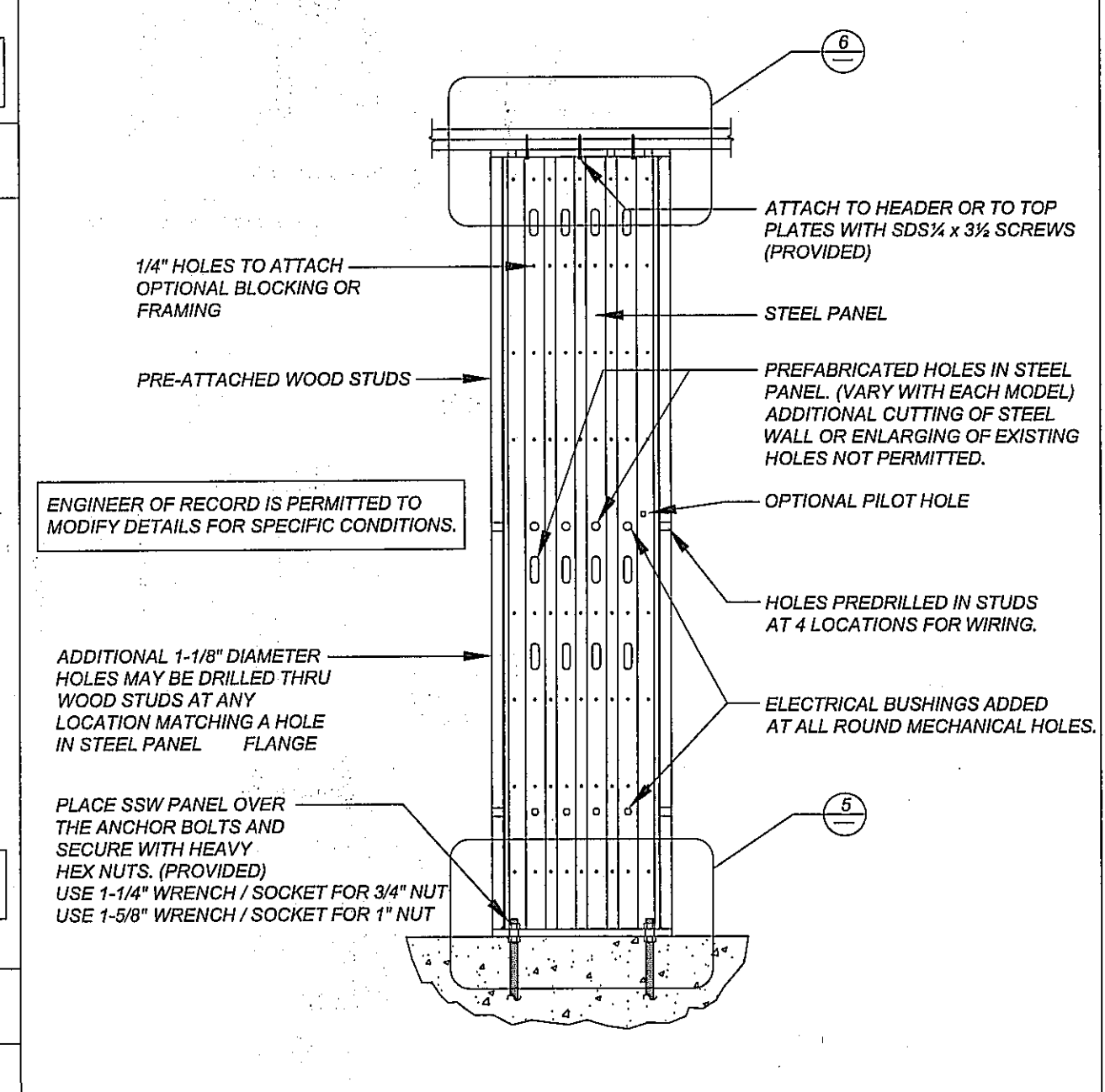
SSW BASE PLATE CONNECTION 5



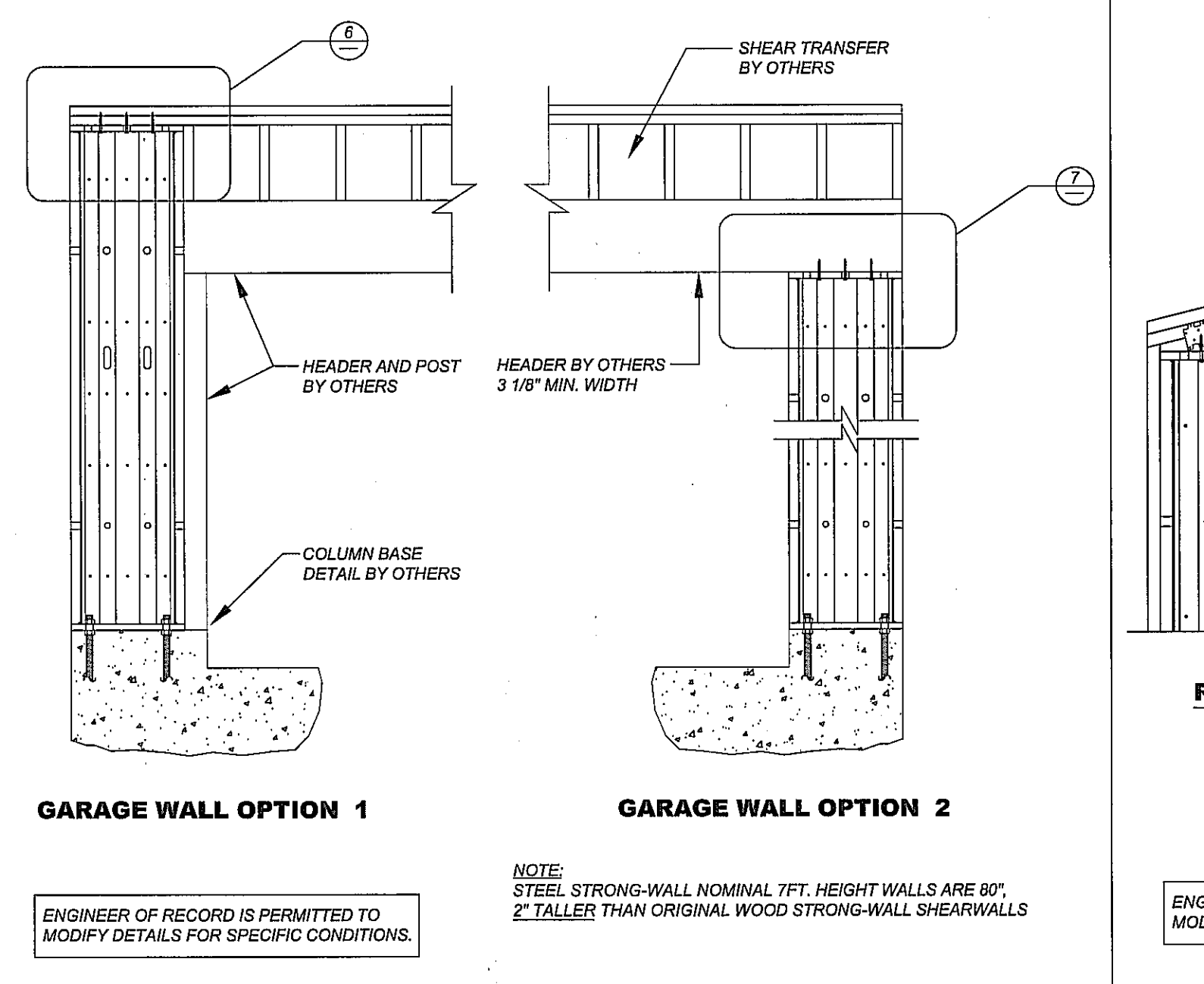
SSW TOP PLATE CONNECTION 6



SSW HEADER CONNECTION 7



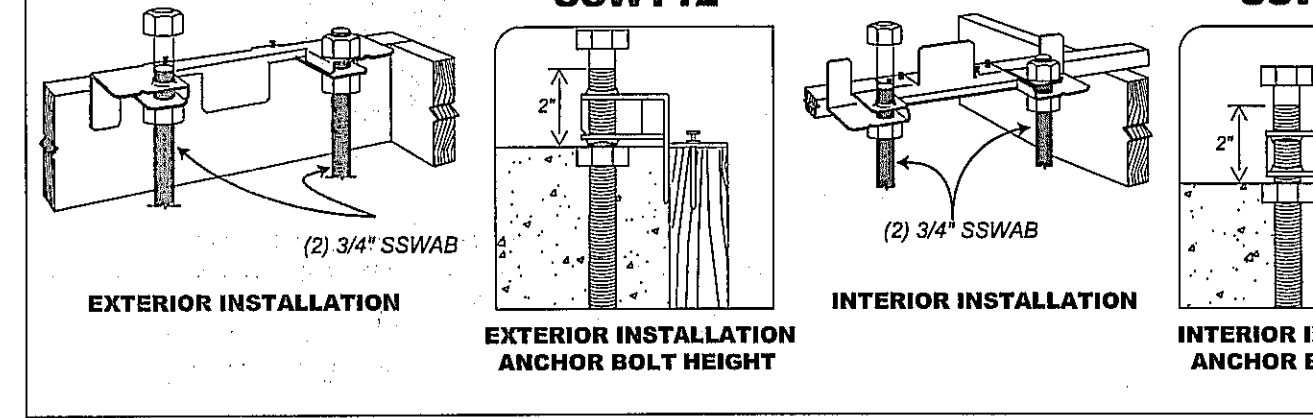
GARAGE WALL OPTION 1



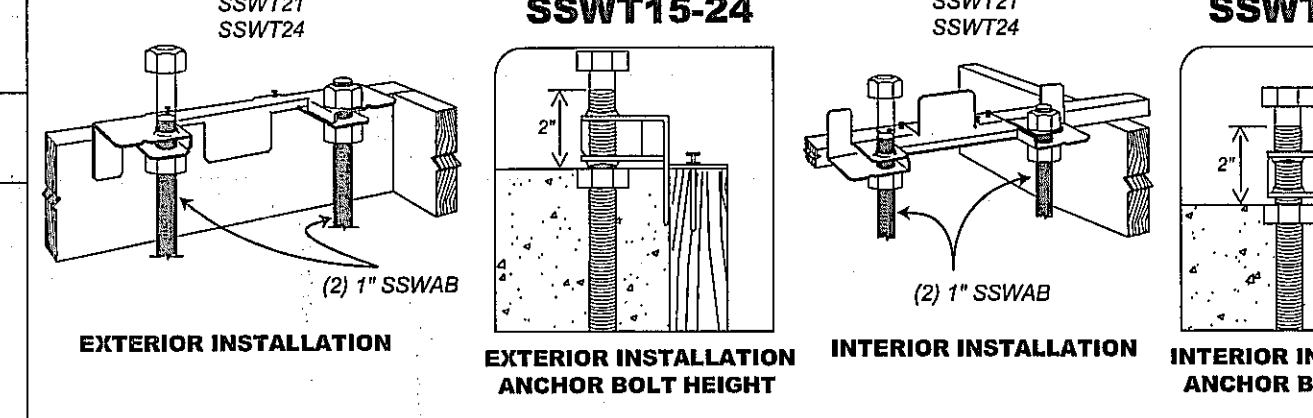
GARAGE WALL OPTION 2

STEEL STRONG-WALL 8

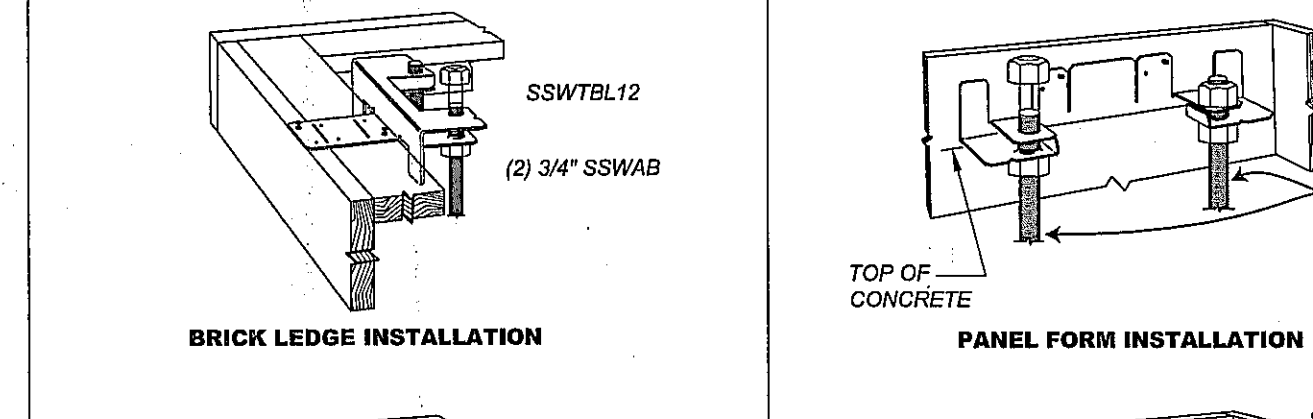
STEEL STRONG-WALL AT GARAGE 9



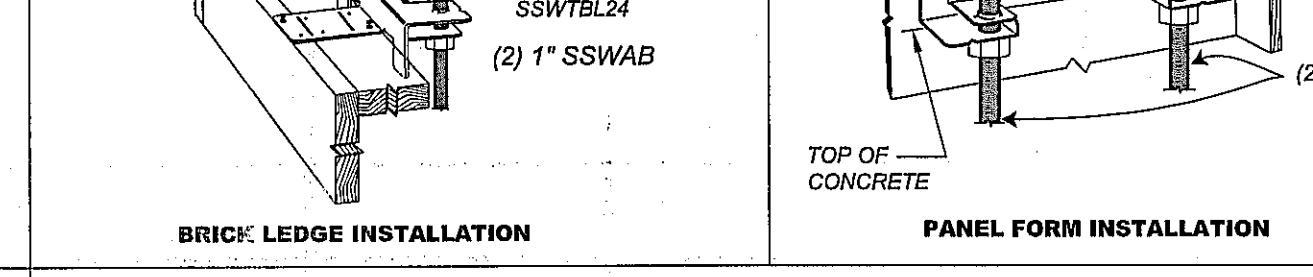
SSWT12 EXTERIOR and INTERIOR INSTALLATION



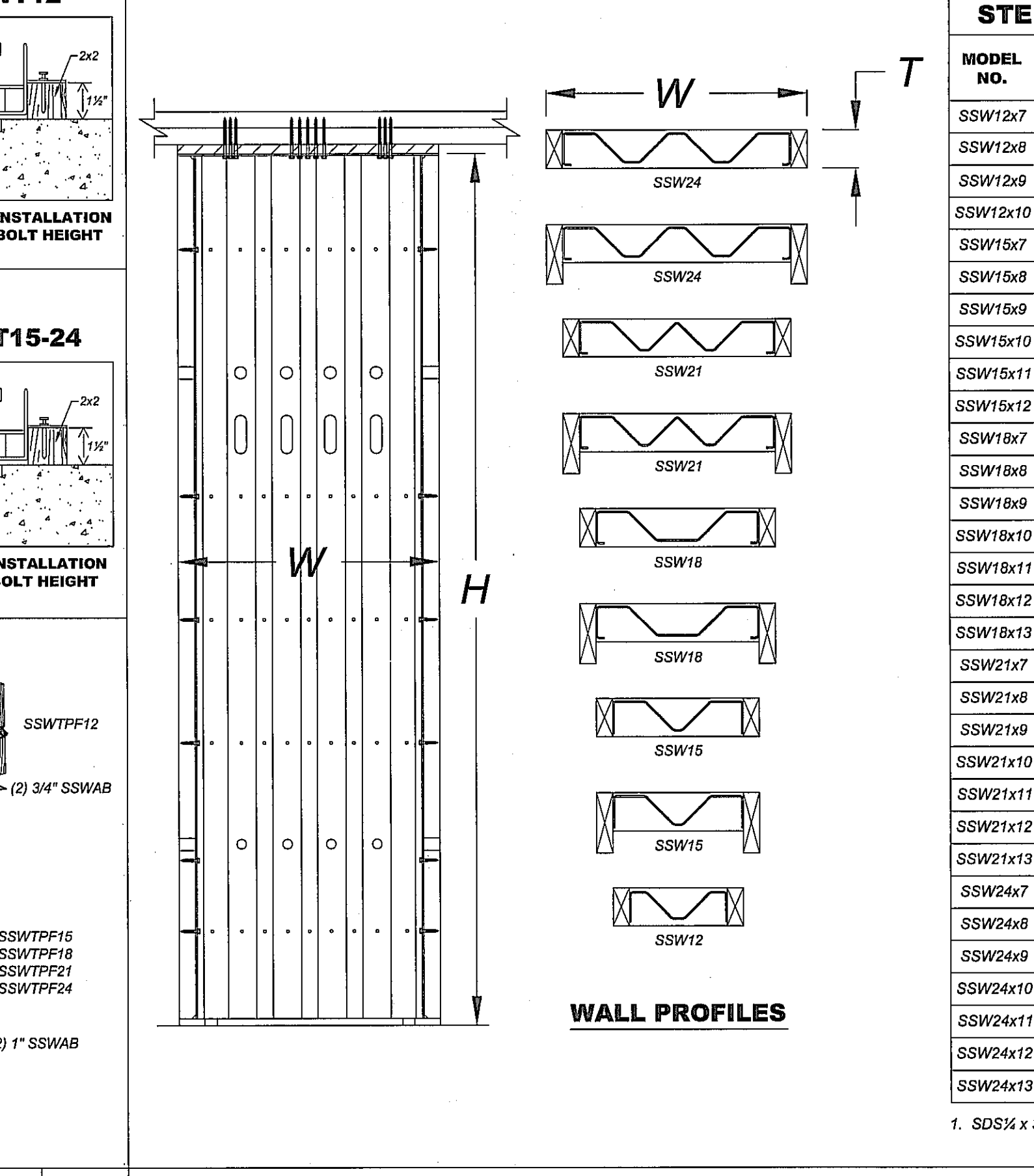
SSWT15-24 EXTERIOR and INTERIOR INSTALLATION



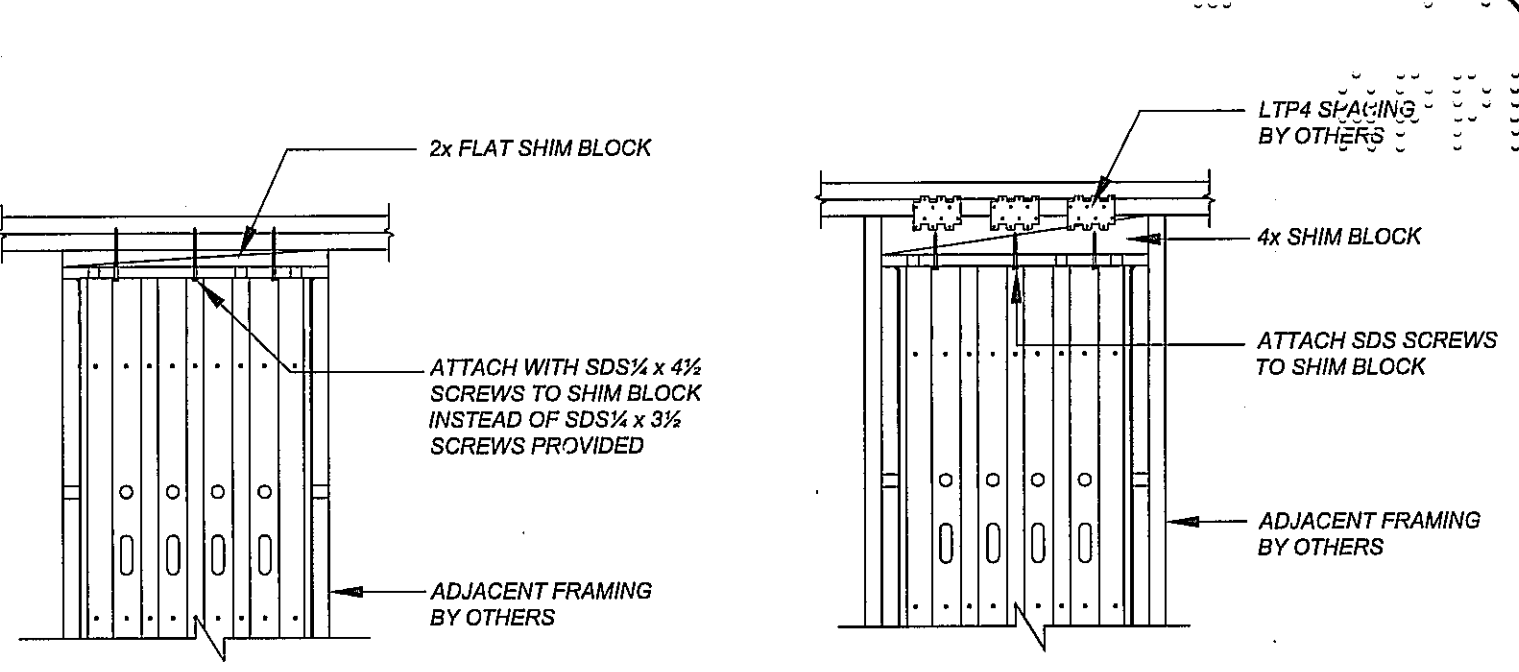
SSWTBL BRICK LEDGE and SSWTPF PANEL FORM INSTALLATION



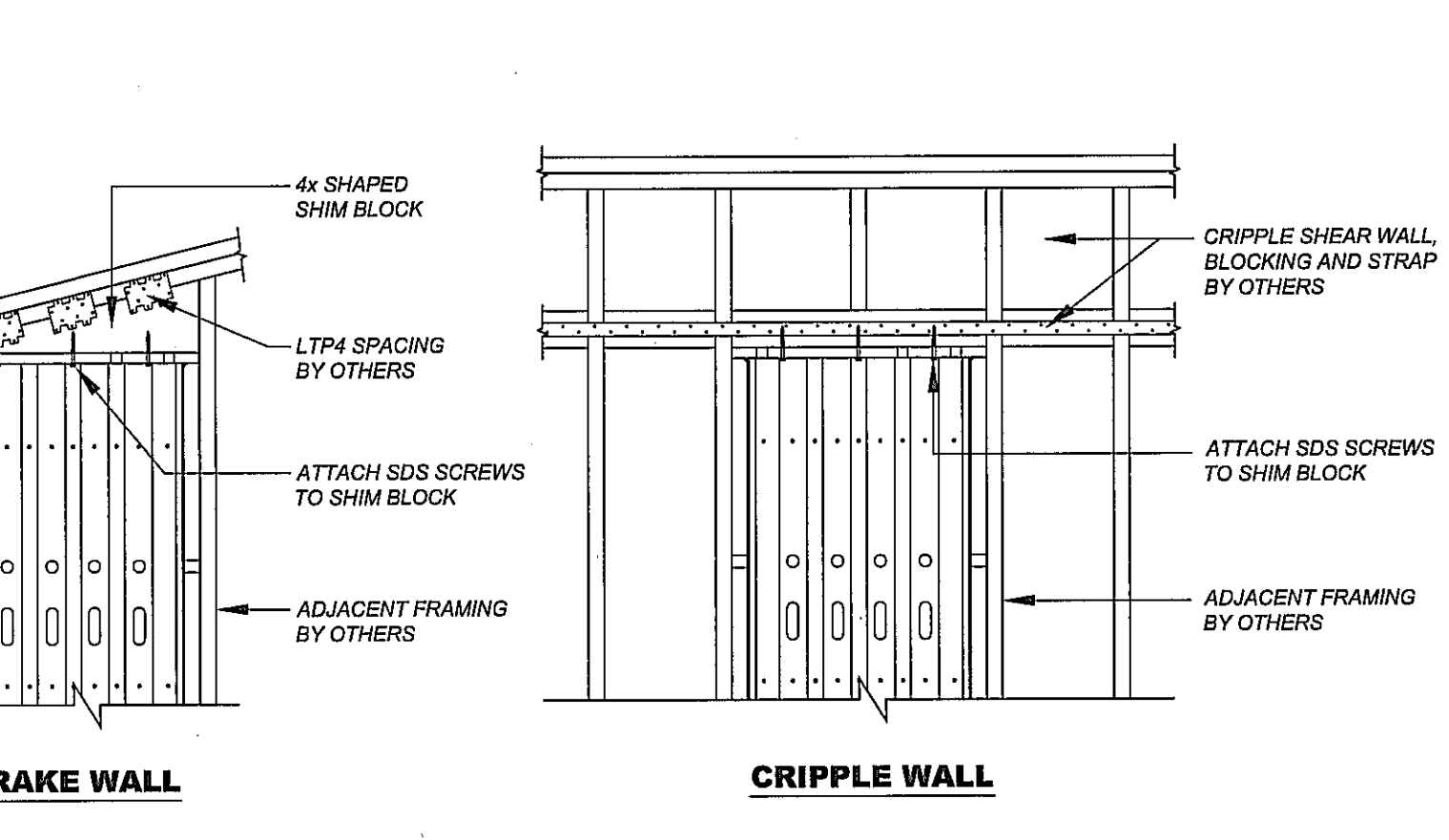
SSWTBL15 BRICK LEDGE and SSWTPF16 PANEL FORM INSTALLATION



STEEL STRONG-WALL SPECIFICATIONS & WALL PROFILES 11



2x FLAT SHIM BLOCK and 4x SHIM BLOCK



RAKE WALL and CRIPPLE WALL

ENGINEER OF RECORD SHALL DESIGN FOR:
1. SHEAR TRANSFER
2. OUT OF PLANE LOADING EFFECT
3. INCREASED OVERTURNING AND DRIFT DUE TO ADDITIONAL HEIGHT.

ENGINEER OF RECORD SHALL DESIGN FOR 12

STEEL STRONG-WALL HEIGHT ADJUSTMENTS 10

STEEL STRONG-WALL MODELS						
MODEL NO.	W (in)	H (in)	T (in)	HOLDOWN ANCHOR BOLTS	QTY. OF TOP OF WALL SCREWS*	
SSW12x7	12	80	3 1/2	(2) 3/4" H.S.	4	
SSW12x8	12	93 1/4	3 1/2	(2) 3/4" H.S.	4	
SSW12x9	12	105 1/4	3 1/2	(2) 3/4" H.S.	4	
SSW12x10	12	117 1/4	3 1/2	(2) 3/4" H.S.	4	
SSW15x7	15	80	3 1/2	(2) 1" H.S.	6	
SSW15x8	15	93 1/4	3 1/2	(2) 1" H.S.	6	
SSW15x9	15	105 1/4	3 1/2	(2) 1" H.S.	6	
SSW15x10	15	117 1/4	3 1/2	(2) 1" H.S.	6	
SSW15x11	15	129 1/4	5 1/2	(2) 1" H.S.	6	
SSW18x7	18	80	3 1/2	(2) 1" H.S.	9	
SSW18x8	18	93 1/4	3 1/2	(2) 1" H.S.	9	
SSW18x9	18	105 1/4	3 1/2	(2) 1" H.S.	9	
SSW18x10	18	117 1/4	3 1/2	(2) 1" H.S.	9	
SSW18x11	18	129 1/4	5 1/2	(2) 1" H.S.	9	
SSW18x12	18	141 1/4	5 1/2	(2) 1" H.S.	9	
SSW18x13	18	153 1/4	5 1/2	(2) 1" H.S.	9	
SSW21x7	21	80	3 1/2	(2) 1" H.S.	12	
SSW21x8	21	93 1/4	3 1/2	(2) 1" H.S.	12	
SSW21x9	21	105 1/4	3 1/2	(2) 1" H.S.	12	
SSW21x10	21	117 1/4	3 1/2	(2) 1" H.S.	12	
SSW21x11	21	129 1/4	5 1/2	(2) 1" H.S.	12	
SSW21x12	21	141 1/4	5 1/2	(2) 1" H.S.	12	
SSW21x13	21	153 1/4	5 1/2	(2) 1" H.S.	12	
SSW24x7	24	80	3 1/2	(2) 1" H.S.	14	
SSW24x8	24	93 1/4	3 1/2	(2) 1" H.S.	14	
SSW24x9	24	105 1/4	3 1/2	(2) 1" H.S.	14	
SSW24x10	24	117 1/4	3 1/2	(2) 1" H.S.	14	
SSW24x11	24	129 1/4	5 1/2	(2) 1" H.S.	14	
SSW24x12	24	141 1/4	5 1/2	(2) 1" H.S.	14	
SSW24x13	24	153 1/4	5 1/2	(2) 1" H.S.	14	

STEEL STRONG-WALL HEIGHT ADJUSTMENTS 10

STEEL STRONG-WALL TEMPLATES 11

STEEL STRONG-WALL SPECIFICATIONS & WALL PROFILES 12

NOTES 13

NO.	DATE	REVISIONS	BY

SIMPSON STRONG-TIE COMPANY, INC.
 HOME OFFICE: 4120 DUBLIN BLVD., #400, DUBLIN, CA 94568
 TEL: (800) 999-5099, FAX: (925) 875-0826
 SIMPSON STRONG-TIE COMPANY, INC. IS AN ISO 9001 REGISTERED COMPANY.

STEEL STRONG-WALL
 ENGINEERED WALL APPLICATIONS

SIMPSON STRONG-TIE COMPANY, INC.
 HOME OFFICE: 4120 DUBLIN BLVD., #400, DUBLIN, CA 94568
 TEL: (800) 999-5099, FAX: (925) 875-0826
 SIMPSON STRONG-TIE COMPANY, INC. IS AN ISO 9001 REGISTERED COMPANY.

NAME: B.H.
 DATE: 10-04-05
 SCALE: NO SCALE
 CHECKED: R.A.
 SHEET: SSW1
 OF OF SHEETS: 13
 JOB NO.

REVISIONS BY

THESE DRAWINGS, DESIGNS AND SPECIFICATIONS ARE THE EXCLUSIVE PROPERTY OF HRD ENGINEERING. NO PART OF THESE DRAWINGS, DESIGNS OR SPECIFICATIONS MAY BE REPRODUCED OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, WITHOUT WRITTEN APPROVAL OF HRD ENGINEERING. COPYRIGHT © HRD ENGINEERING.

HRD ENGINEERING
 STRUCTURAL DESIGN AND DRAFTING
 7463 VARNA AVE., SECOND FLOOR
 N. HOLLYWOOD, CA 91605
 PHONE NO. (818) 431-5415 - FAX NO. (818) 888-4840

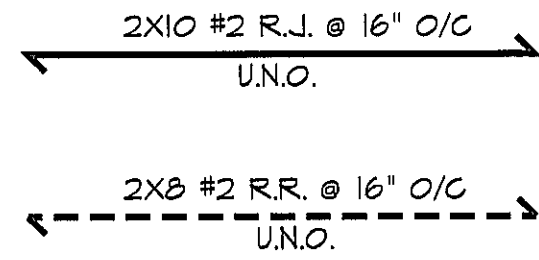
SSW (STEEL STRONG WALL)

TWO STORY SINGLE FAMILY RESIDENCE
 901 LAUREL CYN.
 L.A., CA 90046 FOR: L.I. INVESTMENTS, LLC

Date: 12-10-12
 Scale:
 Drawn: hrd
 Job:
 Sheet:
SSW1
 of

AT FIN WALL (SEE DET.) - 2X6 STUDS ON INTERIOR SIDE OF WALL CONT. FOUND. TO TOP OF FIN WALL (APPROX. 25 1/2 FEET (F.V.) - NO CUTS, LAPS ALLOWED - STUDS CONT. REQUIRED BY ARCHITECTURAL FEATURE

ROOF FRAMING LEGEND

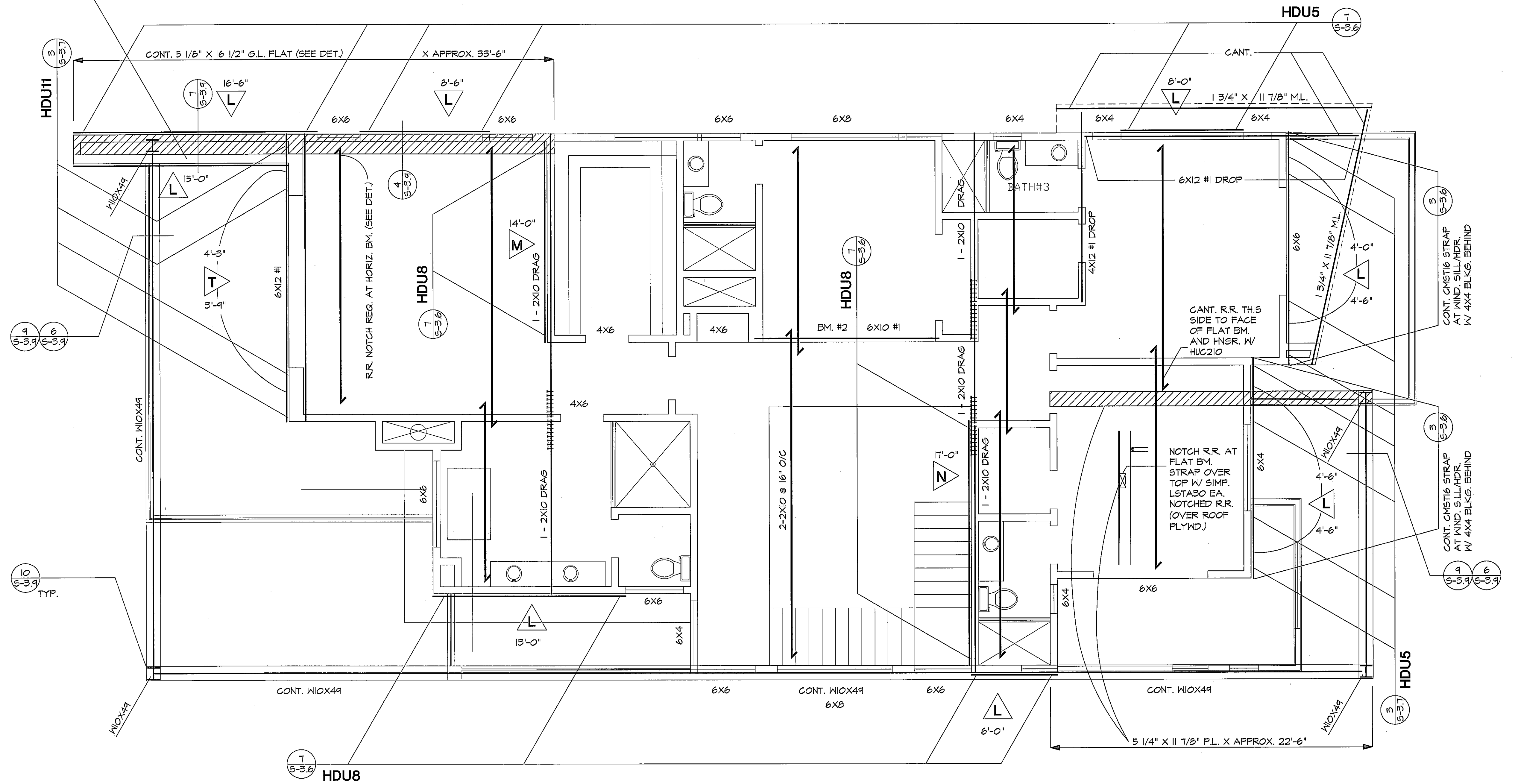


||||| MS112 (U.N.O.) SEE DTL. 0/5-3.3

||||| CMST16 CONT. STRAP FOR DIST. SHOWN

NOTES:

1. ALL EXT. WALLS TO BE 2X6 @ 16" O/C - ALL INTERIOR WALLS TO BE 2X4 @ 16" O/C, EXCEPT FOR SHEAR WALLS - USE: 3X4 OR 2X6 STUDS @ 16" O/C.



1
S-5.2
ROOF FRAMING PLAN

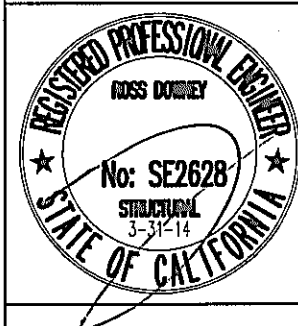
SCALE: 1/4"=1'-0"

REVISIONS	BY

HRD ENGINEERING
STRUCTURAL DESIGN AND DRAFTING
7463 VARNA AVE., SECOND FLOOR
N. HOLLYWOOD, CA 91605
PHONE NO. (805) 431-5415 - FAX NO. (805) 888-4840
COPYRIGHT © HRD ENGINEERING

ROOF FRAMING PLAN

TWO STORY SINGLE
FAMILY RESIDENCE
901 LAUREL CYN.
LA, CA 90046 FOR:
L.I. INVESTMENTS, LLC



Date	12-12-12
Scale	
Drawn	hrd
Job	
Sheet	S-5.2
Of	